

TOWN OF ZEBULON PLANNING BOARD MEETING March 11, 2024 6:00 PM

- I. CALL TO ORDER
- II. APPROVAL OF THE AGENDA
- **III. ELECTION OF VICE-CHAIR**

IV. ADOPTION OF MINUTES

- **a.** September 18, 2023
- **b.** October 9, 2023
- **c.** December 11, 2023

V. OLD BUSINESS

a. PD 2024-01 Zebulon South – The Town has received a Planned Development request to develop 320 residential units (townhomes and single-family detached units) on 118.62 acres at 751 S Wakefield Rd, 700 S Arendell Ave, 0 S Wakefield St, and 0 S Arendell.

VI. DEVELOPMENT UPDATES

VII. ADJOURNMENT

Zebulon Planning Board Minutes September 18, 2023

Present: David Lowry, Laura Johnson, Michael Germano, Domenick Schilling, Stephanie Jenkins, Peggy Alexander, Michael Clark-Planning, Stacie Paratore-Deputy Town Clerk, Adam Culpepper-Planning, Cate Farrell-Planning, Sam Slater-Town Attorney

David Lowry called the meeting to order.

APPROVAL OF AGENDA

Laura Johnson made a motion, second by Domenick Schilling to approve the agenda. There was no discussion and the motion passed unanimously.

ADOPTION OF MINUTES

Michael Germano made a motion, second by Stephanie Jenkins to approve the May 8, 2023 minutes. There was no discussion and the motion passed unanimously.

NEW BUSINESS

A. CZ 2023-04 1620 N. Arendell Avenue

Cate Farrell stated this was a conditional rezoning request for a 1.117-acre parcel from Residential Suburban (R2) to Heavy Commercial – Conditional (HC-C) District for the development of a Veterinary Clinic.

The standards under section 2.2.6.K for a conditional rezoning were:

- 1. Health, safety and welfare
- 2. Appropriate for location
- 3. Reasonable in the public interest
- 4. Concept plan consistent with regulations
- 5. Other relevant factors

The public hearing notification process was detailed. The aerial map, zoning map, future land use plan, timeline, concept plan and proposed elevations were shown. The applicant proposed the following conditions:

- Reduction in the distance the pet exercise zone can be from a residentially zoned parcel.
- Modification on a type D buffer

Cate Farrell spoke about the separation requirements and explained the applicant asked for a reduction from 200ft to 75ft for the separation of the outdoor pet exercise area and a six feet opaque fence to help with screening. The location of the outdoor area was shown being located on the side of Green Pace Rd and was screened from the main corridor of Arendell Avenue. It was explained the buffer was a modified type D buffer and the applicant was asking for a modification from a 40ft buffer to a 10ft buffer on the southeast side and a 20ft. buffer on the northeast side. A fence was proposed along the frontage for additional screening.

Planning Board Minutes September 18, 2023

The proposed road improvements included adding a second lane along the Arendell Avenue frontage and an addition of a sidewalk and planting strip. No Traffic Impact Analysis ("TIA") was needed since the proposed use would not meet the threshold of 50 peak trips or 150 daily trips.

The applicant received 60 points under the Town's Utility Allocation Policy. There were details given about the amenities added to receive points.

David Lowry asked for clarification on the reason for the buffer change. Staff stated the change gave more room for parking, the building, the dog walking area and the stormwater retention.

There was a clarification that the sidewalk along Green Pace would be 5ft wide with a 5ft planting strip.

Michael Germano stated he had concerns about there being so many opaque fences along Arendell and no visual connection. It was suggested the applicant remove the opaque fence.

There was discussion about traffic concerns.

Michael Germano asked if the applicant was willing to reduce the buffer. Graham Smith stated the applicant was agreeable to reduce the buffer.

Michael Germano made a motion, second by Peggy Alexander to recommend approval of CZ 2023-04 with the modification to reduce the buffer along the south property line and remove the fence. There was no further discussion and the motion passed unanimously.

B. Discussion about the update of the Comprehensive and Future Land Use Plans Michael Clark spoke about the goals of the Town's 2030 Strategic Plan of vibrant downtown, small town life and growing smart and how the goals would tie into the Comprehensive and Land Use Plans. The purpose of the plans was given. A chart was shown reflecting the population growth of towns in Wake County from 1990 and future population growth was shown. The demographics of those living in Zebulon were reflected on a graph.

There was discussion about the need for public transportation.

Race, ethnicity, economics and education information for those living in Town was detailed. There was a discussion about economic resources and opportunities and how to diversify employment options.

A map showing the land use makeup and the Planning Board was asked to look at the patterns for vacant/agricultural land, residential, commercial, and industrial.

Some of the significant issues the Town faced were utilities, land use patterns, non-residential uses, economics, infrastructure and traffic. There was a detailed discussion about the Planning Board's thoughts on each issue.

Planning Board Minutes September 18, 2023

Mr. Clark stated the Economic Development Strategic Plan had started and would focus on the big picture economic plan. The Town Sub Area Plan would start in the spring and would focus on details and uses downtown.

The next step for the plans included an in-person public work session in October on Land Use and Design. The Planning Board was asked to think about what kind of Town Zebulon should be with a population of 50,000 and their likes and dislikes about where they live.

DEVELOPMENT UPDATES

Adam Culpepper provided development updates.

Peggy Alexander made a motion, second by Michael Germano to adjourn. There was no discussion and the motion passed unanimously.

Adopted this the 11th day of March 2024.

David Lowry—Chair

SEAL

Stacie Paratore, CMC—Deputy Town Clerk

Zebulon Planning Board Minutes October 9, 2023

Present: David Lowry, Laura Johnson, Michael Germano, Domenick Schilling, Stephanie Jenkins, Peggy Alexander, Michael Clark-Planning, Stacie Paratore-Deputy Town Clerk, Sam Slater-Town Attorney

David Lowry called the meeting to order.

APPROVAL OF AGENDA

Laura Johnson made a motion, second by Stephanie Jenkins to approve the agenda. There was no discussion and the motion passed unanimously.

ADOPTION OF MINUTES

Michael Germano asked to clarify his comments to the August 14, 2023 minutes.

Michael Germano made a motion, second by Stephanie Jenkins to approve the August 14, 2023 minutes as amended. There was no discussion and the motion passed unanimously.

NEW BUSINESS

A. PD 2023-01 Chamblee Lake

Michael Clark stated this was a Planned Development request to develop 355 residential units on 136 acres at 1509 Chamblee Road. The current zoning was Wake County R-30.

The standards under section 2.2.24.J for a rezoning were:

- 1. Health, safety and welfare
- 2. Appropriate for location
- 3. Reasonable in the public interest
- 4. Other relevant factors

The public hearing notification process was detailed. The aerial map, zoning map, future land use plan, timeline, concept plan and proposed elevations were shown. The applicant exceeded the UDO requirements for architecture guidelines. The proposed road improvements were at the following locations:

- Chamblee Road and Site Drive #1
- Chamblee Road and Site Drive #2
- Chamblee Road and Site Drive #3
- Perry Curtis Road and Site Drive #4
- Perry Curtis Road and NC 96
- Wake County Line Road and NC 39
- Perry Curtis Road/Wake County Line Road and Chamblee Road
- NC 39 and Old US 264 Hwy

Planning Board Minutes October 9, 2023

There was discussion about the concerns over neighboring wells, smart growth, police and fire response times and the fiscal analysis. Staff explained the estimated total tax value of the development would exceed \$102 million.

Ryan Acres from McAdams Company provided more details about the nearby wells and gave details about stormwater runoff. There was information given about how the stormwater runoff on the site would be controlled.

Domenick made a motion, second by Laura Johnson to recommend approval of PD 2023-01 Chamblee Lake. There was no discussion and the motion passed unanimously.

OLD BUSINESS

A. Discussion about the updates of the Comprehensive and Future Land Use Plans The Planning Board was asked what they liked and disliked about where they lived. Some likes included family friendly neighborhood, amenities for kids, community center, walkability, and small-town feel. Some dislikes that were mentioned included needing more commercial opportunities, traffic, lack of restaurants and entertainment, similar homes and lack of public transportation.

There was a detailed discussion about density and mixed use and examples were given.

Michel Clark printed the future land use map for each Planning Board member and asked them to color code the land uses and zoning districts for the next meeting.

The first Community Engagement Session was scheduled for October 30, 2023 at 6:00pm at the Community Center.

DEVELOPMENT UPDATES

Michael Clark provided development updates.

Michael Germano made a motion, second by Domenick Schilling to adjourn. There was no discussion and the motion passed unanimously.

Adopted this the 11th day of March 2024.

David Lowry-Chair

SEAL

Stacie Paratore, CMC—Deputy Town Clerk

Zebulon Planning Board Minutes December 11, 2023

Present: David Lowry, Michael Germano, Domenick Schilling, Stephanie Jenkins, Michael Clark-Planning, Stacie Paratore-Deputy Town Clerk, Sam Slater-Town Attorney

Absent: Laura Johnson

David Lowry called the meeting to order.

APPROVAL OF AGENDA

Michael Germano made a motion, second by Stephanie Jenkins to approve the agenda. There was no discussion and the motion passed unanimously.

OLD BUSINESS

A. 2-Year Land Use Plan and Transportation Plan Updates

Michael Clark gave a Land Use Planning update. The results from the public engagement were detailed. During the group discussion most supported mixed use, the option for single family neighborhoods, micro-commercial in neighborhoods and more walkable or short drive to the grocery store and other needs.

Different areas and housing options were shown, and the Planning Board gave their opinions of each. There was discussion about the commercial needs for Zebulon residents.

The Planning Board was asked a series of questions regarding mixed use, urban development, uses within neighborhoods and changes to the Future Land Use Map and there was detailed conversation about each topic.

The Planning Board discussed prioritizing mixed use development and gave specific areas including Green Pace and Arendell and Five County Stadium area.

Michael Clark showed the increase of population and certificates of occupancy on a graph. The population projections to 2050 were detailed.

There was more discussion about the appropriate scale of density in Zebulon and the Planning Board's thoughts on density bonuses.

Planning Board Minutes December 11, 2023

DEVELOPMENT UPDATES

Michael Clark provided development updates.

Michael Germano made a motion, second by Stephanie Jenkins to adjourn. There was no discussion and the motion passed unanimously.

Adopted this the 11th day of March 2024.

David Lowry—Chair

SEAL

Stacie Paratore, CMC—Deputy Town Clerk



STAFF REPORT PLANNED DEVELOPMENT 2024-01 ZEBULON SOUTH MARCH 11, 2024

Topic:PD 2024-01 Zebulon South, Project Number: 886895Speaker:Adam Culpepper, Senior PlannerFrom:Michael J. Clark, AICP, CNU-A, Planning DirectorPrepared by:Catherine Farrell, Planner IIApproved by:Joseph M. Moore II, PE, Town Manager

Executive Summary:

The Board of Commissioners will consider a Planned Development Rezoning for 751 S Wakefield St (PIN# 2704492511), 700 S Arendell Ave (PIN# 2705512202), 0 S Wakefield (PIN# 2705413075). This is a legislative case.

Background:

The Town received a Planned Development request to develop 320 residential units (townhomes and single-family detached units) on 116.14 acres. The land is owned by Harold Narron and Fred Corbett (PIN# 2704492511); Joseph Temple Sr and Alexander Harrison (PIN# 2705512202); Watson Family II LLC (PIN# 2705413075), and is currently in the Town of Zebulon ETJ, and zoned R-2 and R-4. The applicant is seeking annexation simultaneously with this rezoning request.

A Joint Public Hearing was held on February 12, 2024. On the same date, the Planning Board voted to table their decision to March 11, 2024.

Discussion:

The Board shall consider the following questions to determine whether the rezoning is consistent with the intent of the Unified Development Ordinance (Section 2.2.24.J):

- 1. Does the request advance the public health, safety, or welfare?
- 2. Is the request appropriate for its proposed location, and is consistent with the purposes, goals, objectives, and Town's policies?
- 3. Is the request reasonable and in the public interest?
- 4. Are there other factors which the Board of Commissioners determines relevant?

Policy Analysis:

Grow Zebulon: Comprehensive Land Use Plan (Land Use Plan):

The Land Use Plan (adopted June 2021) designated this area "Suburban Residential" and "General Residential". These designations allow a mixture of product types, with increased open space to preserve an overall suburban character, and encourages some density with the inclusion of single family attached lots (Land Use and Development Page 13 & 14, Attached).

Suburban Residential characteristics include a greater focus on the home and less on driveways consuming a large percentage of the front lawn. These characteristics are preserved through alley-loaded town homes while reserving front-loaded homes to wider lots.



STAFF REPORT PLANNED DEVELOPMENT 2024-01 ZEBULON SOUTH MARCH 11, 2024

General Residential characteristics include a denser residential buildings. This may include single family detached and single family attached.

Grow Zebulon: Comprehensive Transportation Plan (Transportation Plan):

The Transportation Plan calls for the construction of a 4-lane median divided arterial road section along the properties fronting S Wakefield St, and a 2-lane median divided arterial road connecting Hwy 96 to S Wakefield Road. The development satisfies both requirements.

Play Zebulon: Parks and Recreation Master Plan (Park Master Plan):

The Park Master Plan does identify a greenways in this general vicinity. The applicant has proposed the addition of the greenway through the site. They are working with Staff to make sure that it aligns with the Parks Master Plan.

Unified Development Ordinance (UDO):

The UDO (Section 2.2.13) allows flexibility from some standards in exchange for a higher quality development more aggressively accomplishing other goals, such as amenities and diverse housing. The applicant proposes a highly amenitized mixed-product residential neighborhood with multiple attached, and detached home options providing a broader range of housing values.

Fiscal Analysis:

When complete, this development will generate approximately \$405,000 per year in property tax revenue.

Staff Recommendation:

Staff recommends the Planning Board recommend approval the proposed planned development request as amended by the applicant, finding that the request is consistent with the Standards of Section 2.2.25.J of the UDO and the Comprehensive Land Use Plan.

Attachments:

- 1. Application
- 2. Site Plan
- 3. Planned Development Narrative
- 4. Utility Allocation Worksheet
- 5. TIA Review Letter from Town Engineer
- 6. TIA Applicant Response
- 7. TIA
- 8. Future Land Use and Character Map
- 9. Aerial Map
- 10. Zoning Map
- 11. Site Pictures
- 12. Public Hearing Notice Affidavit



STAFF REPORT PLANNED DEVELOPMENT 2024-01 **ZEBULON SOUTH** MARCH 11, 2024

- 13. UDO Section 3.5.5 Planned Development14. Comprehensive Land Use Plan (Excerpts)
- 15. Comprehensive Transportation Plan (Excerpts)



Town of Zebulon

Planning Department

1003 N. Arendell Avenue, Zebulon, NC 27597 Phone: (919) 823-1810 Fax: (919) 887-2824 www.townofzebulon.org

PLANNED DEVELOPMENT APPLICATION

GENERAL INFORMATION:

A Planned Development in accordance with Section 2.2.13 and 3.5.5 of the UDO is intended to provide flexibility by establishing site specific regulations including permitted uses, dimensional standards, phasing schedules and additional details to allow for a development that is better than what would otherwise be permitted under the strict interpretation of the UDO. All site-specific standards and conditions must be consistent with the objectives of these regulations, the adopted Comprehensive Land Use Plan, Transportation Plan, and Vision 2030 Strategic Plan. The review process established in this part provides for the accommodation of such uses by a reclassification of property into a Planned Development, subject to site-specific standards and conditions.

INSTRUCTIONS:

PRE-APPLICATION MEETING: A pre-application meeting with staff in accordance with Section 2.3.2 of the UDO to verify the application requirements, processes, and procedures regarding a proposed request. To schedule a meeting, applicants must e-mail a pdf map, drawing, model, site or sketch plan to Assistant Planning Director Meade Bradshaw (<u>mbradshaw@TownofZebulon.org</u>) no later than five (5) working days prior to the desired meeting day.

NEIGHBORHOOD MEETING: Neighborhood meetings are required in accordance with Section 2.3.4 of the UDO prior to application submission. The applicant is required to notify property owners and any neighborhood association that represents citizens within that area within 300 feet of the subject property via first class mail a minimum of 10 days in advance of the neighborhood meeting. The applicant shall use their own return address on the envelopes as the meeting is a private meeting between the developer and the neighbors. The applicant shall submit the "Certified List of Property Owners" and "Neighborhood Meeting Packet" forms included in this application packet with their initial submittal.

ANNEXATION REQUIREMENTS: If a property or portion thereof subject to this rezoning is outside the corporate limits and ETJ, an annexation petition is **required** to be submitted on the same day as this application in accordance with section 2.2.2 of the UDO.

APPLICATION PROCEDURE – The applicant requesting a Planned Development must submit a written application to the Zebulon Planning Department using the forms included in this packet.

- Completed Application Form
- 8 Full Size Plan Sets and 1 PDF set on USB drive. (see site plan checklist)
- Comprehensive Planned Development Document
- Petition Fee (Please See Fee Schedule)
- One (1) Legal Description (metes and bounds) of subject property
- Registered survey of subject property
- Certified List of Property Owners within 150 feet of subject property

- Owner's Consent Form
- Neighborhood Meeting Packet
- Stamped envelopes addressed to Certified List of Property Owners all the homeowners associations of those properties within 150 feet of the outer boundary subject property or properties affixed with the following return address:

Town of Zebulon Planning Department 1003 N. Arendell Ave Zebulon, NC 27597 Page 21



PUBLIC HEARING PROCEDURE – Upon submittal of a complete application, the Planning Department will schedule the application for a joint public hearing before the Planning Board and the Board of Commissioners. APPLICANTS ARE STRONGLY ENCOURAGED TO CONTACT PLANNING STAFF AS SOON AS POSSIBLE TO ADDRESS ANY QUESTIONS ABOUT THE PUBLIC HEARING. Notices of the public hearing will be mailed to all adjacent property owners of the property being considered for a Planned Development Amendment. At the public hearing, the applicant, proponents, and opponents will be given the opportunity to offer evidence in favor of or against the proposal. After completion of the public hearing, the Planning Board will deliberate and forward its recommendation to the Board of Commissioners for final consideration. Deadline dates and Joint Public Hearing dates can be found on the Town of Zebulon's website.



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| PART 1. DESCRIPTION OF REQUEST/PROPERTY Street Address of the Property: Acreage: | | | | |
| Street Address of the Property: | | | Acreage. | |
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| Parcel Identification Number (NC PIN): | | Deed Book: | Deed Page(s): | |
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| Existing Zoning of the Property: | | Proposed Zoning of the Property: | | |
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| Existing Use of the Property: | | Proposed Use of the Property: | | |
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| Reason for rezoning to a Planned Unit Development: | | | | |
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| PART 2. APPLICANT/AGENT INFORMA Name of Applicant/Agent: | 110 | JN | | |
| Name of Applicant/Agent. | | | | |
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| Street Address of Applicant/Agent: | | | | |
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| City: | | State: | Zip Code: | |
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| Email of Applicant/Agent: | | Telephone Number of Applicant/Agent: | Fax Number of Applic | ant/Agent: |
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| Are you the owner of the property? Are you the owner's agent? Note: If you are not the owner of the property, you must obtain the | | | | |
| \Box Yes \Box No \Box Yes \Box No | | | | |
| | NU | application. | | |
| PART 3. PROPERTY OWNER INFORMATION | | | | |
| Name of Property Owner: | | | | |
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| Street Address of Property Owner: | | | | |
| Succe Address of Hoperty Owner. | | | | |
| C'. | <i></i> | | 7.01 | |
| City: | State: | | Zip Code: | |
| Email of Property Owner: | Telen | hone Number of Property Owner: | Fax Number of Proper | ty Owner: |
| Linui of Hoperty Owner. | reiep | none runiber of rioperty Owner. | Tax Number of Troper | ty Owner. |
| | | | | |
| I hereby state that the facts related in this application and any documents submitted herewith are complete, true, | | | | |
| correct, and accurate to the best of my knowledge. | | | | |
| Signature of Applicant: | | Print Name: Date: | | Date: |
| | | | | 10 /21 /2022 |
| Andrew Suriano | | Andrew Suriano | | 10/31/2022 |
| Signature of Owner: | | Print Name: | | Date: |

Attachment 1 PD-2024-01

> PIN 2704492511 751 S Wakefield St DB 3452 PG 715 Narron, Harold Corbett, C Fred 3941 Zebulon Rd Zebulon, NC 27597 Email: Phone:

PIN 2705512202 700 S Arendell Ave DB 8545 PG 1076 Temple, Joseph Wood Sr Hughes, Harrison Alexander PO Box 548 Zebulon, NC 27597-0548 Email: Phone:

PIN 2705413075 O S Wakefield St DB 8099 PG 2738 Watson Family II LLC 6220 Forestville Rd Raleigh, NC 27604 Email: Phone:



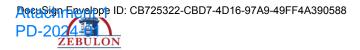
LEGISLATIVE CONSIDERATIONS – PLANNED DEVELOPMENT

The applicant shall propose site-specific standards and conditions that take into account the following considerations, which are considerations that are relevant to the legislative determination of whether or not the proposed planned development is in the public interest. Therese considerations do not exclude the legislative consideration of any other factor that is relevant to the public interest. Failure to adequately address the findings below may result in denial of the application. Please provide responses to the following standards as outlined in Section 2.2.13 of the Unified Development Ordinance.

| 1 | Please provide details on how the proposed Planned Development advances the public health, safety, or welfare |
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| 2. | Please provide details on how the proposed Planned Development is appropriate for its proposed location, and is consistent with |
| ۷. | the purposes, goals, objectives, and policies of the Town's adopted policy guidance. |
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| 3. | Please provide details on how the proposed Planned Development is reasonable and in the public interest. |
| 5. | Thease provide details on now the proposed Flaimed Development is reasonable and in the public interest. |
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| 4. | Please provide details on how the proposed Planned Unit Development provides for innovative land planning and site design |
| •• | concepts that support a high quality of life and achieve a high quality of development, environmental sensitivity, energy |
| | efficiency, and other Town goals and objectives. |
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| 5. | Please provide details on how the proposed planned unit development provides improved means of access, open space, and |
| 5. | design amenities; |
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| 6. | Please provide details on how the proposed Planned Unit Development provides a well-integrated mix of residential and |
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| | nonresidential land uses in the same development, including a mix of housing types, lot sizes, and densities; |
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| 7. | Please provide details on how the proposed Planned Unit Development creates a system of incentives for redevelopment and infill in order to revitalize established areas; |
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| 8. | Please provide details on how the proposed Planned Unit Development promotes a vibrant public realm by placing increased |
| | emphasis on active ground floor uses, pedestrian-oriented building façade design, intensive use of sidewalks, and establishment of public gathering areas; |
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| 9. | Please provide details on how the proposed Planned Unit Development provides for efficient use of land resulting in smaller |
| | networks of utilities and streets and thereby lowering development and housing costs; and |
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| 10. | Please provide details on how the proposed Planned Unit Development provides quality design and environmentally sensitive |
| | development that respects surrounding established land use character and respects and takes advantage of a site's natural and man-made features, such as trees, estuaries, shorelines, special flood hazard area, and historic features. |
| | man-made reatures, such as trees, estuaries, shorelines, special flood hazard area, and historic reatures. |
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| 11. | Other factors as the Board of Commissioners may determine to be relevant. |
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OWNER'S CONSENT FORM

Name of Project:

Zebulon South

Submittal Date:

11/01/2022

OWNER'S AUTHORIZATION

I hereby give CONSENT to Andrew Suriano, Deacon Development (type, stamp or print clearly full name of agent) to act on my behalf, to submit or have submitted this application and all required material and documents, and to attend and represent me at all meetings and public hearings pertaining to the application(s) indicated above. Furthermore, I hereby give consent to the party designated above to agree to all terms and conditions which may arise as part of the approval of this application.

I hereby certify I have full knowledge the property I have an ownership interest in is the subject of this application. I acknowledge and agree that, pursuant to Section 2.2.13. of the Town of Zebulon Unified Development Ordinance, that lands subject to a Planned Development shall be subject to all the standards, conditions, and plans approved as part of that application. These standards, plans, and approved conditions are perpetually binding on the land as an amendment to this Ordinance and the Official Zoning Map, and may only be changed in accordance with the procedures established in this Ordinance. Development located outside the Town of Zebulon's corporate limits shall comply with all Town policies related to annexation and the extension of utilities. I understand that all other applicable standards and regulations of the UDO will remain applicable to the subject lands unless specifically listed as conditions or deviations as part of this request. I understand that any false, inaccurate or incomplete information provided by me or my agent will result in the denial, revocation or administrative withdrawal of this application. I further consent to the Town of Zebulon to publish, copy or reproduce any copyrighted document submitted as a part of this application for any third party. I further agree to all terms and conditions, which may be imposed as part of the approval of this application.

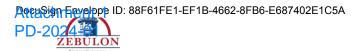
| goly water | Johnny Watson | 10/28/2022 11:22 AM PDT |
|--------------------|---------------|---------------------------|
| Signature of Owner | Print Name | Date |

CERTIFICATION OF PROPERTY OWNER

I hereby certify the statements or information made in any paper or plans submitted herewith are true and correct to the best of my knowledge. I understand this application, related material and all attachments become official records of the Planning Department of the Town of Zebulon, North Carolina, and will not be returned.

| Johnny Watson | | 10/28/2022 11:22 AM PDT |
|--------------------|------------|---------------------------|
| Signature of Owner | Print Name | Date |

*Owner of record as shown by the Wake County Revenue Department (<u>www.wakegov.com</u>). An option to purchase does not constitute ownership. If ownership has been recently transferred, a copy of the deed must accompany this form.



Temple

OWNER'S CONSENT FORM

Name of Project:

Zebulon South

Submittal Date:

11/01/2022

OWNER'S AUTHORIZATION

I hereby give CONSENT to Andrew Suriano, Deacon Development (type, stamp or print clearly full name of agent) to act on my behalf, to submit or have submitted this application and all required material and documents, and to attend and represent me at all meetings and public hearings pertaining to the application(s) indicated above. Furthermore, I hereby give consent to the party designated above to agree to all terms and conditions which may arise as part of the approval of this application.

I hereby certify I have full knowledge the property I have an ownership interest in is the subject of this application. I acknowledge and agree that, pursuant to Section 2.2.13. of the Town of Zebulon Unified Development Ordinance, that lands subject to a Planned Development shall be subject to all the standards, conditions, and plans approved as part of that application. These standards, plans, and approved conditions are perpetually binding on the land as an amendment to this Ordinance and the Official Zoning Map, and may only be changed in accordance with the procedures established in this Ordinance. Development located outside the Town of Zebulon's corporate limits shall comply with all Town policies related to annexation and the extension of utilities. I understand that all other applicable standards and regulations of the UDO will remain applicable to the subject lands unless specifically listed as conditions or deviations as part of this request. I understand that any false, inaccurate or incomplete information provided by me or my agent will result in the denial, revocation or administrative withdrawal of this application. I further consent to the Town of Zebulon to publish, copy or reproduce any copyrighted document submitted as a part of this application for any third party. I further agree to all terms and conditions, which may be imposed as part of the approval of this application.

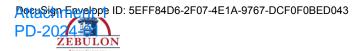
| Docusigned by: Joseph W timple, Jr. | Joseph W Temple, Jr. | 10/31/2022 8:20 AM PDT |
|--|------------------------|-------------------------------------|
| DocuSigned by: | Holly T Hughes Harriso | n Alexand@r⁄29u/g1023 7:28 AM CDT |
| Holly Thylus Hamson Illy ander tuylus Signature of Owner-DODTRAEGEE4487_ | Print Name | Date 10/31/2022 10:26 AM CDT |

CERTIFICATION OF PROPERTY OWNER

I hereby certify the statements or information made in any paper or plans submitted herewith are true and correct to the best of my knowledge. I understand this application, related material and all attachments become official records of the Planning Department of the Town of Zebulon. North Carolina, and will not be returned.

| Joseph W Temple, Jr. | Joseph w TempTe, | JI. 10/ J1/2022 0.20 AM PDI |
|--------------------------------------|------------------|--|
| 9855D8DF448048E DocuSigned by: | Holly T Hughes | Harrison Alexande ¹⁰ Hughe ²²² 7:28 AM CDT |
| tally thous thous Signature of Owner | Print Name | Date |
| | | 10/31/2022 10:26 AM CDT |

*Owner of record as shown by the Wake County Revenue Department (<u>www.wakegov.com</u>). An option to purchase does not constitute ownership. If ownership has been recently transferred, a copy of the deed must accompany this form.



Narron

OWNER'S CONSENT FORM

Name of Project:

Zebulon South

Submittal Date:

11/01/2022

OWNER'S AUTHORIZATION

I hereby give CONSENT to Andrew Suriano, Deacon Development (type, stamp or print clearly full name of agent) to act on my behalf, to submit or have submitted this application and all required material and documents, and to attend and represent me at all meetings and public hearings pertaining to the application(s) indicated above. Furthermore, I hereby give consent to the party designated above to agree to all terms and conditions which may arise as part of the approval of this application.

I hereby certify I have full knowledge the property I have an ownership interest in is the subject of this application. I acknowledge and agree that, pursuant to Section 2.2.13. of the Town of Zebulon Unified Development Ordinance, that lands subject to a Planned Development shall be subject to all the standards, conditions, and plans approved as part of that application. These standards, plans, and approved conditions are perpetually binding on the land as an amendment to this Ordinance and the Official Zoning Map, and may only be changed in accordance with the procedures established in this Ordinance. Development located outside the Town of Zebulon's corporate limits shall comply with all Town policies related to annexation and the extension of utilities. I understand that all other applicable standards and regulations of the UDO will remain applicable to the subject lands unless specifically listed as conditions or deviations as part of this request. I understand that any false, inaccurate or incomplete information provided by me or my agent will result in the denial, revocation or administrative withdrawal of this application. I further consent to the Town of Zebulon to publish, copy or reproduce any copyrighted document submitted as a part of this application for any third party. I further agree to all terms and conditions, which may be imposed as part of the approval of this application.

| Charles E. Corbett | Charles K. Corbett | 10/28/2022 10:57 AM PDT |
|--------------------|--------------------|---------------------------|
| SignaturesofiÖwner | Print Name | Date |

CERTIFICATION OF PROPERTY OWNER

I hereby certify the statements or information made in any paper or plans submitted herewith are true and correct to the best of my knowledge. I understand this application, related material and all attachments become official records of the Planning Department of the Town of Zebulon, North Carolina, and will not be returned.

| Charles te. Corbett | Charles K. Corbett | 10/28/2022 10:57 AM PDT |
|---------------------|--------------------|---------------------------|
| Signature of Owner | Print Name | Date |

*Owner of record as shown by the Wake County Revenue Department (<u>www.wakegov.com</u>). An option to purchase does not constitute ownership. If ownership has been recently transferred, a copy of the deed must accompany this form.



CONCEPT PLAN REQUIREMENTS

Every applicant requesting Planned Development approval shall submit 8 copies and 1 pdf (email or USB Drive) of a concept plan drawing with the application for a Planned Development. The concept plan shall contain sufficient information to adequately determine the type of development being proposed. The concept plan drawing shall include, at a minimum, the following features unless otherwise specified by the Planning Department:

CHECK IF SUBMITTED

ITEM

- 1. Plot plan showing all existing and planned structures, building setback lines, perimeter boundaries, and easements.
- 2. Elevation drawings of all buildings indicating the proposed exterior finish materials.
- 3. Landscaping plan, lighting, fencing, screening, and walls, indicating all heights and locations.
- 4. Location of all ingress and egress.
- 5. Off-street parking and loading facilities, with calculations showing how the quantities were obtained.
- 6. All pedestrian walks and open areas for use by residents, tenants, or the public.
- 7. Proposed land uses indicating areas in square feet.
- 8. The location and types of all signs, including lighting and heights, with elevation drawings.
- 9. Existing and/or proposed street names.
- 10. Proposed potable or reuse water, wastewater connections, and storm sewer line; proposed grading and drainage patterns; proposed water and sewer allocations.
- 11. Such additional items and conditions, including design standards as the Planning Board and Board of Commissioners deems necessary.
- 12. Trip generation data and TIA

✓ ✓ ✓ ✓ ✓ ✓ N/A ✓ ✓ ✓ ✓



PROPOSED USES

An application has been duly filed requesting that the property described in this application be rezoned from to ________. It is understood and acknowledged that if the property is rezoned as requested, the property described in this request will be perpetually bound to the use(s) authorized and subject to such conditions as imposed, unless subsequently changed or amended as provided for in the Unified Development Ordinance. It is further understood and acknowledged that final plans for any specific development to be made pursuant to any such Planned Development shall be submitted for site or subdivision plan approval. Use additional pages as needed.

The Rezoned Lands may be used for, and only for, the uses listed immediately below. The permitted uses are subject to the limitations and regulations stated in the Use Table and any additional limitations or regulations stated below. For convenience, some relevant sections of the Unified Development Ordinance may be referenced; such references do not imply that other sections of the Unified Development Ordinance do not apply.

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PROPOSED DEVELOPMENT CONDITIONS

The applicant hereby requests that the Zebulon Board of Commissioners, pursuant to Section 3.3.5 of the Unified Development Ordinance, approve the Proposed Planned Development with above listed use(s), subject to the following condition(s), requested deviations, and proposed alternative means of compliance. (Attach additional pages as needed)





ADJACENT OWNERS

Provide a certified list of property owners subject to this application and all properties owners within 150-feet feet of the subject property, and any HOA Contacts for developments which fall within 300-feet of the subject property.

| Parcel Address | Parcel ID Number | Owner's Name |
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HOA CONTACTS

| Development Name | Contact Person | Address |
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ADJACENT OWNERS

Provide a certified list of property owners subject to this application and all properties owners within 150-feet feet of the subject property, and any HOA Contacts for developments which fall within 300-feet of the subject property.

| Parcel Address | Parcel ID Number | Owner's Name |
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HOA CONTACTS

| Development Name | Contact Person | Address |
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Beginning at an existing iron pipe along the eastern right of way of Pulley Gordon Road, having a North Carolina State Plane Coordinate (NAD 1983-2011) value of North 749044.53 feet, East 2203638.58 feet. Thence S89° 06' 44"W, 43.52' to a point in the centerline of Pulley Gordon Road; thence with the centerline of Pulley Gordon Road N31° 40' 13"W, 17.82' to a point; thence with a curve to the right having a radius of 745.63', a length of 258.84', and a chord bearing and distance of N21° 43' 32"W, 257.54' to a point; thence N11° 46' 51"W, 141.30' to a point in the centerline of South Wakefield Street; thence with the centerline of South Wakefield Street with a curve to the left having a radius of 898.66', a length of 224.11', and a chord bearing and distance of N1° 14' 44"E, 223.53' to a point; thence N5° 53' 55"W, 188.25' to a point; thence with a curve to the right having a radius of 2330.34', a length of 135.93', and a chord bearing and distance of N4° 13' 39"W, 135.91' to a point; thence N2° 33' 23"W, 47.94' to a point; thence with a curve to the right having a radius of 1695.94', a length of 104.97', and a chord bearing and distance of N0° 47' 00"W, 104.96' to a point; thence with a curve to the left having a radius of 4451.25', a length of 133.35', and a chord bearing and distance of N0° 07' 54"E, 133.35' to a point; thence N0° 43' 36"W, 120.66' to a point; thence leaving the centerline of the aforesaid road S88° 41' 25"E, 331.00' to an existing iron pipe; thence N0° 50' 44"W, 407.57' to an existing iron pipe; thence N89° 19' 57"W, 330.04' to a point in the centerline of South Wakefield Street; thence with the centerline of South Wakefield Street N0° 43' 36"W, 59.99' to a point; thence leaving the aforesaid centerline S89° 19' 57"E, 330.02' to an existing iron pipe; thence N0° 44' 29"W, 389.63' to an existing iron pipe; thence N89° 53' 37"W, 129.83' to an existing iron pipe; thence N3° 52' 53"E, 233.74' to an existing iron pipe; thence S89° 15' 00"E, 1384.54' to an existing iron pipe; thence N1° 20' 40"E, 480.67' to an existing iron pipe; thence N78° 36' 17"E, 124.17' to an existing iron pipe; thence continuing N78° 36' 17"E, 30.59' to a point in the centerline of South Arendell Avenue; thence with the centerline of South Arendell Avenue S28° 50' 24"E, 761.08' to a point; thence S28° 50' 24"E, 425.02' to a point; thence S29° 13' 43"E, 667.05' to a point; thence leaving the centerline of the aforesaid road N88° 26' 36"W, 584.64' to an existing iron pipe; thence S89° 11' 28"W, 68.13' to a 5/8" capped iron rod set; thence S2° 12' 27"E, 1759.55' to a 5/8" capped iron rod set; thence S88° 17' 33"W, 346.50' to an existing iron pipe; thence N3° 17' 33"E, 478.27' to an existing iron pipe; thence S89° 06' 44"W, 1540.50' to the Beginning, containing 118.61 acres more, or less.

SAVE AND EXCEPT the cemetery that exist on the above-described property, described as Beginning at the southeast corner of the cemetery, having a North Carolina State Plane Coordinate (NAD 1983-2011) value as North 751140.59', East 2205340.06'. Thence N90° 00' 00"W, 30.56' to a point; thence N0° 00' 00"E, 32.27' to a point; thence N90° 00' 00"E, 30.56' to a point; thence S0° 00' 00"E, 32.27' to the Beginning, containing 986 square feet more, or less.

THIS DESCRIPTION IS PROVIDED WITHOUT THE BENEFIT OF A TITLE COMMIMENT



5410 Trinity Road Suite 102 Raleigh, NC 27607

P 919.866.4951 F 919.859.5663 www.timmons.com

September 22, 2022

Notice of Proposed Zoning Change

Wake County PINs 2705-41-3075, 2704-49-2511, & 2705-51-2202

Dear Property Owner:

On behalf of the applicant and property owners, Timmons Group would like to invite you to attend a neighborhood information meeting concerning the following proposal. Timmons Group will be submitting a request to rezone the property located between the intersections of S Wakefield Street and Morphus Bridge Road and the intersection at S Arendell Ave and Perry Curtis Road. The parcels under consideration are shown on the attached map.

The existing zoning is Zebulon Residential Neighborhood District (R4) & Residential Suburban District (R2), and the proposed zoning classification requested is Zebulon Planned Development District (PD). The proposed rezoning will not change the existing zoning status of surrounding properties. The proposed development will consist of a variety of lot sizes, including attached and detached single family.

Per Town of Zebulon ordinance requirements, we are notifying you of this meeting because your property is located within the written notification area for public hearings. While this meeting is not a public hearing, it is an opportunity for you to meet with the owners and/or applicants to hear about their intention to rezone the land. You are encouraged to ask questions and express concerns so that we may help you to understand the proposed project more fully. There will not be an in-person meeting.

The meeting participation options are as follows:

- 1. An online virtual meeting to be held on October 05, 2022 at 6:00 pm.
 - a) Virtual meeting link: https://timmons.zoom.us/j/99067768952?pwd=MGU0WGRRampkMWgvOGhOYV grUEgzQT09
 - b) Password: **942736**
 - c) Instructions: You may join from any browser. Upon joining, you be placed in the waiting room until the meeting host allows entrance. At the start of the meeting, we will take some time to gather the required information (Name, Address, Email and Phone number) though the chat feature on screen.

- 2. A toll-free conference call for audio only access to the virtual meeting held on October 05, 2022 at 6:00 pm.
 - a) Call about five (5) minutes prior to the aforementioned date and time:
 - Phone Number: +1 646 558 8656
 - b) You may be asked to dial the following information:
 - Meeting ID: 990 6776 8952
 - Password: **942736**

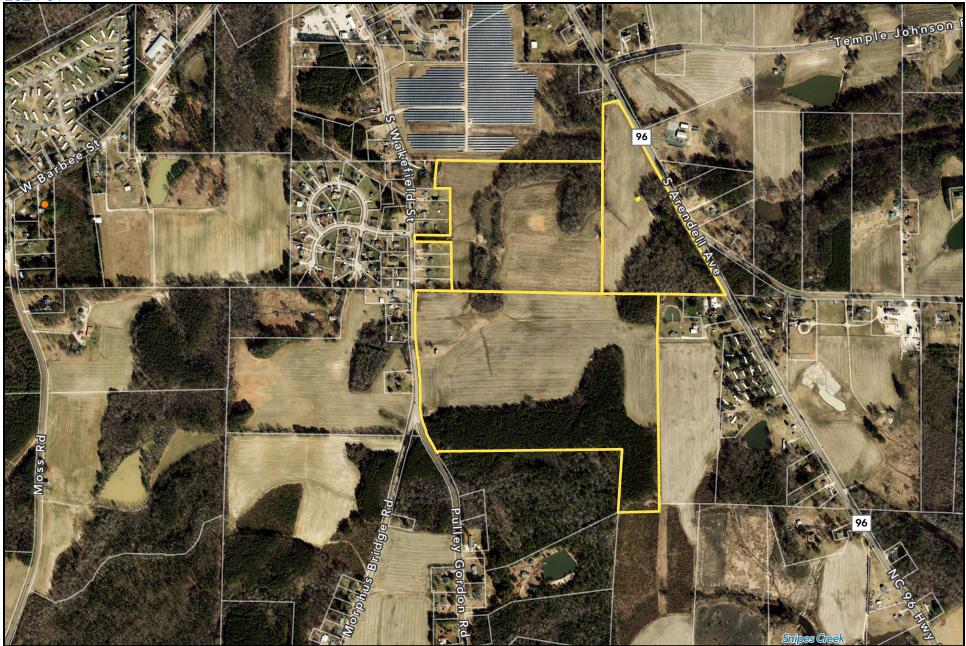
If you have any questions about this neighborhood information meeting, or if you are unable to attend and would like to leave comments for our consideration, please feel free to contact me at 919-866-4509 or <u>beth.blackmon@timmons.com</u>.

Thank you for your interest.

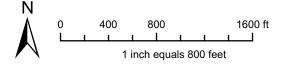
Sincerely,

Beth Blackpm

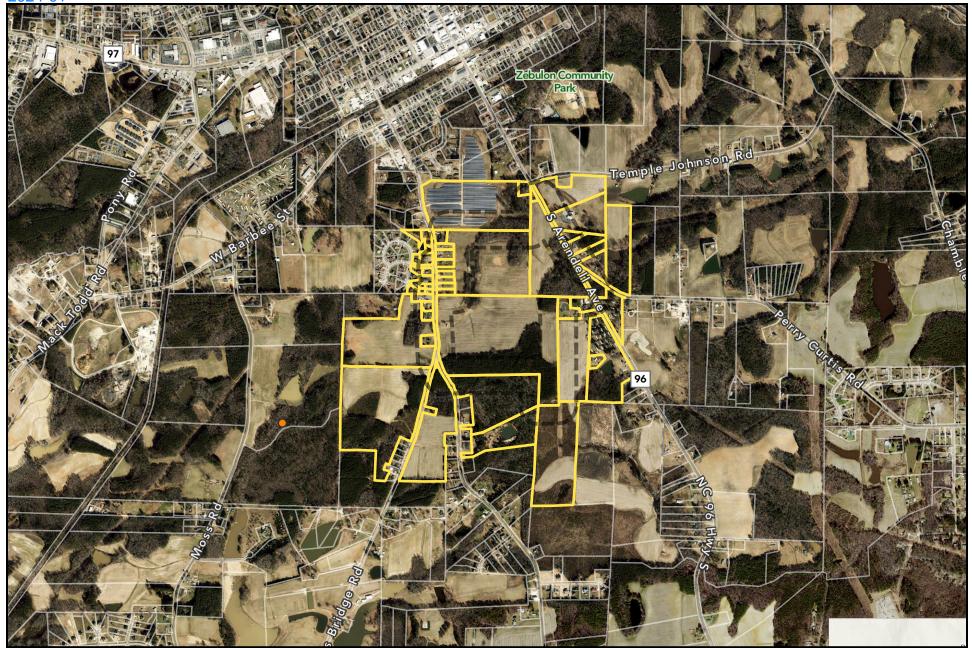
Beth Blackmon, PE Sr. Project Manager



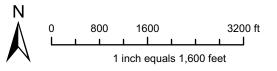
Zebulon South Neighborhood Meeting



<u>Disclaimer</u> iMaps makes every effort to produce and publish the most current and accurate information possible. However, the maps are produced for information purposes, and are **NOT** surveys. No warranties, expressed or implied ,are provided for the data therein, its use,or its interpretation.



Zebulon South Mailing List



<u>Disclaimer</u> iMaps makes every effort to produce and publish the most current and accurate information possible. However, the maps are produced for information purposes, and are **NOT** surveys. No warranties, expressed or implied ,are provided for the data therein, its use, or its interpretation.

ΡI

| IN_NUM | OWNER | ADDR1 | ADDR2 |
|------------|---|------------------------------|---------------------|
| | AGARWAL ASSOCIATES LLC | 2000 KILLEARN MILL CT | CARY NC 27513-4293 |
| 2705303716 | BARRERA, EMILIO VILLEGAS ARMENTA, BE | 301 SIR DAVID DR | ZEBULON NC 27597-0 |
| | BLOUNT, BARBARA ANN | 707 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2705306724 | BRADSHAW, BETTIE SUE | 697 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2705302989 | C W S SYSTEMS INC | 500 W MONROE ST STE 3600 | CHICAGO IL 60661-37 |
| 2704392692 | CARRILLO, JOSE SANTOS CARRILLO, MARY | 812 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| | CHURCH OF GOD EASTERN NC STATE OFF | | KENLY NC 27542-010 |
| 2705306922 | CONYERS, BEVERLY A CONYERS, CASSAND | 631 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2705520074 | CREECH, ROBERT E CREECH, KATHEY P | 13713 POSSUM TRACK RD | RALEIGH NC 27614-9 |
| 2705311337 | DAVID BULLOCK CONSTRUCTION INC | 2805 AUBURN KNIGHTDALE RD | RALEIGH NC 27610-9 |
| 2705429117 | DEAN, ATWELL STUART | 604 S ARENDELL AVE | ZEBULON NC 27597-8 |
| 2705601533 | ESTRADA, JILBER VELAZQUEZ | 106 WOODGATE DR | GARNER NC 27529-2 |
| 2705601920 | FOX, JEFFERY M FOX, PENNY M | 753 S ARENDELL AVE | ZEBULON NC 27597-8 |
| 2705301515 | FUENTES, BAYRON JOSUE LOPEZ HILARIO, | 709 CHANCE CIR | ZEBULON NC 27597-6 |
| 2705316027 | GARCIA, LAURA A REYES, GERARDO REYES | 621 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2705302523 | GILL, W E | PO BOX 474 | ZEBULON NC 27597-0 |
| | GILL, W E GILL, GENEVIEVE M | PO BOX 474 | ZEBULON NC 27597-0 |
| 2705302424 | GILL, WILLIAM E GILL, GENEVIEVE M | PO BOX 474 | ZEBULON NC 27597-0 |
| 2705303434 | GILL, WILLIAM E GILL, GENEVIEVE M | PO BOX 474 | ZEBULON NC 27597-0 |
| 2705301927 | HANNAH, KRYSTAL | 304 SIR DAVID DR | ZEBULON NC 27597-6 |
| 2705302674 | HERNANDEZ, LYDIA FABIOLA MATEO | 610 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2705302314 | HICKS, MELVILLE HOWARD JR | PO BOX 660 | ZEBULON NC 27597-0 |
| 2705610110 | HILL, TIMOTHY GORDON HILL, LILLIAN AV | 745 S ARENDELL AVE | ZEBULON NC 27597-8 |
| 2705306404 | HINTON, MARY E HEIRS HOLDER, MARY A | 709 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2705518284 | HOLLAND, JIMMY LEON HOLLAND, HELEN | 737 S ARENDELL AVE | ZEBULON NC 27597-8 |
| 2704693410 | JAYS ARENDELL PROPERTIES LLC | 2709 BELMONT VIEW LOOP | CARY NC 27519-7725 |
| 2705506134 | LONG, RANDALL S | 908 S ARENDELL AVE | ZEBULON NC 27597-8 |
| 2704485074 | MARTIN, COY BERKLEY SR MARTIN, COY B | 2202 NC 561 HWY | LOUISBURG NC 2754 |
| 2705316119 | MCCULLERS, JAMIE | 611 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2705526056 | MCNABB, INEZ PITTS HEIRS | 1900 LITTLE ELM TRL APT 70 | CEDAR PARK TX 7861 |
| 2704492511 | NARRON, HAROLD CORBETT, C FRED | 3941 ZEBULON RD | ZEBULON NC 27597-8 |
| 2705509203 | PARKER, LARRY N | 900 S ARENDELL AVE | ZEBULON NC 27597-8 |
| 2704597445 | PARKER, LARRY N PARKER, TAMMY M | 900 S ARENDELL AVE | ZEBULON NC 27597-8 |
| 2704574734 | PARKER, LARRY N PARKER, TAMMY M | 900 S ARENDELL AVE | ZEBULON NC 27597-8 |
| 2704287413 | RAPER, F WAYNE TRUSTEE RAPER, JEAN D | 1317 MORPHUS BRIDGE RD | WENDELL NC 27591- |
| 2705312399 | ROSSMAN, MAXINE | 108 REGGIE OWENS DR | HARBINGER NC 2794 |
| 2705315336 | SANTOS, ALEJANDRO WILIBALDO ROSALE | PO BOX 332 | WENDELL NC 27591- |
| 2705303119 | SHAW, SHIRLEY D | 738 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2704297696 | SILBER, EVA TRUSTEE EVA SILBER LIVING T | 5117 MELBOURNE RD | RALEIGH NC 27606-1 |
| 2705301759 | SPRUILL, JOSEPH PAUL SPRUILL, CONNIE S | 305 SIR DAVID DR | ZEBULON NC 27597-0 |
| 2705305694 | STANCIL, L J | 701 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2705513114 | TEMPLE, J M | PO BOX 548 | ZEBULON NC 27597-0 |
| 2705614179 | TEMPLE, JOSEPH WOOD | PO BOX 548 | ZEBULON NC 27597-0 |
| 2705512202 | TEMPLE, JOSEPH WOOD SR HUGHES, HAR | PO BOX 548 | ZEBULON NC 27597-0 |
| 2705302076 | TISDALE, ALICE KIRK DUNN, MARY FRANC | 748 S WAKEFIELD ST | ZEBULON NC 27597-2 |
| 2705300426 | VILLAFRANCA, IRIS | 713 CHANCE CIR | ZEBULON NC 27597-0 |
| 2705516356 | VILLALPANDO, MIGUEL ANGEL | 110 LEGEND VALLEY LN UNIT 13 | ZEBULON NC 27597-9 |
| 2705410911 | VINSON, MARTHA H | 500 PERRY CURTIS RD | ZEBULON NC 27597-8 |
| 2705413075 | WATSON FAMILY II LLC | 6220 FORESTVILLE RD | RALEIGH NC 27604-8 |
| 2705520074 | Current Resident | 614 S ARENDELL AVE | ZEBULON NC 27597 |
| 2705303532 | Current Resident | 720 S WAKEFIELD ST | ZEBULON NC 27597 |
| 2705303434 | Current Resident | 728 S WAKEFIELD ST | ZEBULON NC 27597 |
| 2705302314 | Current Resident | 734 S WAKEFIELD ST | ZEBULON NC 27597 |
| 2704485074 | Current Resident | 1131 PULLEY GORDON RD | ZEBULON NC 27597 |
| 2705312399 | Current Resident | 600 S WAKEFIELD ST | ZEBULON NC 27597 |
| | | | |

ADDR2 CARY NC 27513-4293 ZEBULON NC 27597-6801 ZEBULON NC 27597-2567 ZEBULON NC 27597-2565 CHICAGO IL 60661-3779 ZEBULON NC 27597-2568 KENLY NC 27542-0100 ZEBULON NC 27597-2565 RALEIGH NC 27614-9381 RALEIGH NC 27610-9712 ZEBULON NC 27597-8202 GARNER NC 27529-2738 ZEBULON NC 27597-8205 ZEBULON NC 27597-6809 ZEBULON NC 27597-2565 ZEBULON NC 27597-0474 ZEBULON NC 27597-0474 ZEBULON NC 27597-0474 ZEBULON NC 27597-0474 ZEBULON NC 27597-6800 ZEBULON NC 27597-2564 ZEBULON NC 27597-0660 ZEBULON NC 27597-8205 ZEBULON NC 27597-2567 ZEBULON NC 27597-8205 CARY NC 27519-7725 ZEBULON NC 27597-8208 OUISBURG NC 27549-8469 ZEBULON NC 27597-2565 CEDAR PARK TX 78613-2834 ZEBULON NC 27597-8187 ZEBULON NC 27597-8208 ZEBULON NC 27597-8208 ZEBULON NC 27597-8208 WENDELL NC 27591-8377 HARBINGER NC 27941-9704 WENDELL NC 27591-0332 ZEBULON NC 27597-2566 RALEIGH NC 27606-1747 ZEBULON NC 27597-6801 ZEBULON NC 27597-2567 ZEBULON NC 27597-0548 ZEBULON NC 27597-0548 ZEBULON NC 27597-0548 ZEBULON NC 27597-2566 ZEBULON NC 27597-6809 ZEBULON NC 27597-9503 ZEBULON NC 27597-8877 RALEIGH NC 27604-8618 ZEBULON NC 27597 ZEBULON NC 27597 ZEBULON NC 27597 ZEBULON NC 27597 ZEBULON NC 27597

2705315336 Current Resident **Current Resident** Current Resident **Current Resident** Current Resident **Current Resident Current Resident Current Resident** Current Resident Current Resident **Current Resident** Current Resident **Current Resident** Current Resident **Current Resident Current Resident** Current Resident Current Resident **Current Resident** Current Resident Current Resident Current Resident Current Resident Current Resident **Current Resident Current Resident Current Resident** Current Resident Current Resident **Current Resident Current Resident Current Resident** Current Resident Current Resident **Current Resident** Current Resident **Current Resident** Current Resident **Current Resident Current Resident Current Resident Current Resident** Current Resident Current Resident **Current Resident Current Resident** Current Resident **Current Resident** Current Resident **Current Resident Current Resident Current Resident Current Resident** Current Resident **Current Resident Current Resident** 601 S WAKEFIELD ST 100 Bingo Blvd 100 Bingo Blved LT 27 100 Green Grove Ln 100 Green Grove Ln LT 5 100 Legend Valley Ln 100 Legend Valley Ln LT 12 100 Long Park Dr 100 Long Park Dr LOT 1 100 Rocky Road Dr 100 Rocky Road Dr LT 19 100 Royal View Dr 100 Royal View Dr LT 34 1007 S Arendell Ave 1014 S Arendell Ave 1020 S Arendell Ave 1028 S Arendell Ave 105 Rocky Road Dr 105 Rocky Road Dr LT 15 106 Long Park Dr 106 Long Park Dr LT 11 108 Long Park Dr 108 Long Park Dr LT 10 110 Bingo Blvd 110 Bingo Blvd LT 28 110 Green Grove Ln 110 Green Grove Ln LT 6 110 Legend Valley Ln LT 13 110 Long Park Dr 110 Long Park Dr LT 9 110 Rocky Road Dr 111 Rocky Road Dr LT 20 110 Royal View Dr 110 Royal View Dr LT 23 1100 S Arendell Ave 115 Rocky Road Dr 115 Rocky Road Dr LT 16 120 Bingo Blvd 120 Bingo Blvd LT 29 120 Legend Valley Ln 120 Legend Valley Ln LT 14 120 Long Park Dr 120 Long Park Dr LT 8 120 Rocky Road Dr 120 Rocky Road Dr LT 21 120 Royal View Dr 120 Royal View Dr LT 24 125 Legend Valley Ln 125 Legend Valley Ln LT 33 125 Rocky Road Dr 125 Rocky Road Dr LT 17 130 Bingo Blvd 130 Bingo Blvd LT 30 130 Long Park Dr 130 Long Park Dr LT 7 130 Rocky Road Dr

ZEBULON NC 27597 ZEBULON NC 27597 ZEBULON NC 27597 **ZEBULON NC 27597 ZEBULON NC 27597** ZEBULON NC 27597 **ZEBULON NC 27597 ZEBULON NC 27597 ZEBULON NC 27597 ZEBULON NC 27597** ZEBULON NC 27597 **ZEBULON NC 27597 ZEBULON NC 27597** ZEBULON NC 27597 **ZEBULON NC 27597 ZEBULON NC 27597**

Current Resident 130 Rocky Road Dr LT 22
130 Royal View Dr
130 Royal View Dr LT 25
135 Rocky Road Dr
135 Rocky Road Dr LT 18
140 Bingo Blvd
140 Bingo Blvd LT 31
140 Royal View Dr
150 Bingo Blvd LT 32
805 S Arendell Ave
805 S Arendell Ave

ZEBULON NC 27597 ZEBULON NC 27597

NORTH CAROLINA WAKE COUNTY

AFFIDAVIT OF MAILING

I, Elizabeth Ange, Project Engineer III with Timmons Group, being first duly sworn, deposes and says as follows:

1. That I am a project engineer regarding a rezoning petition to be filed with the Town of Zebulon (the "Town"), for a project known as "Zebulon South" (the "Project").

2. In accordance with the Town's Unified Development Ordinance (the "UDO"), a Neighborhood Meeting for the Project was scheduled for and did occur on October 5, 2022.

3. In accordance with the Town's UDO, a notice of the Neighborhood Meeting was mailed to those individuals and property addresses identified on the exhibit attached hereto. To the best of my knowledge, the individuals identified on the attached exhibit are all of the landowners and occupants within 300 linear feet of the outer perimeter of the site where the Project is proposed.

4. The notice of the Neighborhood Meeting was mailed no less than ten days prior to the date of the Neighborhood Meeting.

This the <u>22</u> day of <u>September</u>, 2022. beth Ange

Project Engineer III Timmons Group 5410 Trinity Road, Suite 102 Raleigh, NC 27607

NORTH CAROLINA COUNTY OF Wake

BEFORE ME, the undersigned authority, this day personally appeared Elizabeth Ange, who, being first duly sworn, deposes and says that she has read the foregoing Affidavit of Mailing and knows the facts contained therein to be true and correct to the best of her knowledge and belief.

| SWORN TO AND SUBSCRIBED before me this | Oct. 12, | 2022. | |
|--|-----------------|--|------------------------|
| Horen & Lost | , Notary Public | ۰۰ ۲ | |
| RIGINI | | (SEA) | L) |
| My commission expires: $8/8/24$ | _ | Karen L. I NOTARY PU Wake Con North Care My Commission Expires | JBLIC unty olina |
| | | 202122500 | 189 |





TIMMONS GROUP

YOUR VISION ACHIEVED THROUGH OURS.

| Meeting Sign-in | Sheet | | | |
|---------------------|---|---------------|--------------------------------------|----------------------|
| Project: | Zebulon South | Meeting Date: | 5-Oct-22 | 2 |
| Faciliator: | Timmons Group | Place/ Room: | Zoom |] |
| Nomo | Address | Dhono | Email | Sent presentation |
| Name | Address | Phone | | to |
| Beth Blackmon | 5410 Trinity Rd, Suite 102, Raleigh NC 27607 | 919-866-4509 | beth.blackmon@timmons.com | |
| Elizabeth Ange | 5410 Trinity Rd, Suite 102, Raleigh NC 27607 | 984-255-2366 | elizabeth.ange@timmons.com | |
| John Adcock | PO Box 1478, Fuquay-Varina, NC 27526 | 919-552-6600 | john@adcocklawfirm.com | |
| Andrew Suriano | PO Box 1080, Wake Forest, NC 27588 | 919-608-3542 | andrew@deaconcompanies.com | |
| Jeff Hochanadel | 5410 Trinity Rd, Suite 102, Raleigh NC 27607 | 919-866-4511 | jeff.hochanadel@timmons.com | |
| Hunter Mullins | 5410 Trinity Rd, Suite 102, Raleigh NC 27607 | 919-532-3272 | hunter.mullins@timmons.com | |
| Lynn Mcnabb | 1900 Little Elm Trail Apt. 70, Cedar Park Tx | | mcnabbvolunteer1@aol.com | x |
| Tracie Hicks | PO Box 251, Pittsfield, ID | 919-868-7592 | tracie.hicks@whitetailproperties.com | |
| Jane Mccullers | 611 S Wakefield St, Zebulon NC 27597 | | jamie.mccullers@yahoo.com | x |
| Temple | PO Box 548, Zebulon NC 27597 | | | |
| Chuck, Fred Corbett | | | | |
| Apurva | | | JAYSArendellProperties@gmail.com | x |
| Jack Yen | | | jackyen@gmail.com | x |



YOUR VISION ACHIEVED THROUGH OURS.

| Project: | Zebulon South | Meeting Date: | Wednesday, October 5, 2022 |
|------------------------|--|---------------------|----------------------------|
| Applicant: | Timmons Group - Beth Blackmon | Place/ Room: | Zoom |
| Contact Information: | beth.blackmon@timmons.com 919-866-4509 | Time: | 6:00 PM |
| Questions/ Concern #1: | Is there a chance that I might have to mo | ove? | |
| Applicant Response: | No ma'am, your property is not on site ar property. Additionally, there is environme | nd we are not allov | |

Questions/ Concern #2: Are you going to build the thoroughfare road? That will be good for the town!

| Applicant Response: | Yes, because it shows on the transportation plan, it is required to be built. It is a 2 lane |
|---------------------|--|
| | divided road. |

Questions/ Concern #3: What's the plan for this site? Will there be a new development?

Applicant Response: There are 3 properties and the developer is going to submit a rezoning application to rezone to PD, Planned Development. The PD allows for an initiative design. As of right now, it is proposed to be a residential development with single family housing and townhomes. The proposed density is 2.67 du/a. 314 lots are proposed, 108 are rear load lots.



ZEBULON SOUTH NEIGHBORHOOD MEETING

TIMMONS^aGROUP

October 5, 2022

INTRODUCTIONS PURPOSE OF MEETING THE DEVELOPMENT PROCESS PROJECT INFORMATION LOCATION

AGENDA

CURRENT ZONING

FUTURE LAND USE MAP

COMPREHENSIVE TRANSPORTATION PLAN

EXISTING CONDITIONS

PD REZONING TIMELINE

Q & A



INTRODUCTIONS

TIMMONS GROUP:

Beth Blackmon, PE

Sr. Project Manager

9 | 9 - 8 6 6 - 4 5 0 9

beth.blackmon@timmons.com

ADCOCK LAW FIRM:

John Adcock, Esq.

9 | 9 - 5 5 2 - 6 6 0 0

john@adcocklawfirm.com

Attendees



PURPOSE

WHO RECEIVED NOTIFICATION?

Property owners within 300 feet of the proposal

WHY ARE WE HOLDING THIS MEETING?

Unified Development Ordinance requirement to meet with adjacent property owners

To have an opportunity before submittal to receive feedback

To improve the proposal with that feedback

HOW WILL WE DO THAT?

Following tonight's meeting, the applicant & Town staff will discuss your comments Look for ways to improve the proposal using your comments



THE DEVELOPMENT PROCESS

PLANNED DEVELOPMENT REZONING (PD)

Master plan rezoning with concept plan and conditions to guide future development

PRELIMINARY SUBDIVISION PLAT/CONSTRUCTION DOCUMENTS

Detailed subdivision plans to establish streets, lot layout and utilities Proposing single family detached homes and attached townhomes Both rear load and front load homes will be proposed Reviewed by staff for conformance with PD master plan rezoning

Detailed plans to be utilized for construction of infrastructure Including erosion control, road improvements, streets, utilities and stormwater

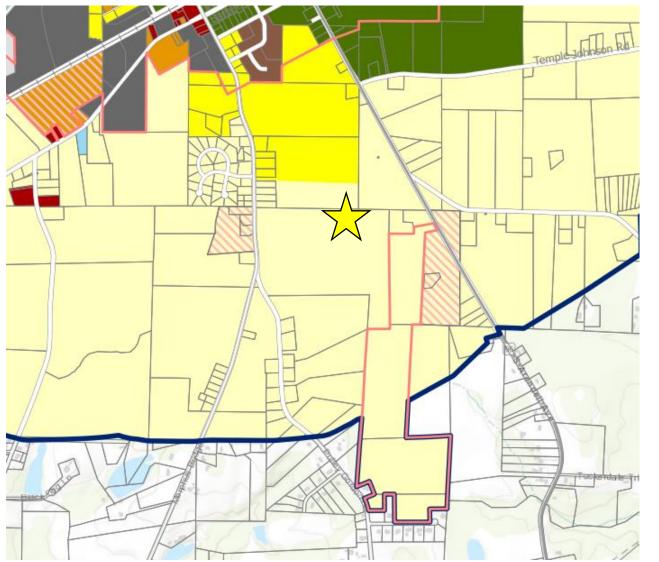


LOCATION



MONS®ROUP

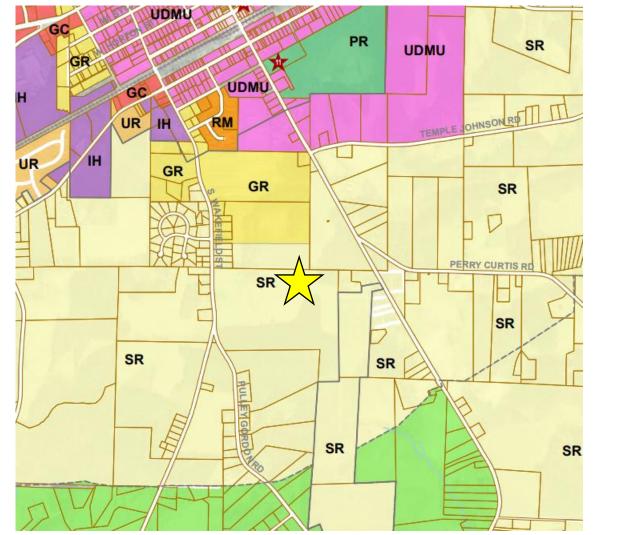
CURRENT ZONING



R2, RESIDENTIAL SUBURBAN R4, RESIDENTIAL NEIGHBORHOOD



FUTURE LAND USE MAP



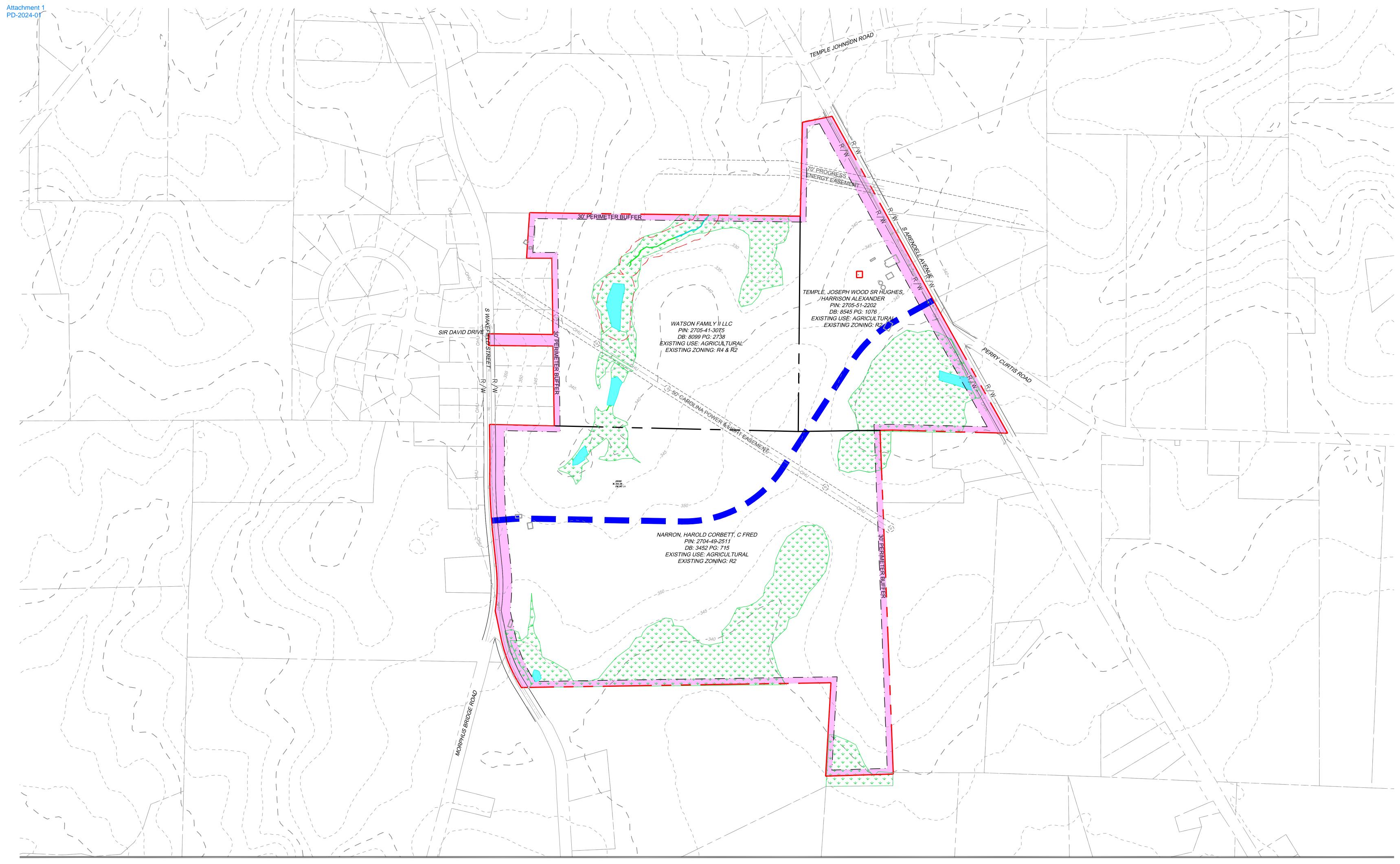
Suburban Residential (SR) General Residential (GR)



COMPREHENSIVE TRANSPORTATION PLAN

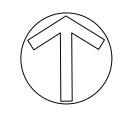


TIMMONS



ZEBULON SOUTH

Existing Conditions - October 4, 2022





Page 56

- -

PD REZONING TIMELINE

NEIGHBORHOOD MEETING DATE:

October 5, 2022

ANTICIPATED APPLICATION SUBMITTAL DATE:

November I, 2022

ANTICIPATED JOINT PUBLIC HEARING MEETING:

January 23, 2023

ANTICIPATED PLANNING BOARD RECOMMENDATION:

January 30, 2023

ANTICIPATED BOARD OF COMMISSIONERS DECISION:

February 6, 2023



Q & A

TIMMONS GROUP:

Beth Blackmon, PE

Sr Project Manager

9 | 9 - 8 6 6 - 4 5 0 9

beth.blackmon@timmons.com

ADCOCK LAW FIRM:

John Adcock, Esq.

9 | 9 - 5 5 2 - 6 6 0 0

john@adcocklawfirm.com

ZEBULON PLANNING DEPARTMENT CONTACT:

Michael Clark

Planning Director

9 | 9 - 8 2 8 - | 8 0 8

mclark@townofzebulon.org



Attachment 1 NS GROUP RALEIGH NC 275 Research Triangle Region 28 SEP 2022 PM 3 L **ROSSMAN, MAXINE 108 REGGIE OWENS DR** HARBINGER NC 27941-9704 Z76 NEE 1 42110210/03/22 TIME EXP RTN TO SEND FORWARD ROSSMAN 600 S W GCO S WA WAKEFIELD ST N NC 27597-2564 . A 93 93 93 93 82 89 92 8 9 88 8 INT RETURN TO SENDER 27597 > 5093

Returned Letter

Page 59

RALEIGH NC 275 Research Triangle Region 28 SEP 2022 PM 3 L



ROSSMAN, MAXINE 108 REGGIE OWENS DR HARBINGER NC 27941-9704

> 276 NEE 1 42110210/03/22 FORWARD TIME EXP RTN TO SEND G00 5 WAKEFIELD ST ZEBULON NC 27597-2564

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Attachment 1

INT 2759725003 27541-970408

RETURN TO SENDER

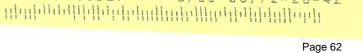
RALEIGH NC 275 Research Triangle Region 28 SEP 2022 PM 3 L

Attachment 1



Current Resident 110 Royal View Dr LT 23 **ZEBULON NC 27597** NIXIE 276 CE 1 0210/08/22 RETURN TO SENDER VACANT TO FORWARD UNABLE 2759×4556 27607>6093 * 8888-92677-28-41 28 SEP 2022 PM 3 1 TEMPLE, JOSEPH WOOD **PO BOX 548** ZEBULON NC 27597-0548 NIXIE 276 NEE 1 2210210/01/22 NOT DELIVERABLE AS ADDRESSED UNABLE TO FORWARD INT MANUAL FROC REQ * 0880-02599-28-41 2753756836640 28 SEP 2022 PM 3 뢽 TEMPLE, JOSEPH WOOD . HARRISON ALEXANDER PO BOX 548 ZEBULON NC 27597-0548 276 NCE 1 2200210/08/22 TURN VERA RE TO SENDER NOT TO FORWARD 1 UNABLE 1: 9400922 802812 FWD 8C: 27607600327 * 0880-03099-28-41

27607 > 6003



* 0780-00772-28-41

0210/05/22

308920860821988

HINTON, MARY E HEIRS HOLDER, MARY 709 S WAKEFIELD ST ZEBULON NC 27597-2567

NSS

2788752565709

NIXIE

APEX NC 27539-8759 0210/08/22 CE 1 276 NIXIE RETURN TO SENDER NO SUCH NUMBER FURWARD UNABLE TU * 0880-02774-03-43 2753 1759 1^{8 C1} 27 607 6003 27 27607>6003

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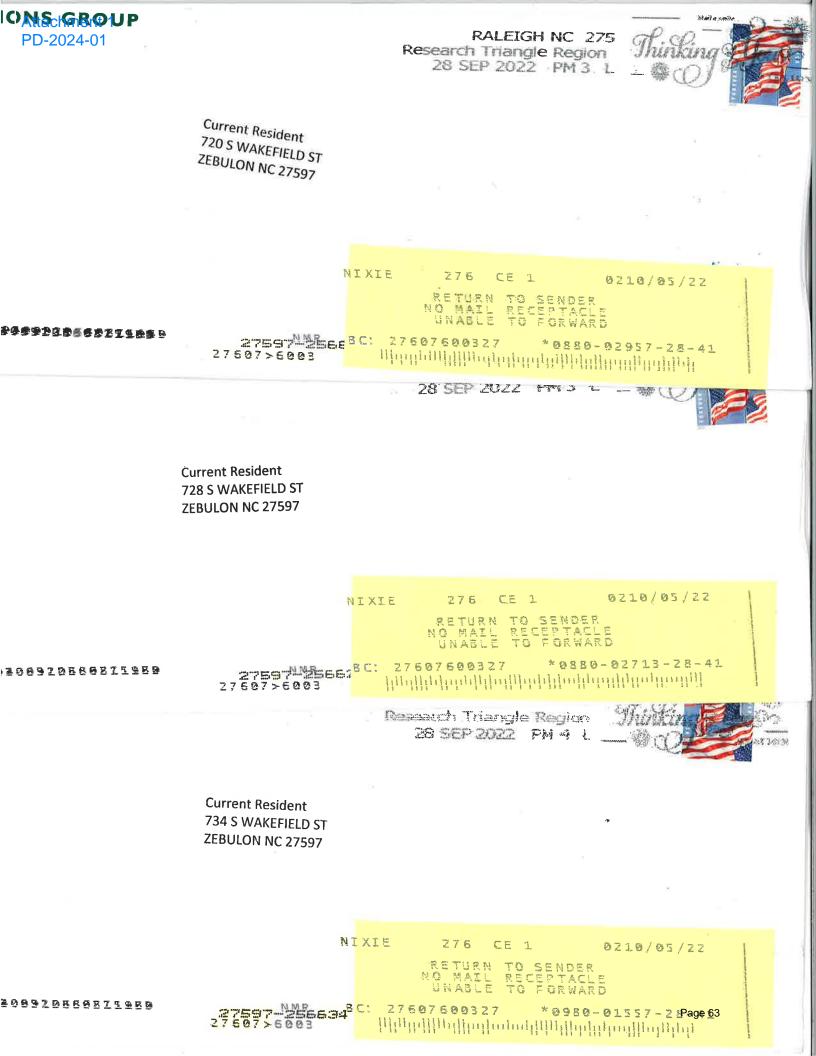
> **BEHNKE, DEBRA ANN** 4313 FRIENDSHIP RD

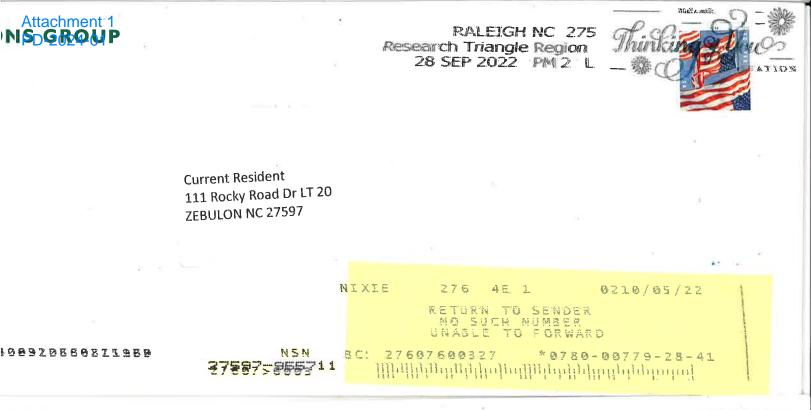
\1:94009227802812

TEMPLE, J M **PO BOX 548** ZEBULON NC 27597-0548

RALEIGH NC 275 Research Triangle Region 28 SEP 2022 PM 3 L

Attachment 1 PD-2024-01





ONS GROUP

RALEIGH NC 275 Research Triangle Region 16 SEP 2022 PM 3 L





RALEIGH NC 275 Research Triangle Region 28 SEP 2022 PM 3 L



Current Resident 110 Royal View Dr ZEBULON NC 27597

> NIXIE 276 4E 1 0210/09/22 RETURN TO SENDER VACANT UNABLE TO FORWARD

ZEBULON SOUTH PRELIMINARY PLAN TOWN OF ZEBULON, WAKE COUNTY, NORTH CAROLINA

SITE DATA

| PROJECT: | ZEBULON SOUTH |
|--------------------|---|
| ENGINEER: | TIMMONS GROUP 5410 TRINITY ROAD, SUITE 102 RALEIGH, NC 27607 PHONE: 919-866-4509 FAX: 919-859-5663 BETH BLACKMON, PE EMAIL: BETH.BLACKMON@TIMMONS.COM |
| DEVELOPER: | DEACON DEVELOPMENT GROUP PO BOX 1080 WAKE FOREST, NC 27588 PHONE: 919 608-3542 ANDREW SURIANO ANDREW@DEACONCOMPANIES.COM |
| PROPERTY LOCATION: | 751 S WAKEFIELD ROAD 700 S ARENDELL AVE 0 S WAKEFIELD STREET 0 S ARENDELL AVE |
| PIN: | 2705-41-3075, 2704-49-2511, & 2705-51-2202, 2705-51-3114 |
| EXISTING ZONING: | R4 & R2 |
| PROPOSED ZONING: | PD |
| EXISTING USE: | AGRICULTURAL |
| TOTAL TRACT AREA: | 118.62 ACRES |

L



VICINITY MAP - 1" = 500'

OWNERS OF RECORD

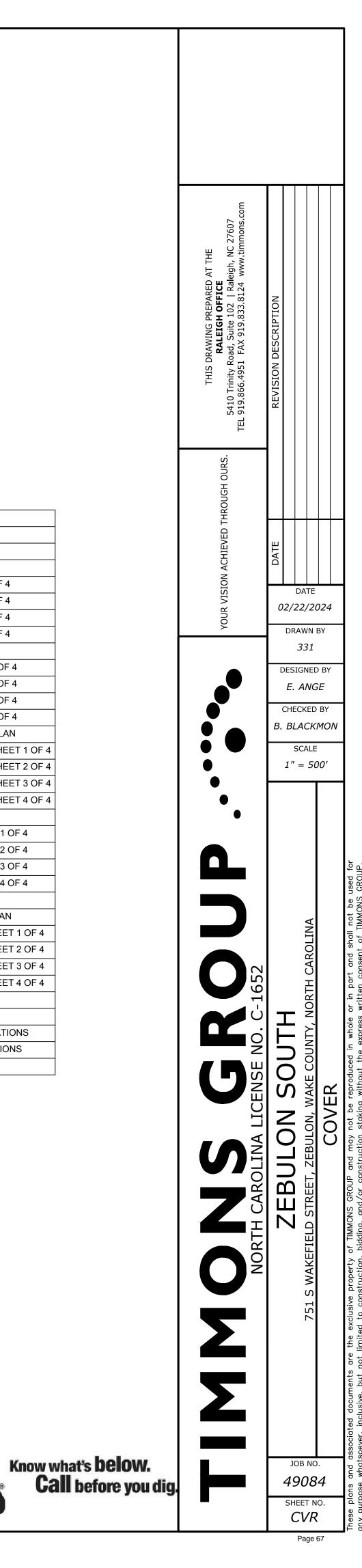
PIN: 2704-49-2511 NARRON, HAROLD CORBETT, C FRED 3941 ZEBULON RD ZEBULON, NC 27597

PIN: 2705-41-3075 WATSON FAMILY LLC. 6220 FORESTVILLE RD RALEIGH, NC 27604

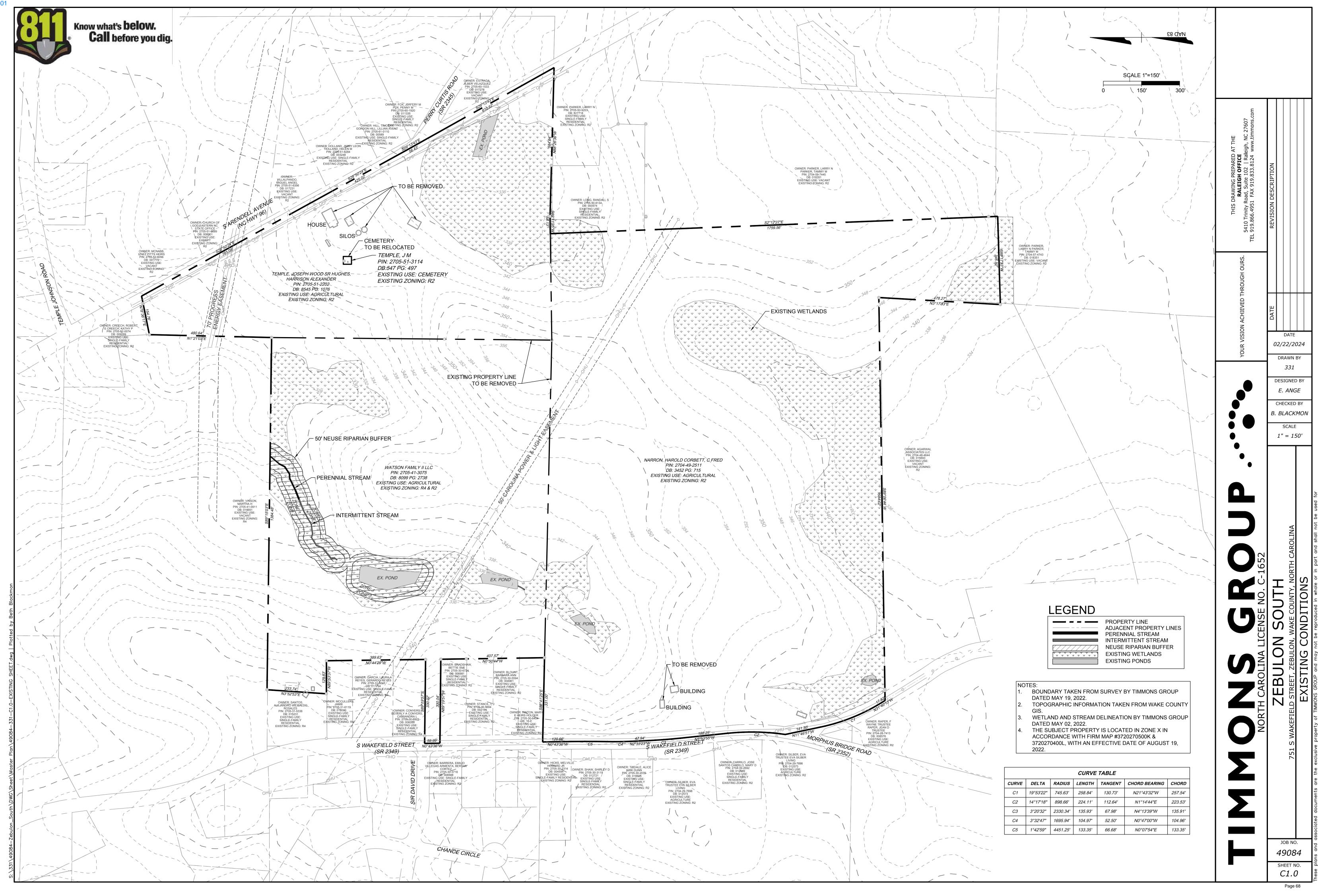
PIN: 2705-51-2202 TEMPLE, JOSEPH WOOD SR HUGHES, HARRISON ALEXANDER PO BOX 548 ZEBULON, NC 27597

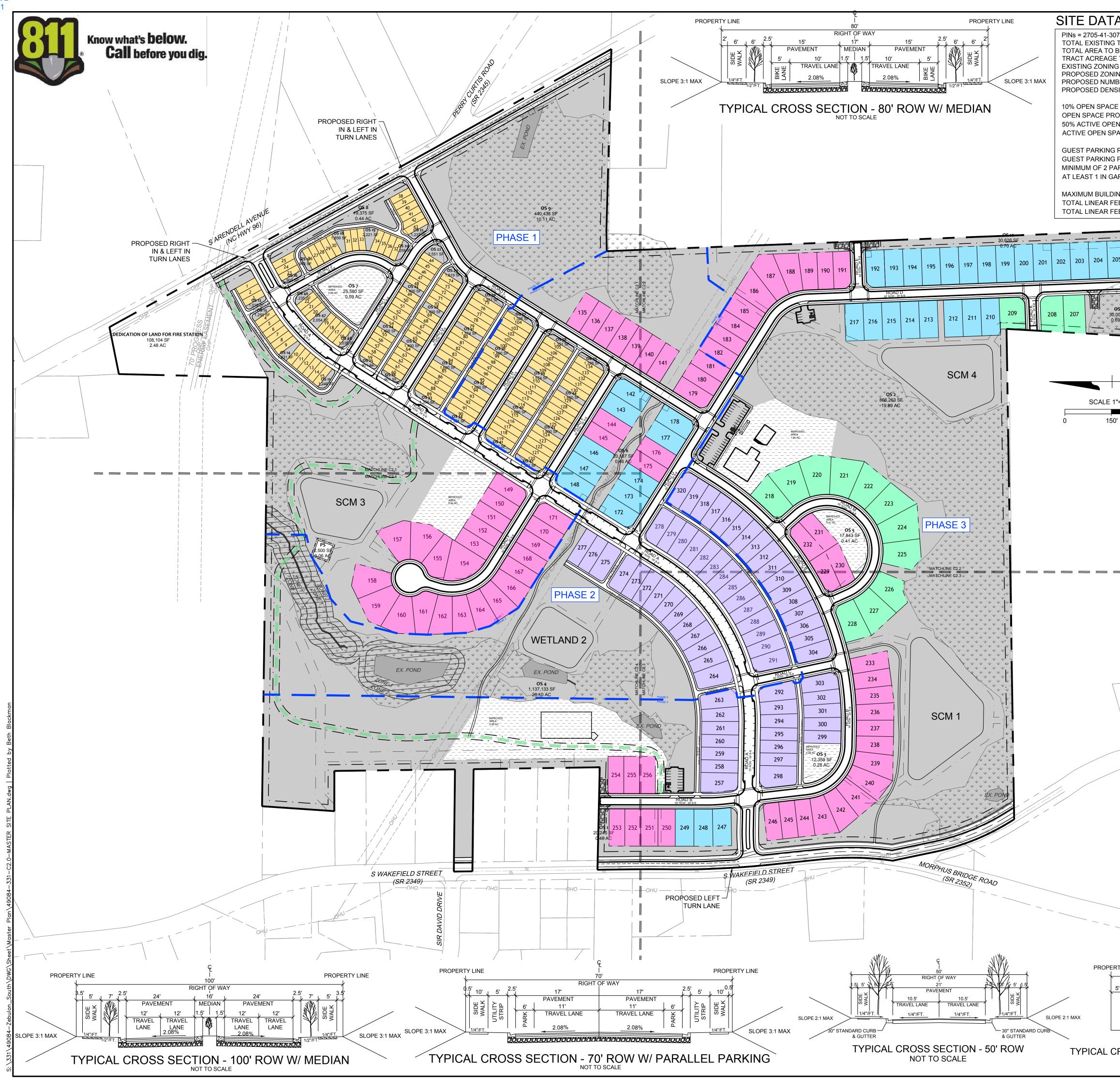
PIN: 2705-51-3114 TEMPLE, J M 1424 S HOLLYBROOK RD WENDELL NC 27591-9584

| | Sheet List Table |
|--------------|---|
| Sheet Number | Sheet Title |
| CVR | COVER |
| C1.0 | EXISTING CONDITIONS |
| C2.0 | OVERALL SITE PLAN |
| C2.1 | DETAILED SITE PLAN SHEET 1 OF 4 |
| C2.2 | DETAILED SITE PLAN SHEET 2 OF 4 |
| C2.3 | DETAILED SITE PLAN SHEET 3 OF 4 |
| C2.4 | DETAILED SITE PLAN SHEET 4 OF 4 |
| C3.0 | OVERALL UTILITY PLAN |
| C3.1 | DETAILED UTILITY PLAN SHEET 1 OF 4 |
| C3.2 | DETAILED UTILITY PLAN SHEET 2 OF 4 |
| C3.3 | DETAILED UTILITY PLAN SHEET 3 OF 4 |
| C3.4 | DETAILED UTILITY PLAN SHEET 4 OF 4 |
| C4.0 | OVERAL GRADING & DRAINAGE PLAN |
| C4.1 | DETAILED GRADING & DRAINAGE PLAN SHEET 1 OF 4 |
| C4.2 | DETAILED GRADING & DRAINAGE PLAN SHEET 2 OF 4 |
| C4.3 | DETAILED GRADING & DRAINAGE PLAN SHEET 3 OF 4 |
| C4.4 | DETAILED GRADING & DRAINAGE PLAN SHEET 4 OF 4 |
| C5.0 | OVERALL LANDSCAPE PLAN |
| C5.1 | DETAILED LANDSCAPE PLAN SHEET 1 OF 4 |
| C5.2 | DETAILED LANDSCAPE PLAN SHEET 2 OF 4 |
| C5.3 | DETAILED LANDSCAPE PLAN SHEET 3 OF 4 |
| C5.4 | DETAILED LANDSCAPE PLAN SHEET 4 OF 4 |
| C6.0 | LIGHTING PLAN |
| C7.0 | OVERALL SIGNS & MARKINGS PLAN |
| C7.1 | DETAILED SIGNS & MARKINGS PLAN SHEET 1 OF 4 |
| C7.2 | DETAILED SIGNS & MARKINGS PLAN SHEET 2 OF 4 |
| C7.3 | DETAILED SIGNS & MARKINGS PLAN SHEET 3 OF 4 |
| C7.4 | DETAILED SIGNS & MARKINGS PLAN SHEET 4 OF 4 |
| C8.0 | DETAILED AMENITY PLAN |
| C8.1 | DETAILED AMENITY PLAN |
| C9.1 | SINGLE FAMILY CONCEPTUAL ELEVATIONS |
| C9.2 | TOWNHOME CONCEPTUAL ELEVATIONS |
| C9.3 | CONCEPUTUAL AMENITY |



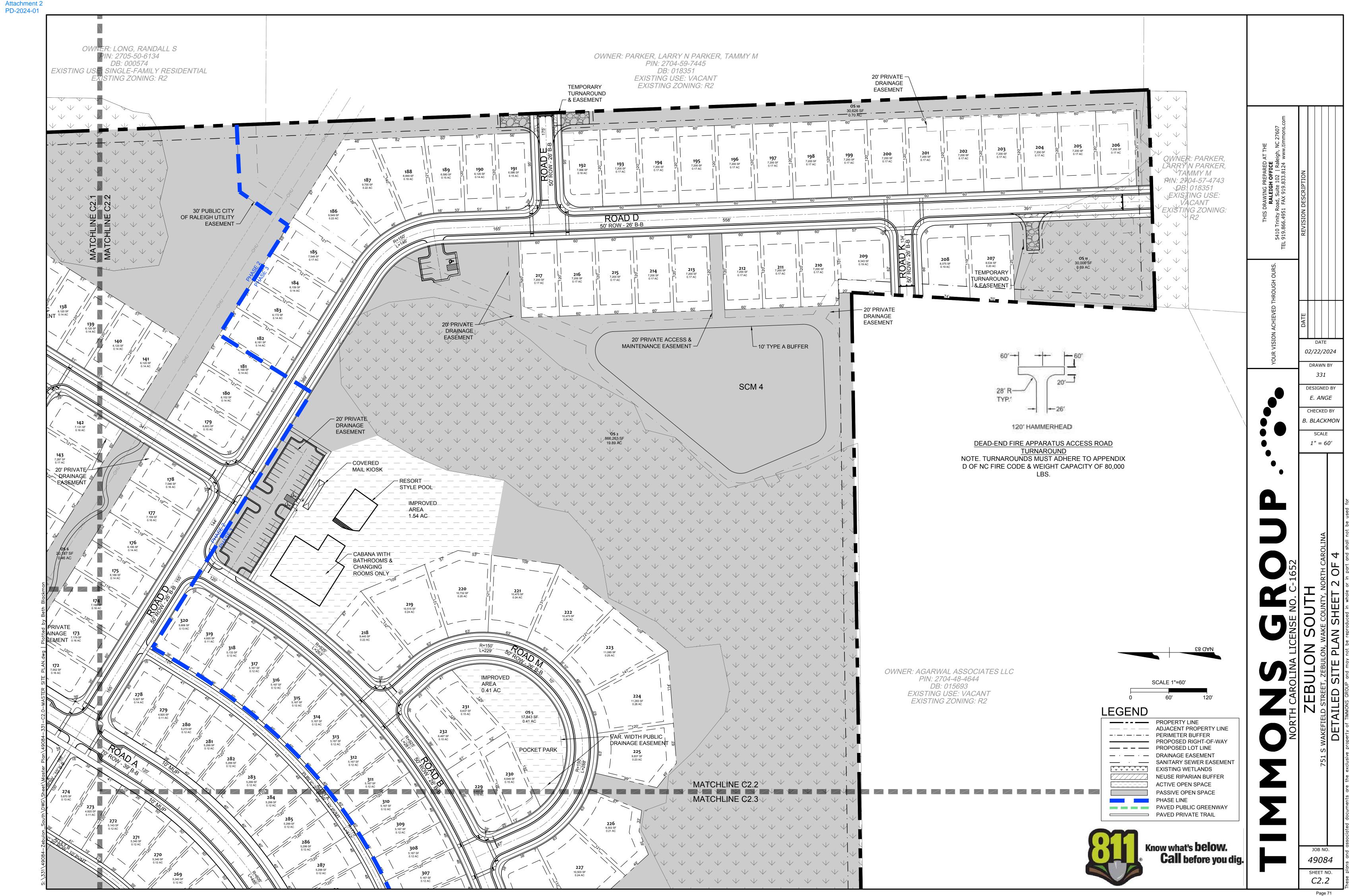




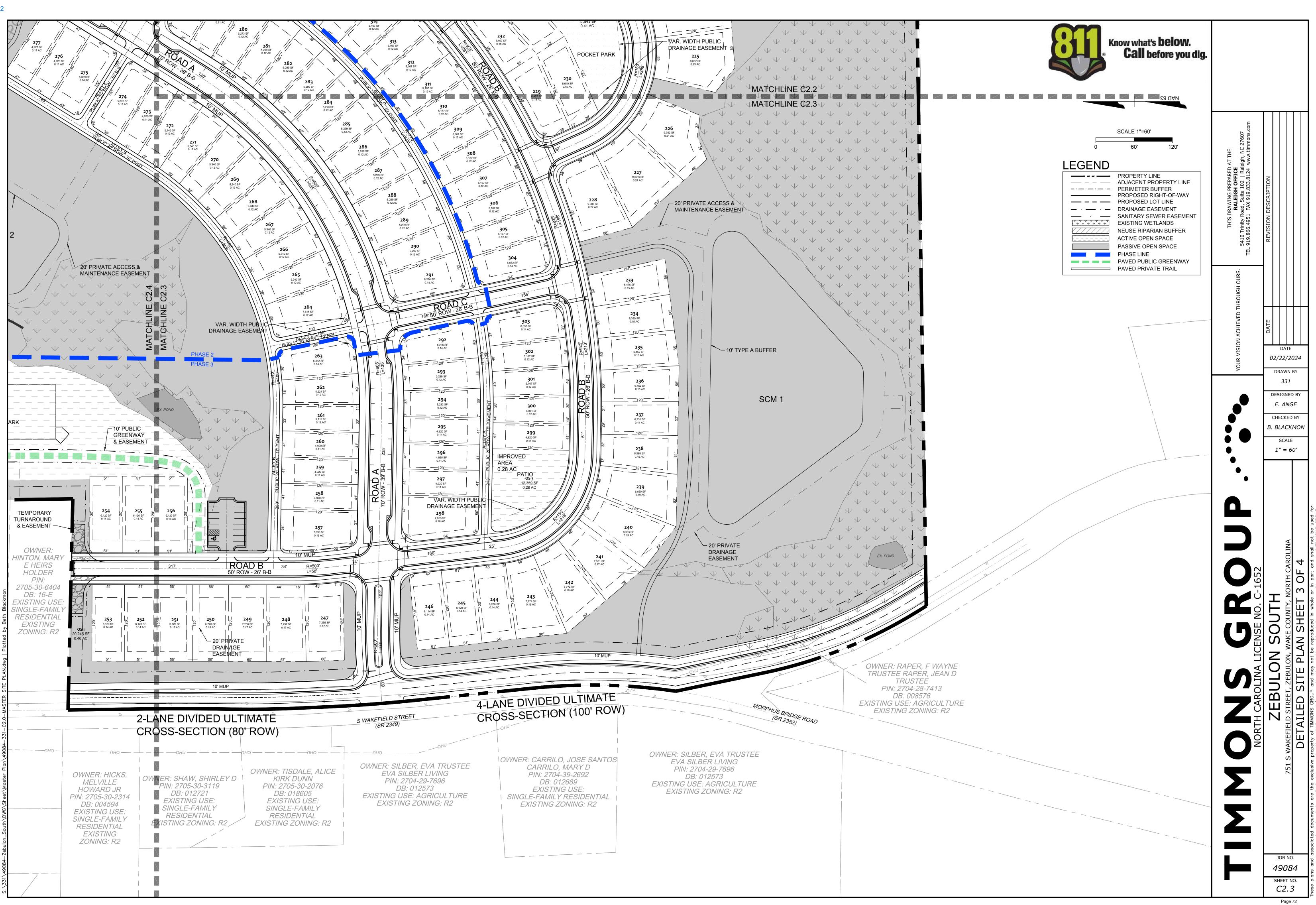


| SPACE PROVIDED = 6.27 acres G REQUIRED 0.25 UNITS = 80 space G PROVIDED = 170 spaces PARKING SPACES PROVIDED ON I GARAGE AND 1 IN DRIVEWAY DING HEIGHT = 50' FEET OF 10' PUBLIC GREENWAY = | C s ces EACH LOT, = 3,415' | | MINIMUM LC AVERAGE L MINIMUM LC DRIVEWAY MINIMUM D 64 - REAF MINIMUM LC AVERAGE L MINIMUM LC DRIVEWAY MINIMUM D 72 - FROM MINIMUM LC AVERAGE L MINIMUM LC DRIVEWAY | R LOAD S DT SIZE PRO OT SIZE PRO T WIDTH: 1 LENGTH: 20 RIVEWAY W R LOAD SI DT SIZE PRO DT SIZE PRO DT SIZE PRO T WIDTH: 4 LENGTH: 20 RIVEWAY W | ' 'IDTH: 10' NGLE FAMILY OVIDED: 4,920' OVIDED: 5,410' '0' | ACHED | | | <u> </u> | |
|---|---|-----------------|---|--|---|---------------------|--|-------------|----------------------|--------|
| D BE DEDICATED = 2.48 AC GE TO BE DEVELOPED = 116.14 AC NG = R2 & R4 NING = PD MBER OF LOTS = 320 NSITY = 2.75 DU/A CE REQUIRED = 11.61 acres ROVIDED = 61.36 acres (52.83%) PEN SPACE REQUIRED = 5.81 acres SPACE PROVIDED = 6.27 acres G REQUIRED 0.25 UNITS = 80 space G PROVIDED = 170 spaces PARKING SPACES PROVIDED ON I GARAGE AND 1 IN DRIVEWAY DING HEIGHT = 50' FEET OF 10' PUBLIC GREENWAY = FEET OF 6' PRIVATE GREENWAY = | s ces EACH LOT, = 3,415' | | AVERAGE L MINIMUM LC DRIVEWAY MINIMUM D 64 - REAF MINIMUM LC AVERAGE L MINIMUM LC DRIVEWAY MINIMUM D 72 - FROM MINIMUM LC AVERAGE L MINIMUM LC DRIVEWAY | OT SIZE PR DT WIDTH: 1 LENGTH: 20 RIVEWAY W R LOAD SI DT SIZE PR DT SIZE PR DT WIDTH: 4 LENGTH: 20 RIVEWAY W NT LOAD S DT SIZE PR | OVIDED: 1,880' 8' 'IDTH: 10' NGLE FAMILY OVIDED: 4,920' OVIDED: 5,410' 0' 'IDTH: 10' | | | | <u></u> | |
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| G PROVIDED = 170 spaces PARKING SPACES PROVIDED ON I GARAGE AND 1 IN DRIVEWAY DING HEIGHT = 50' FEET OF 10' PUBLIC GREENWAY = FEET OF 6' PRIVATE GREENWAY = | EACH LOT, = 3,415' | | AVERAGE L MINIMUM LO DRIVEWAY | | CHARLE I MINILT LU | s | COM | | | |
| GARAGE AND 1 IN DRIVEWAY DING HEIGHT = 50' FEET OF 10' PUBLIC GREENWAY = FEET OF 6' PRIVATE GREENWAY = | = 3,415' | | DRIVEWAY | UL SIZE PR | OVIDED: 6,114' OVIDED: 6,963' | | AT THE aigh, NC 27607 www.timmons.com | | | |
| DING HEIGHT = 50' FEET OF 10' PUBLIC GREENWAY = FEET OF 6' PRIVATE GREENWAY = | | | | | | | THE , NC 3 w.timi | | | |
| FEET OF 10' PUBLIC GREENWAY = FEET OF 6' PRIVATE GREENWAY = | | | MINIMUM D | RIVEWAY W | 'IDTH: 10' | | U U | | | |
| | = 2,040' | | | | SINGLE FAMILY LOT DVIDED: 7,046' | S | REPARED OFFICE 02 Ral 33.8124 | NO | | |
| 205 206 | | | | OT WIDTH: 6 | | | | IPTI | | |
| 205 206 | | | DRIVEWAY MINIMUM D | | | | THIS DRAWING PF RALEIGH inity Road, Suite 1 6.4951 FAX 919.8 | DESCRIPTION | | |
| | | | | | NGLE FAMILY | | HIS DF ity Ro 1951 | OND | | |
| | | | AVERAGE L | OT SIZE PR | OVIDED: 8,343 OVIDED: 9,878' | | | REVISION | | |
| | | | MINIMUM LO DRIVEWAY MINIMUM D | LENGTH: 20 | , | | 54 | R | | |
| | | | | | | | TEL | | | |
| OS 11/2 ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ | | | | | N SPACE | 0.47 AC | SS. | | | |
| | ידסררי | г т ^ г | | OS 2 | 866,263 SF | 19.89 AC | H OUF | | | |
| | STREET TREET NAME | ROW | 3LE LENGTH (LF) | OS 3 OS 4 | 12,359 SF 1,137,133 SF | 0.28 AC 26.10 AC | SOUG | | | |
| R | OAD A OAD B | 70 50 | 2,775 2,023 | OS 5 OS 6 | 17,843 SF 20,187 SF | 0.41 AC 0.46 AC | YOUR VISION ACHIEVED THROUGH OURS | ┢┼┼┼ | ++ | + |
| | OAD C | 50 | 320 | OS 7 OS 8 | 25,580 SF | 0.59 AC 0.44 AC | HIEVE | DATE | | |
| R | OAD D OAD E | 50 50 | 2,112 175 | OS 9 | 19,375 SF 440,438 SF | 10.11 AC | N ACF | ۵ ا | | |
| R | OAD F OAD G | 50 50 | 1,060 464 | OS 10 OS 11 | 30,626 SF 30,006 SF | 0.70 AC 0.69 AC | IOISI/ | D | ATE | |
| 50' 300' <u>R</u> (| OAD H | 50 | 464 | OS 12 OS 13 | 1,019 SF 1,250 SF | 0.02 AC 0.03 AC | | 02/22 | 2/20. | 24 |
| | OAD I OAD J | 50 50 | 537 439 | OS 14 | 1,631 SF | 0.04 AC | × | | WN B | γ |
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| | CORNER: 10' | , | | OS 43 OS 44 | 1,600 SF 1,600 SF | 0.04 AC 0.04 AC | | | | z |
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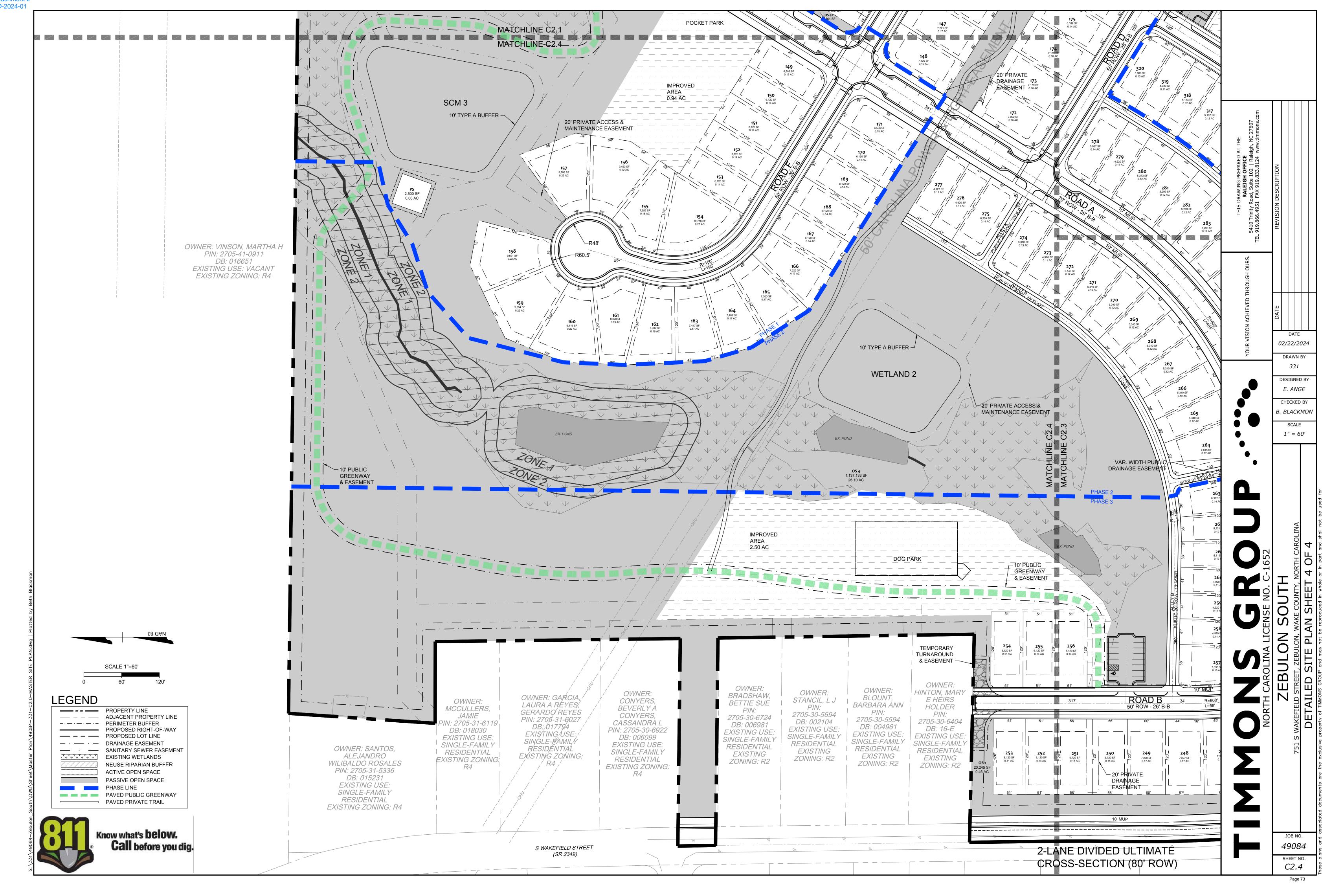




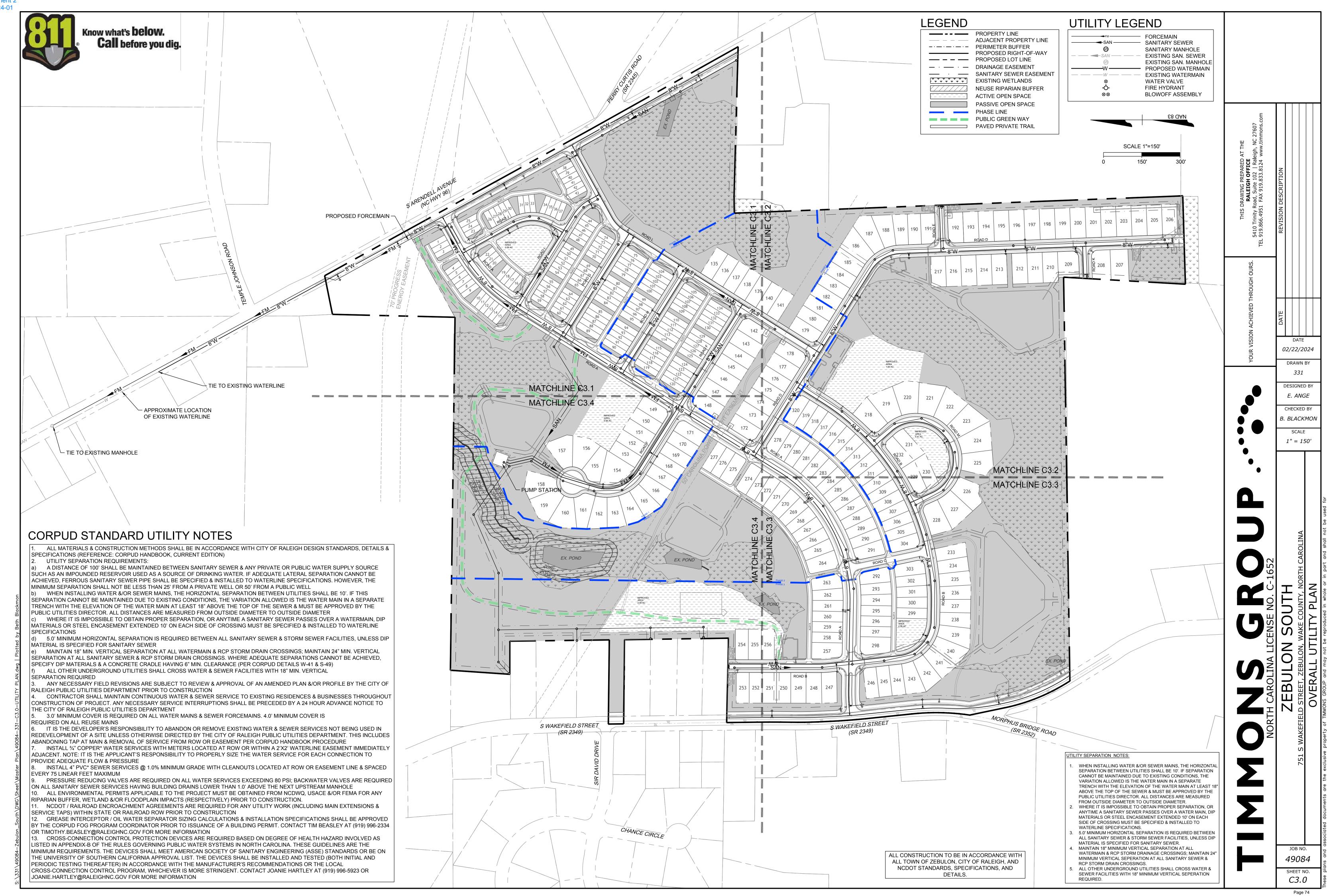
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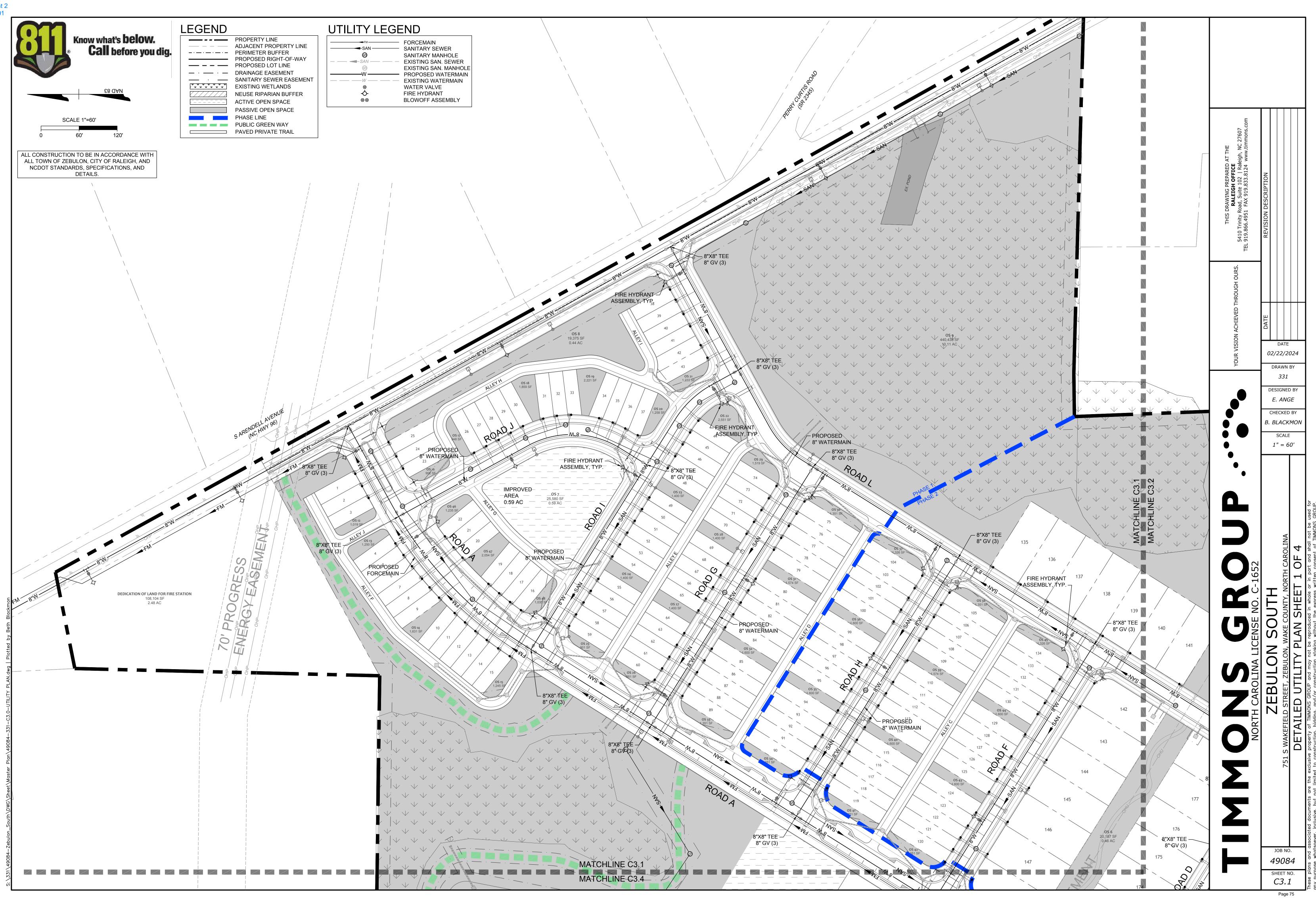


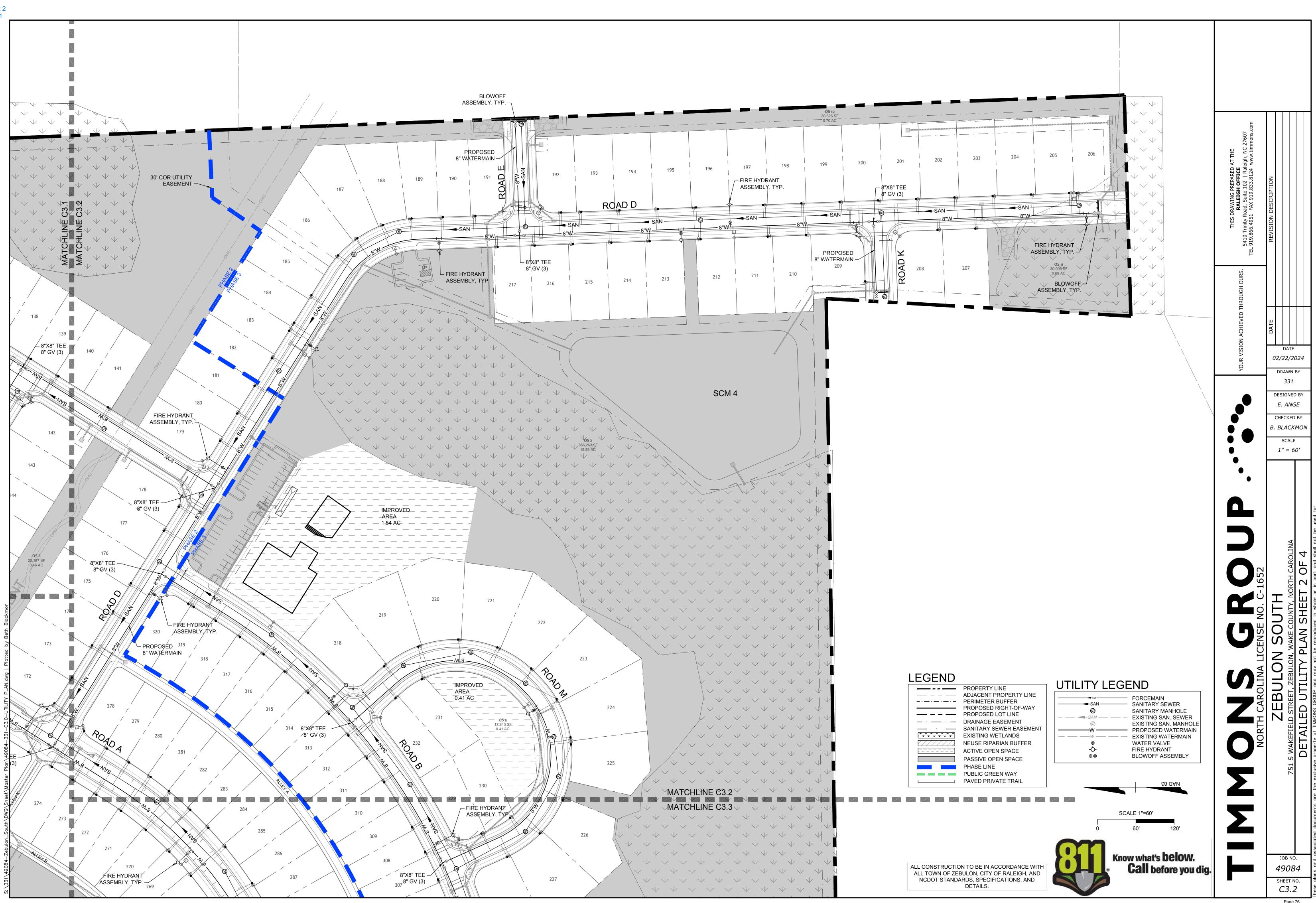


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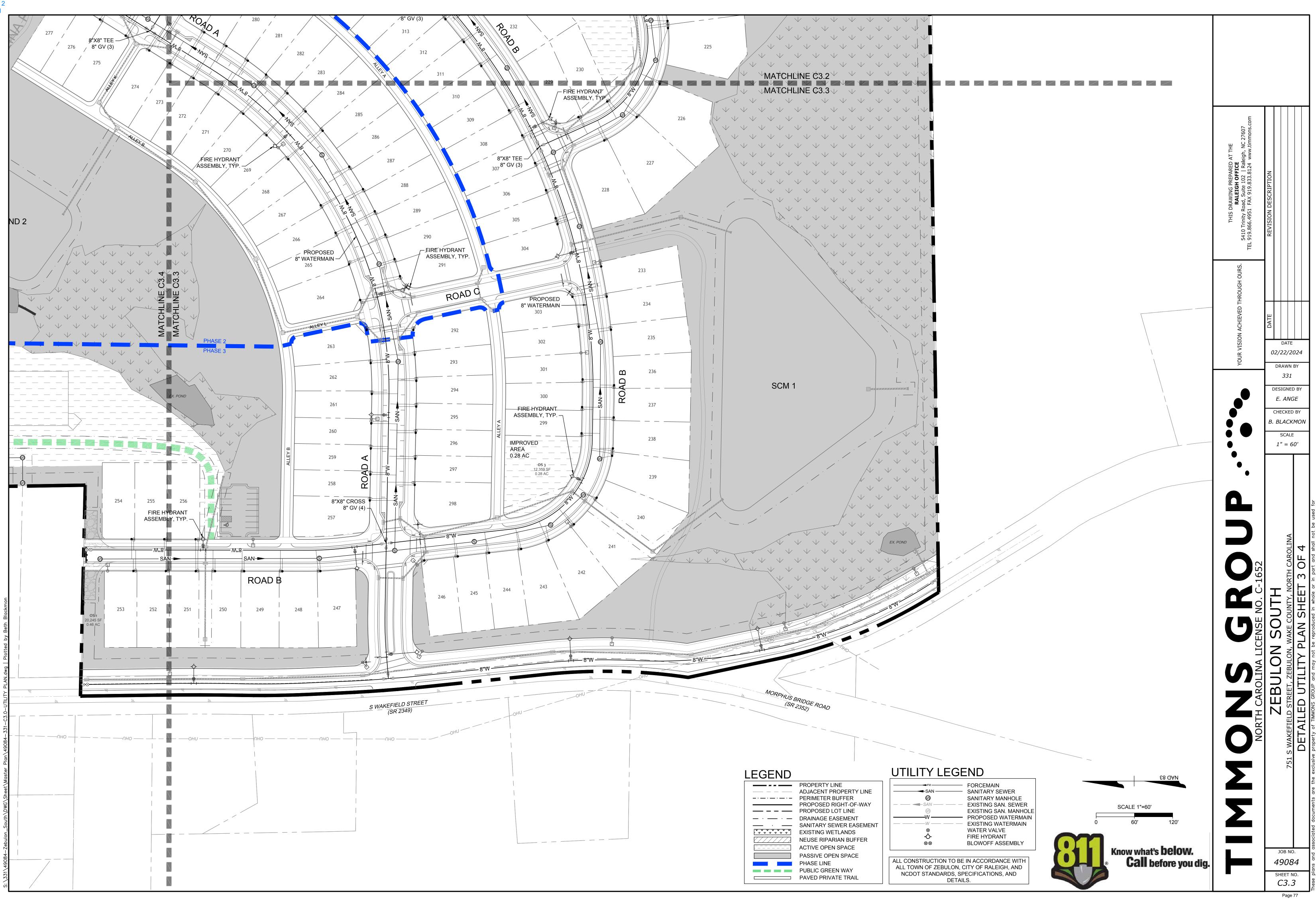


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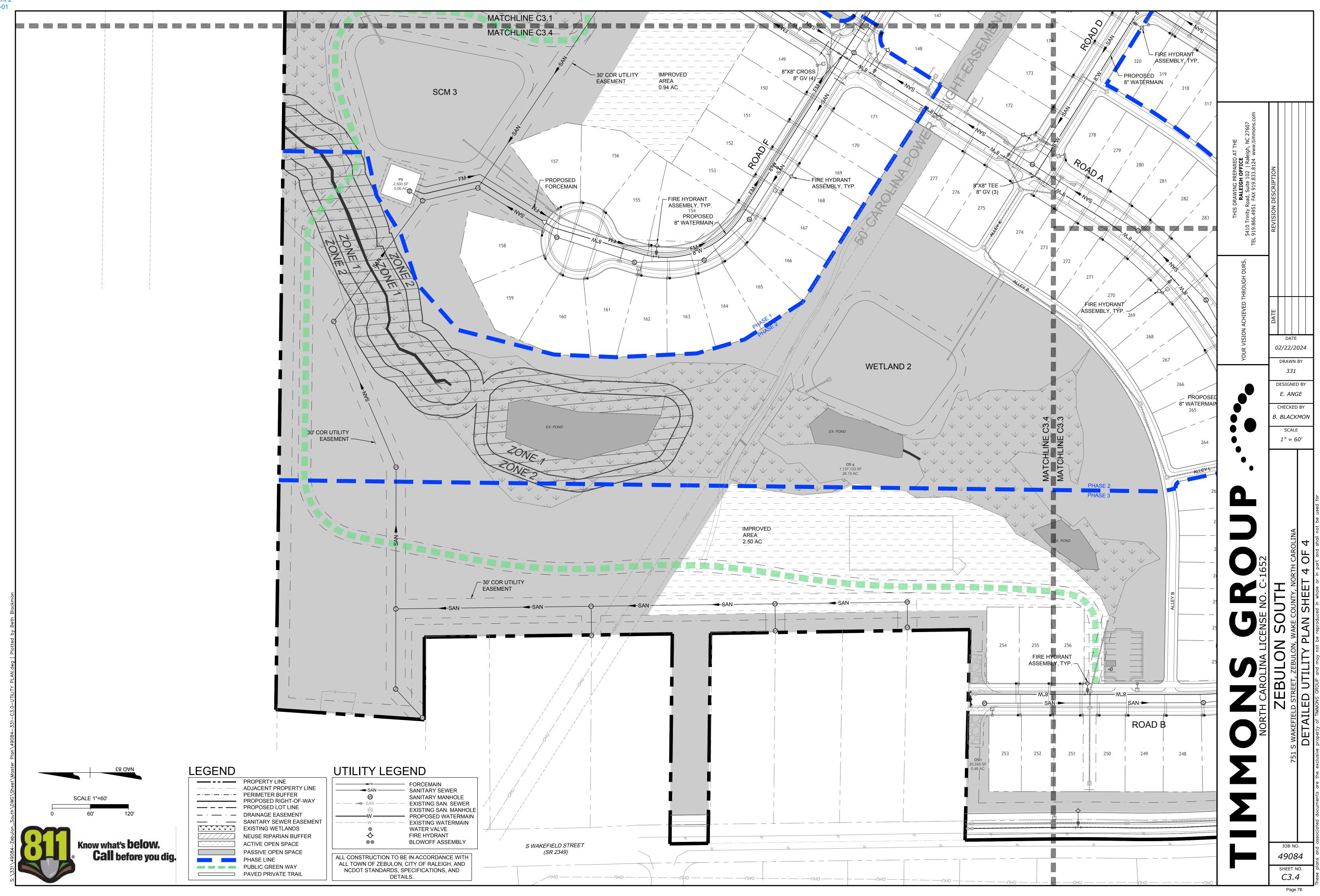


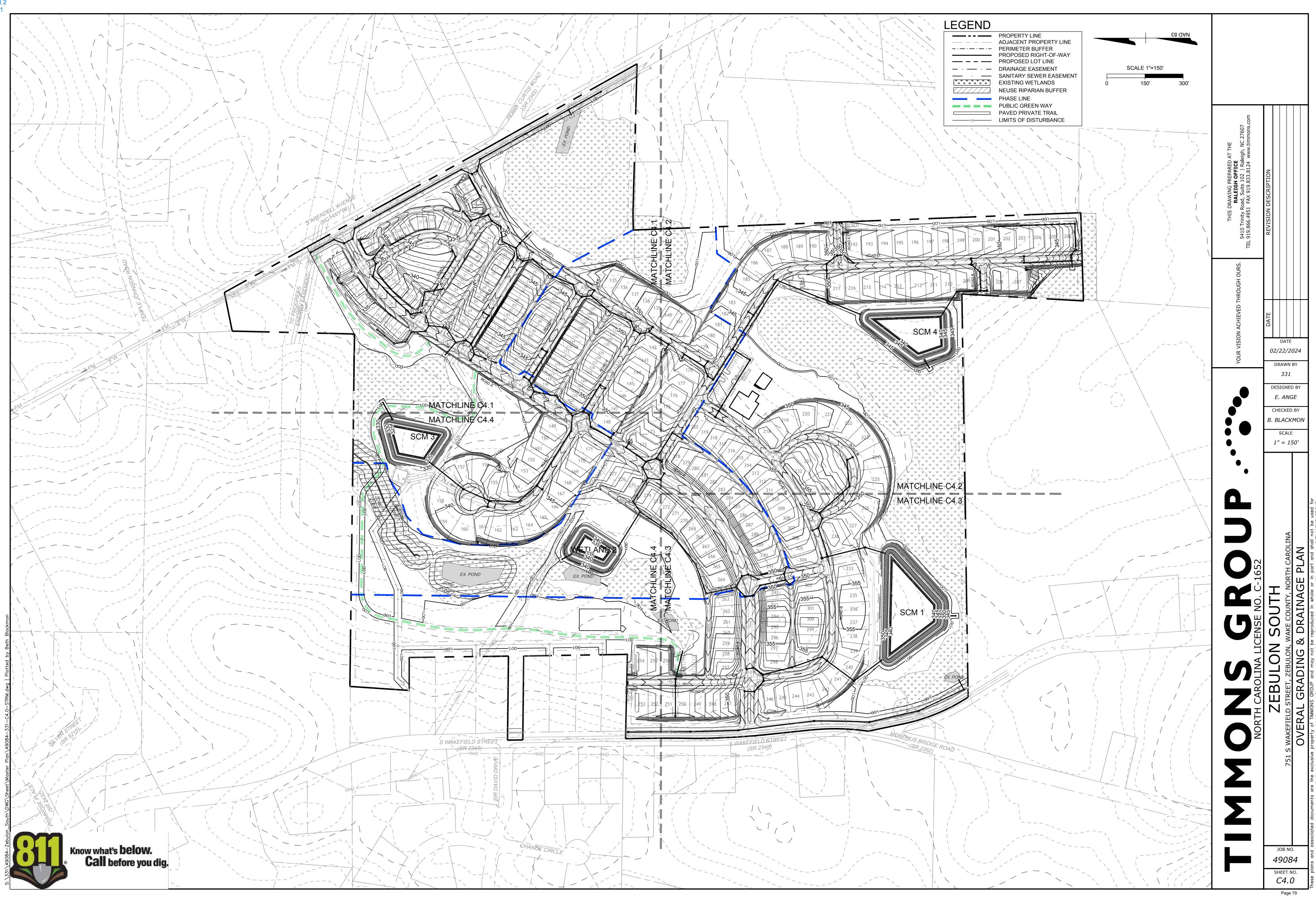


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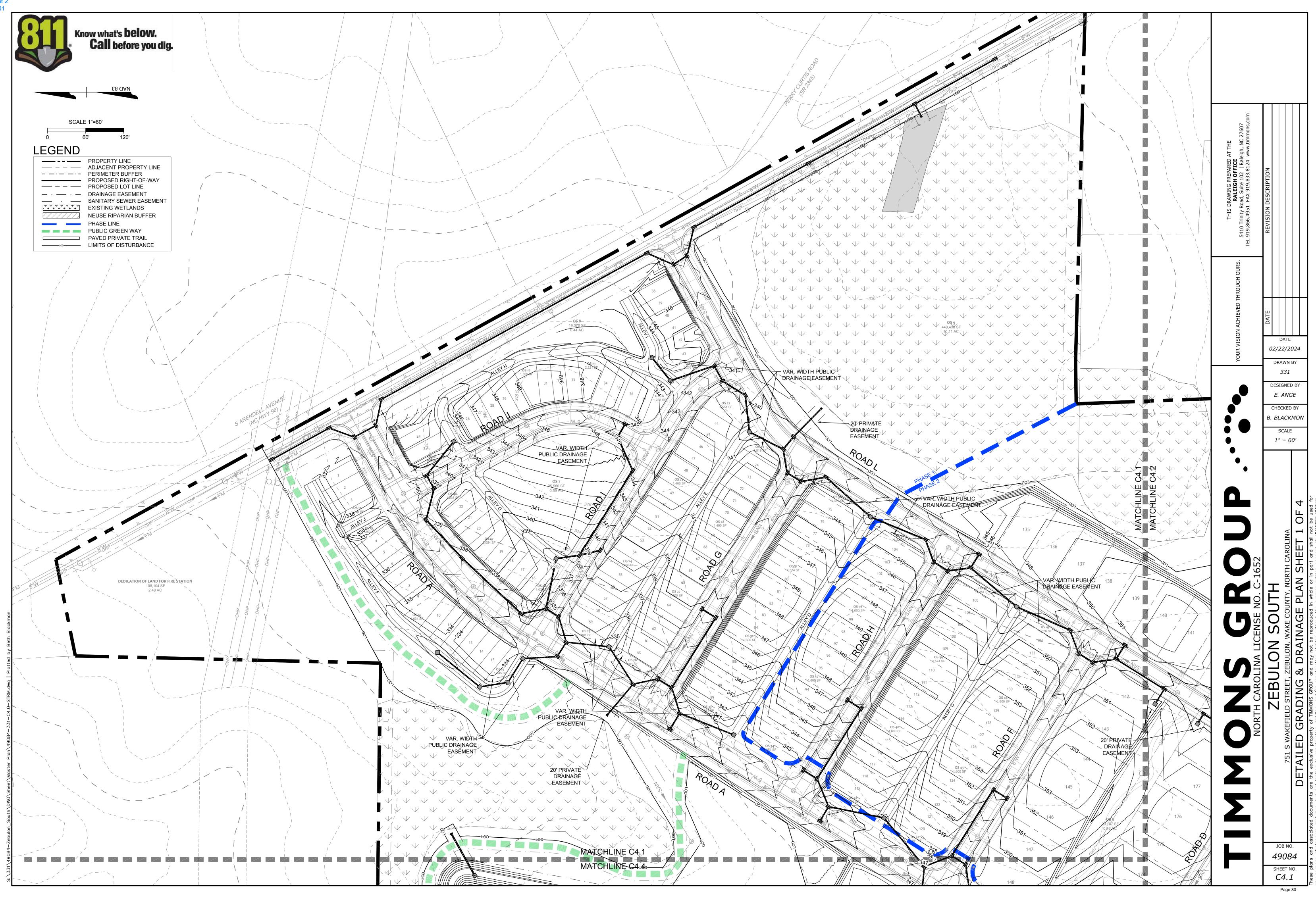




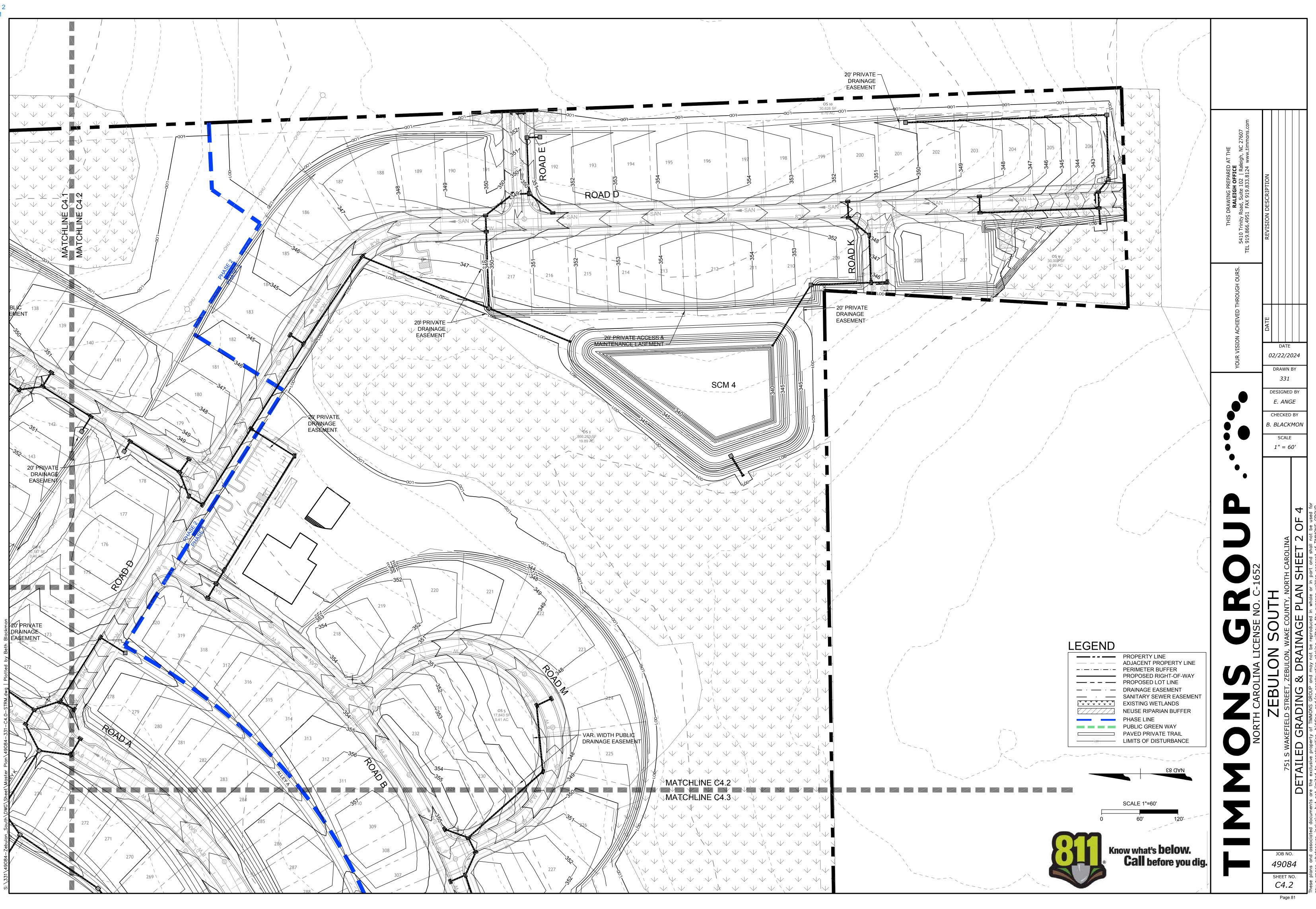


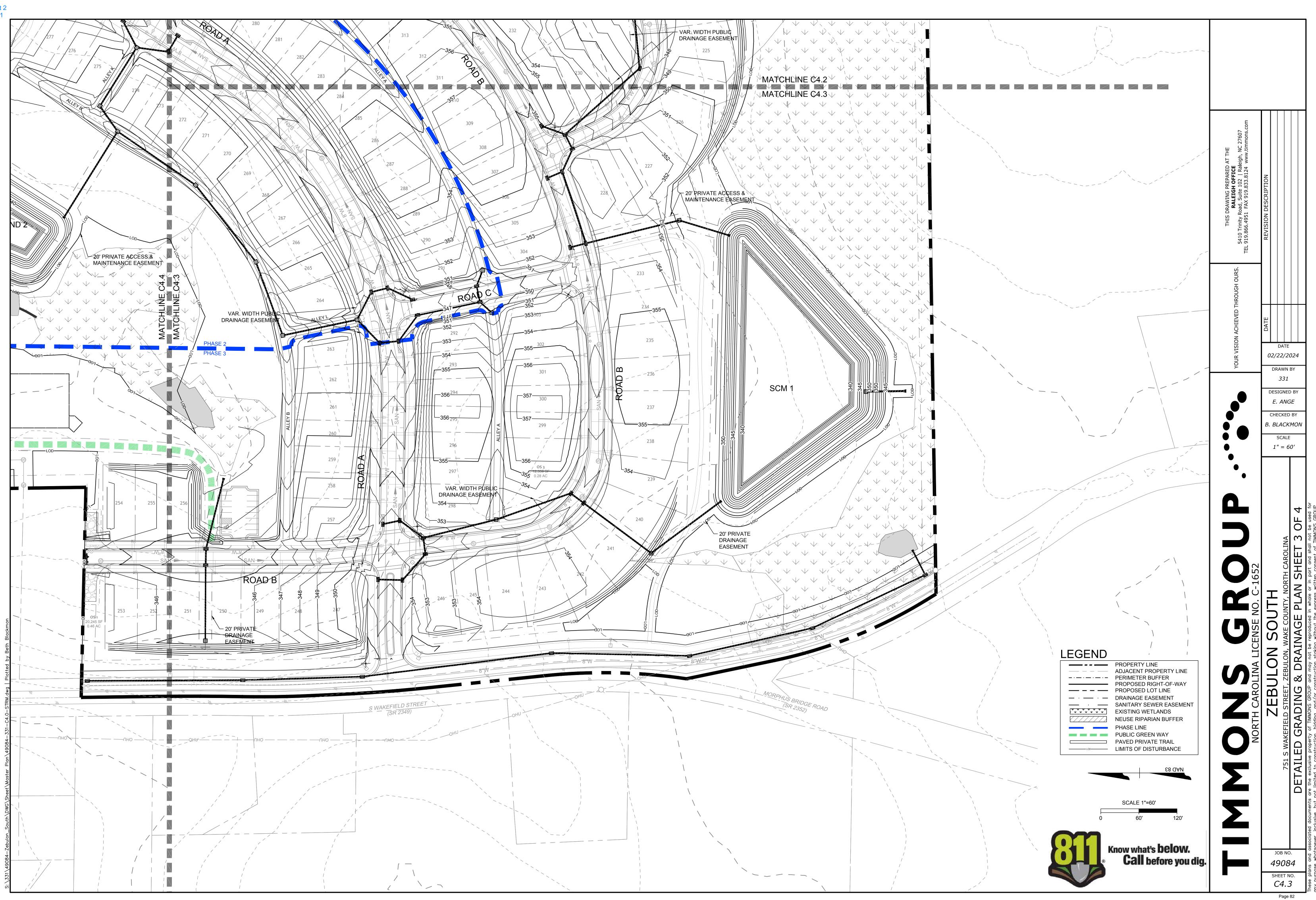


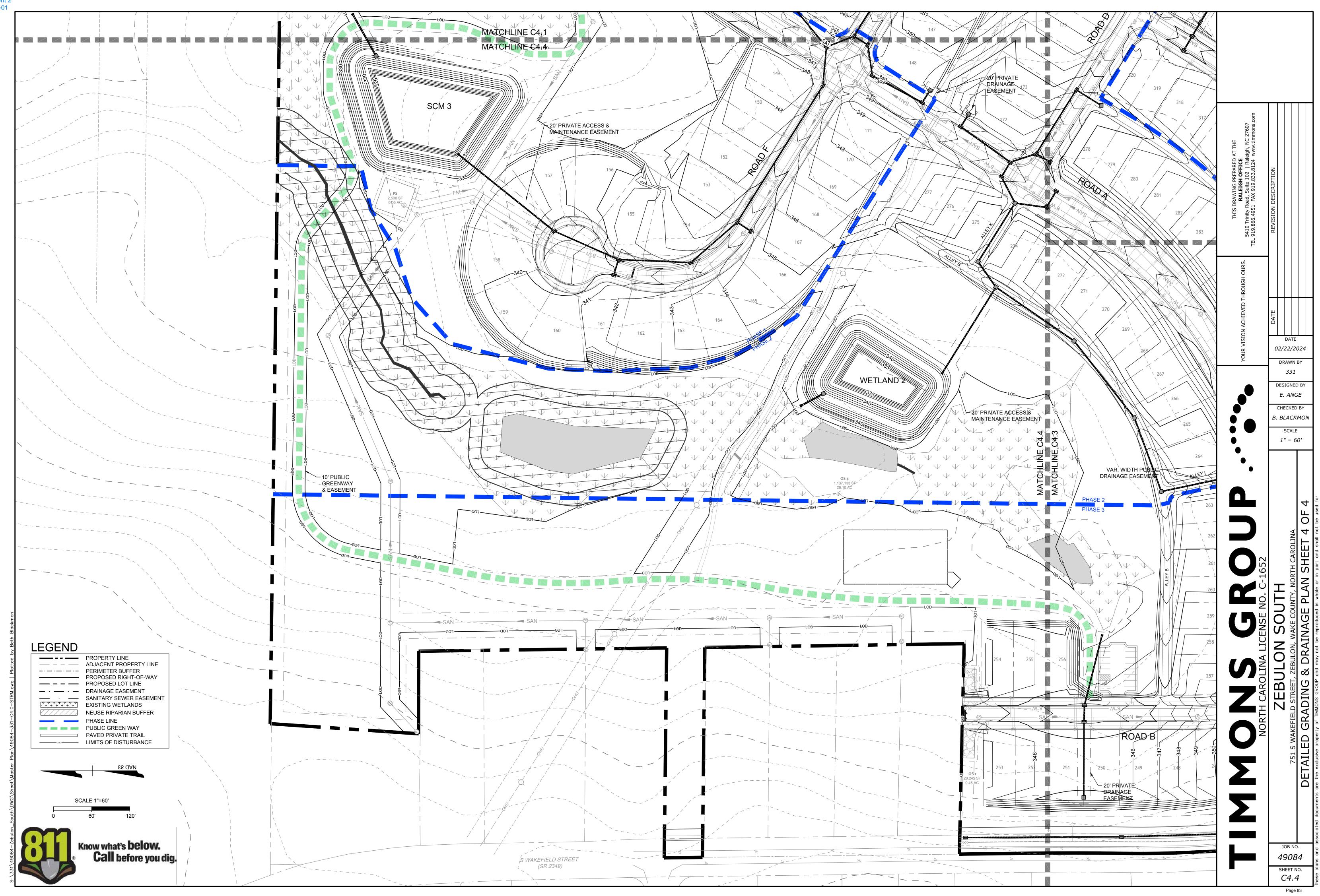




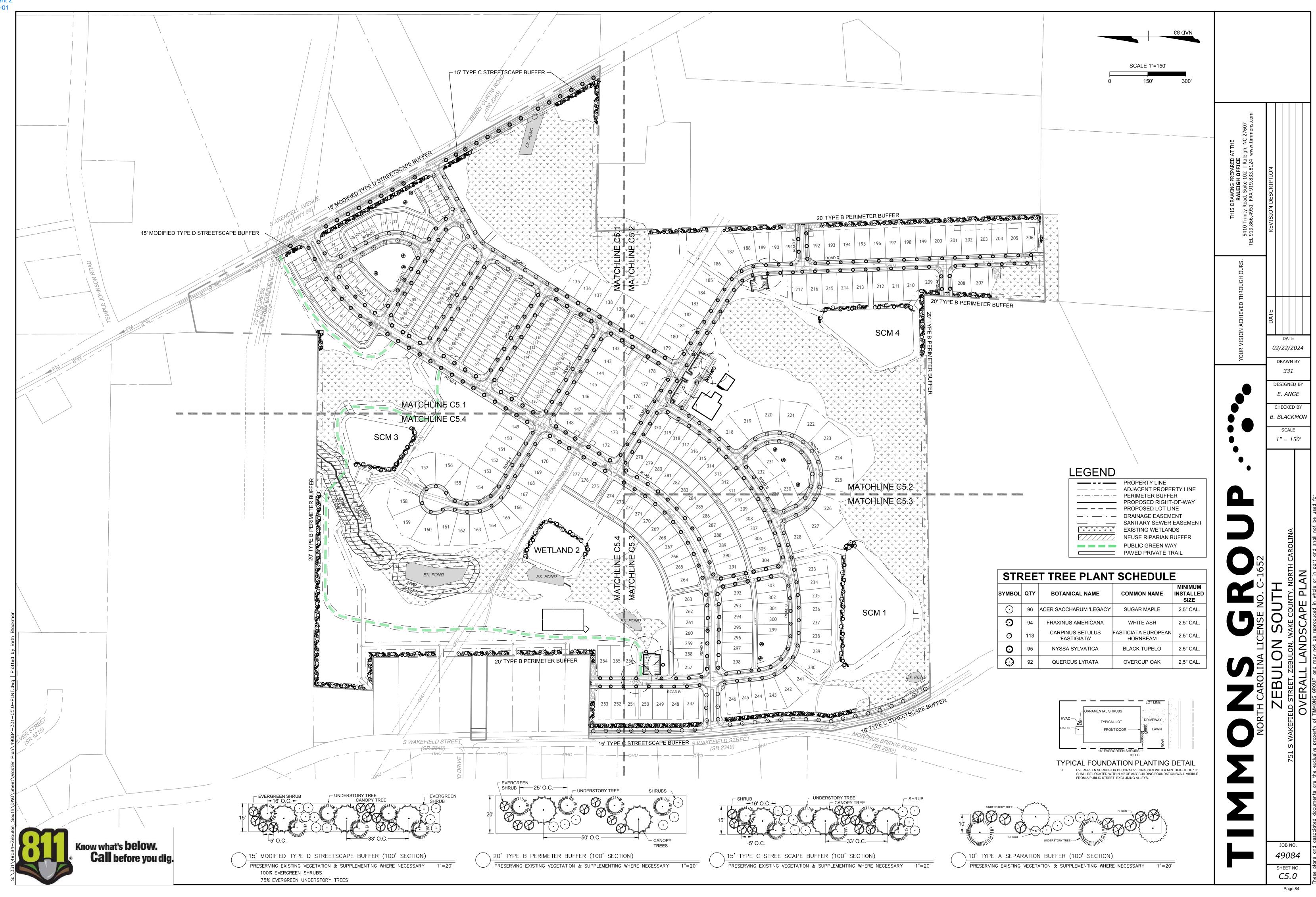
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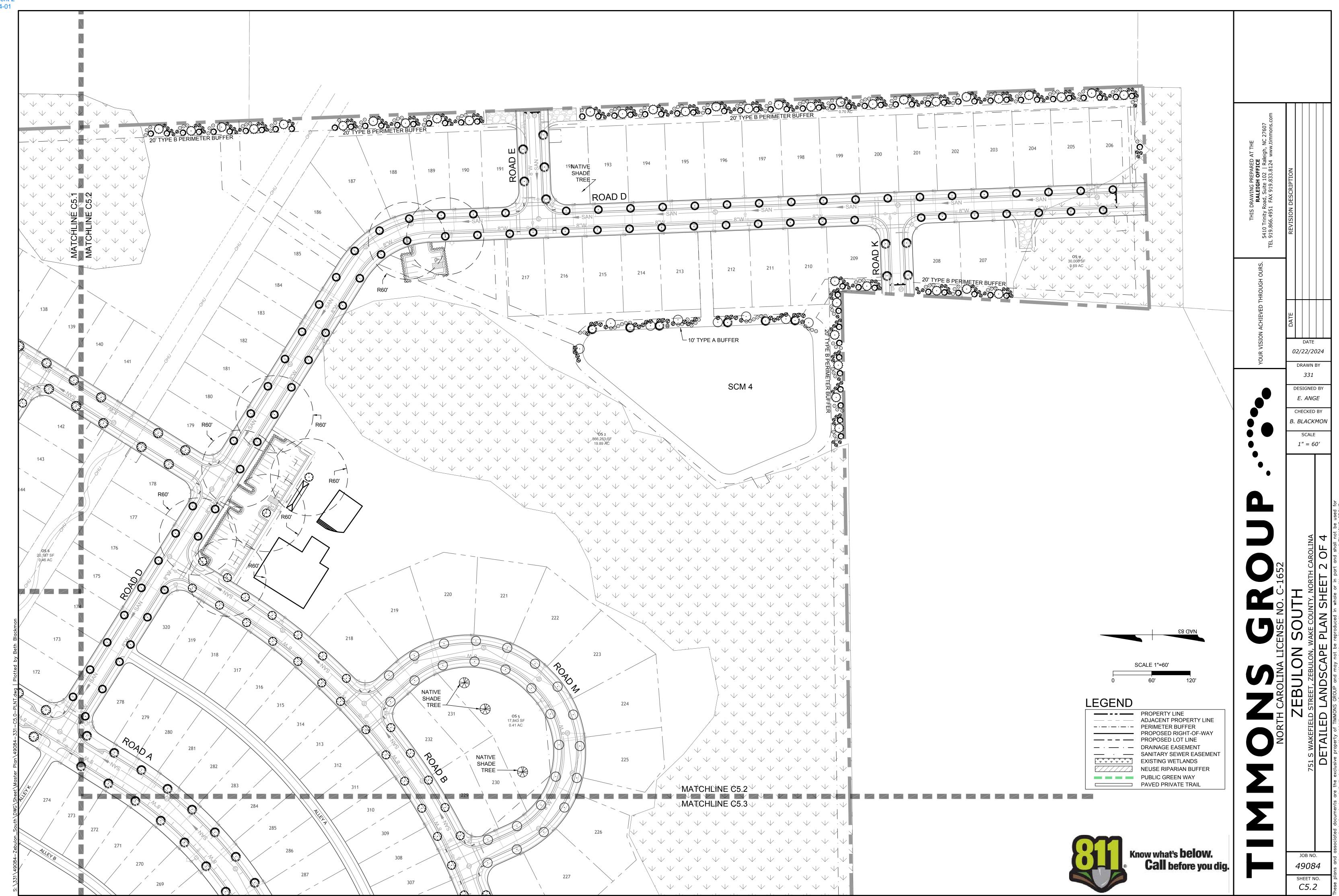




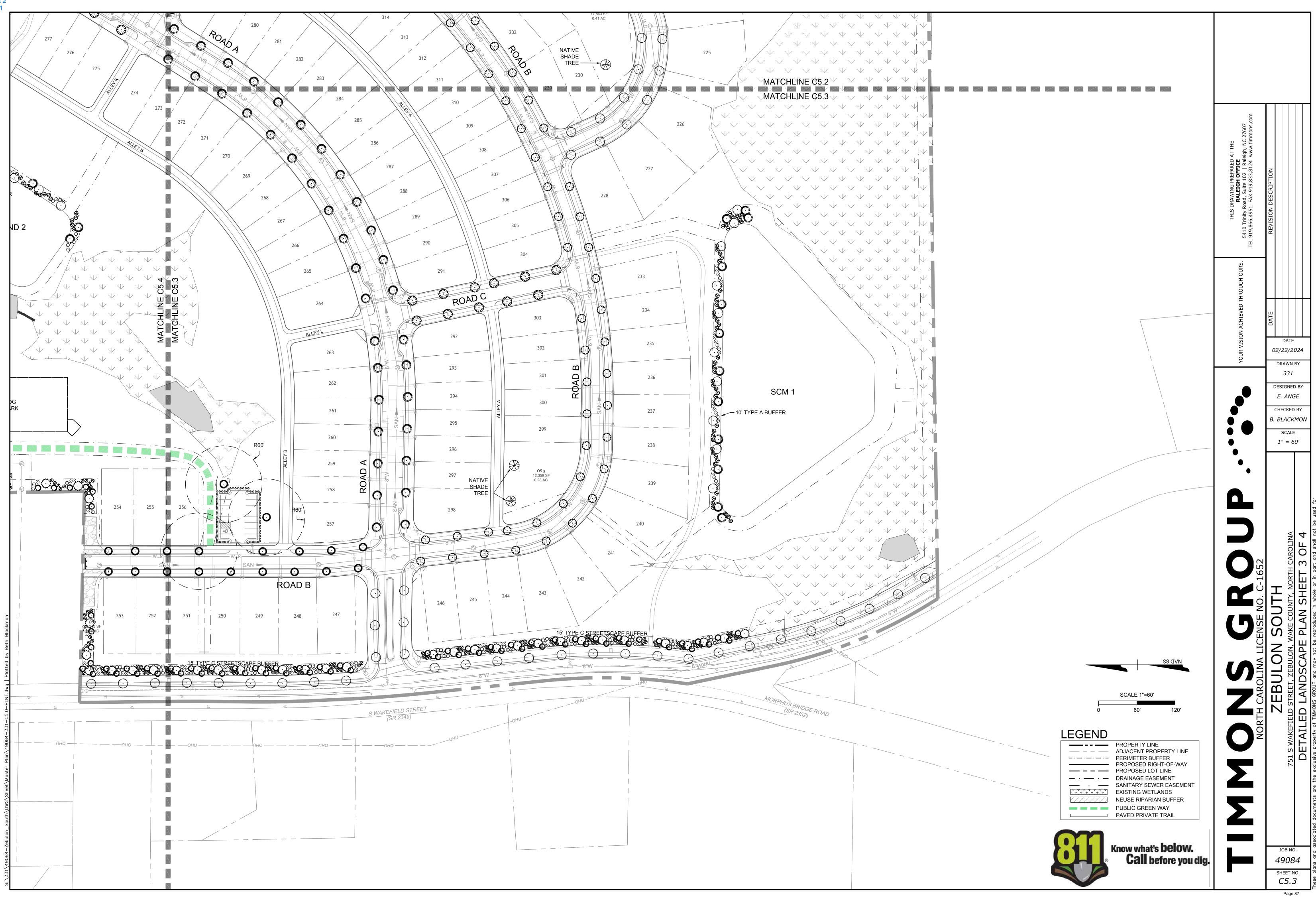




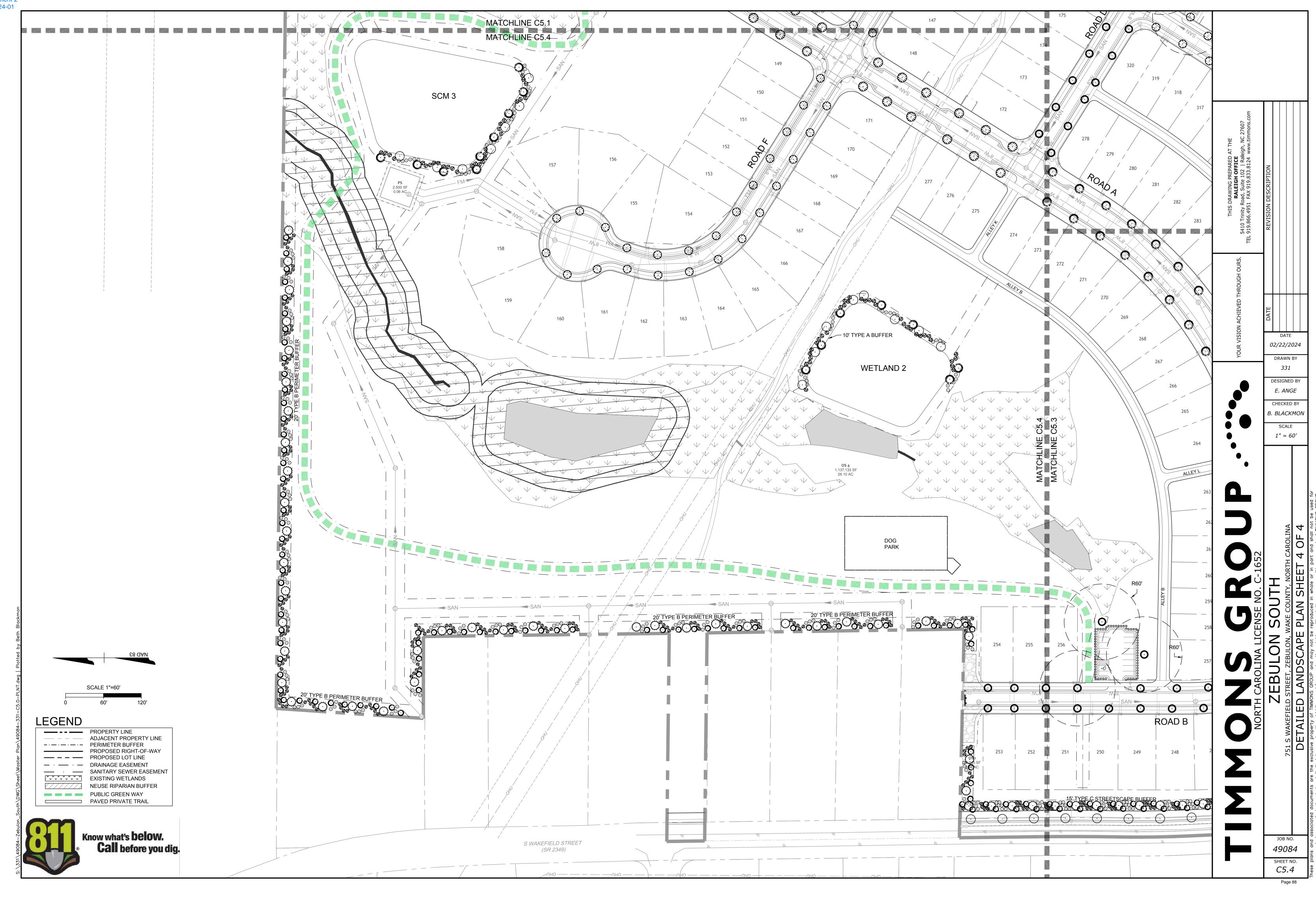




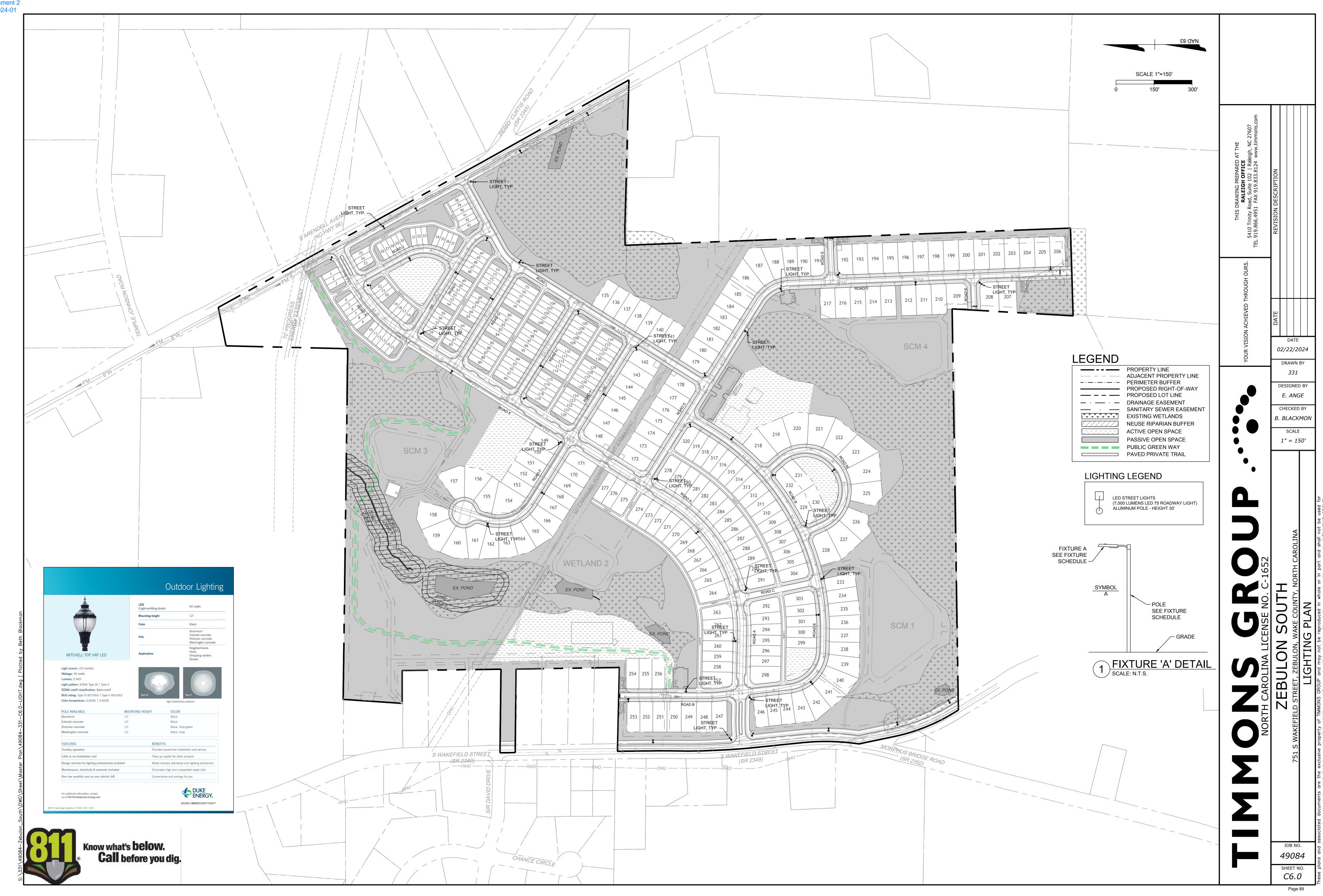
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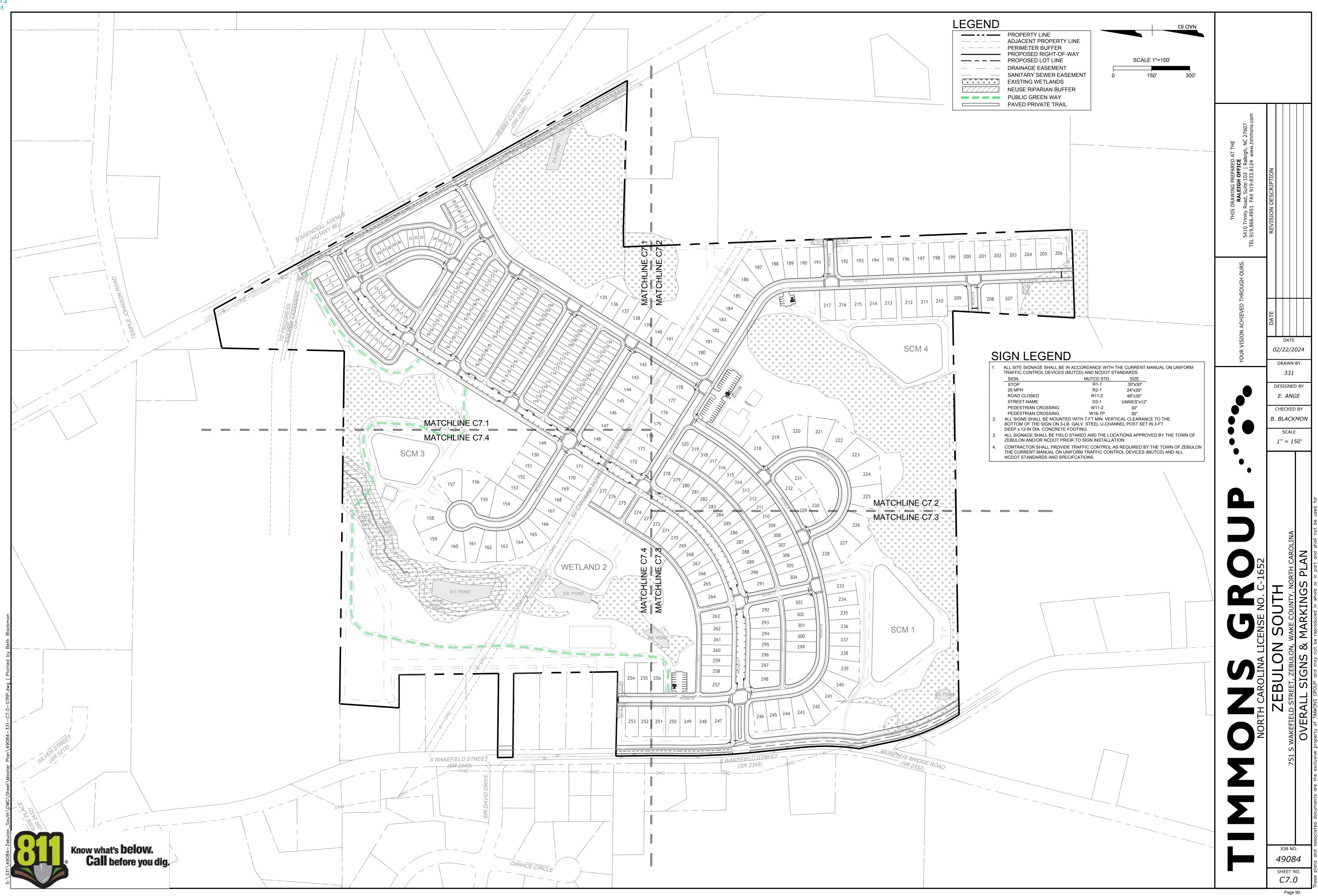




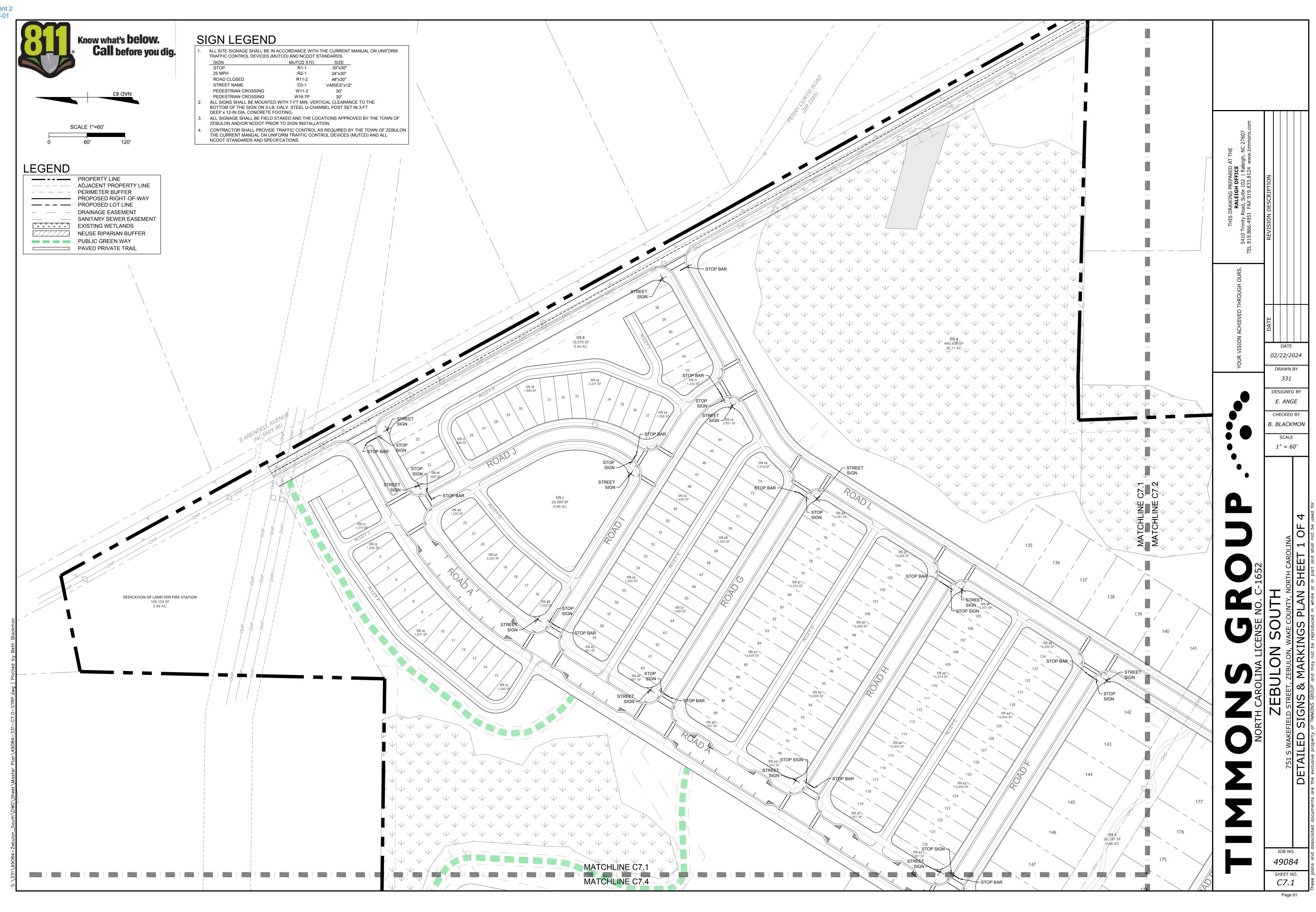




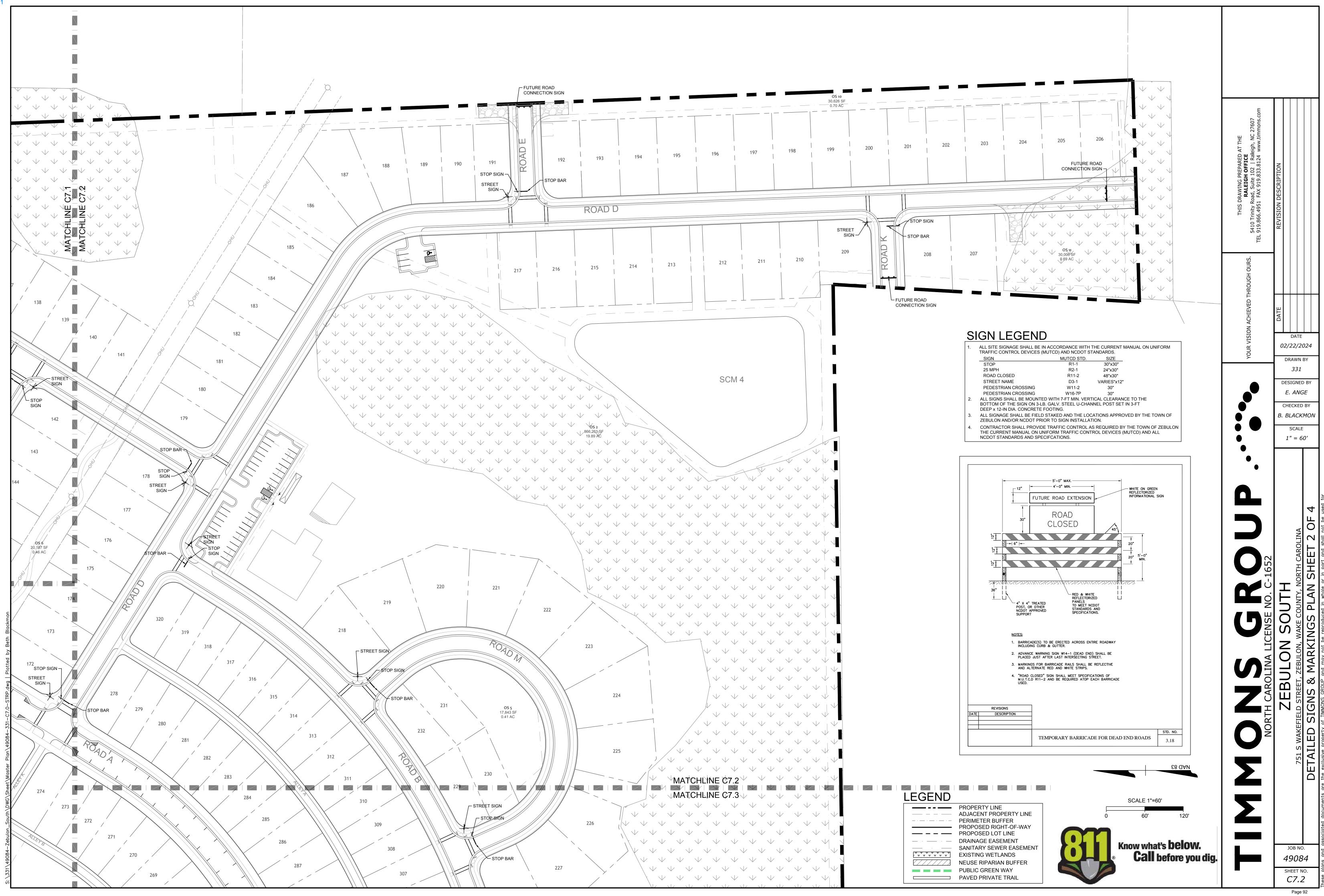


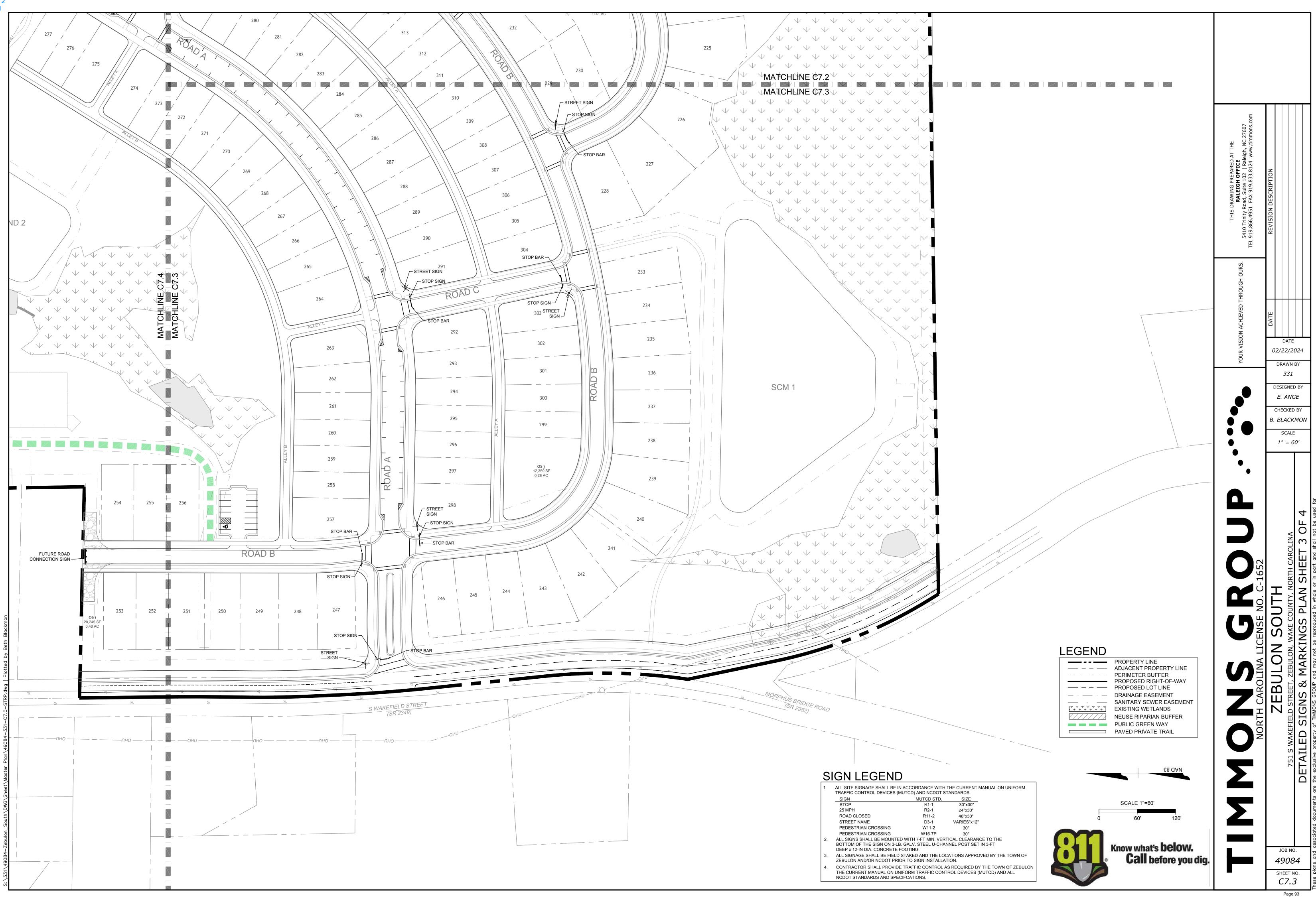


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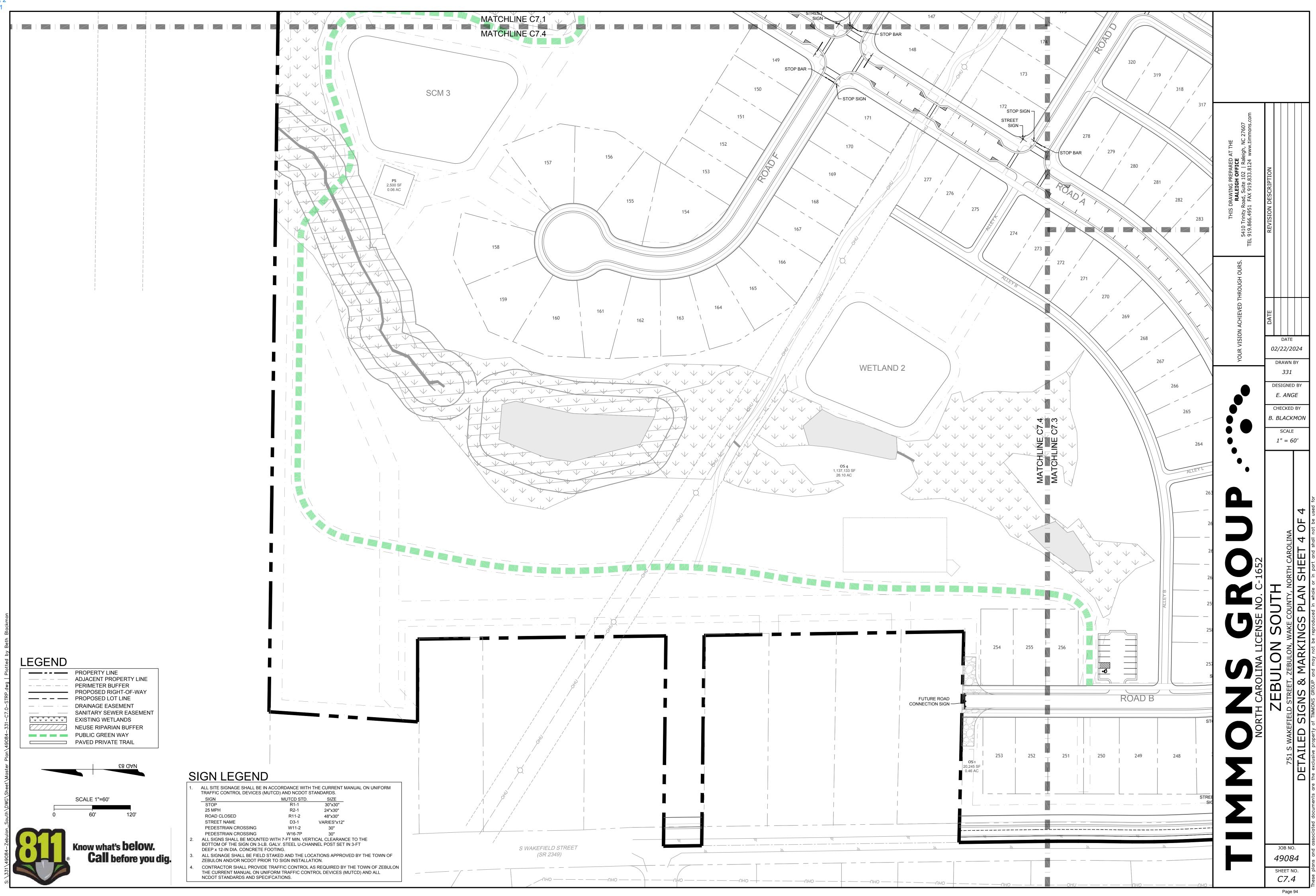






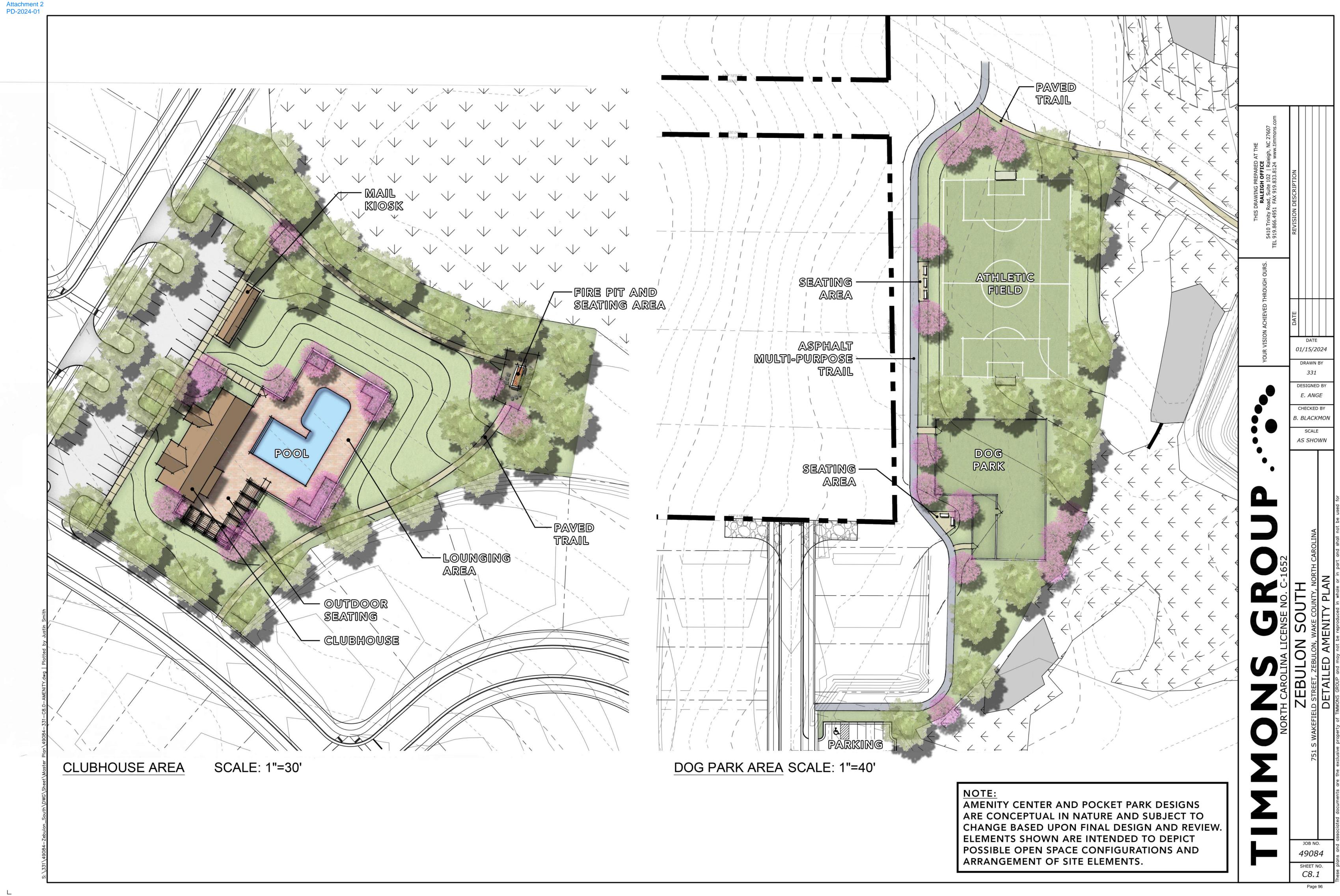








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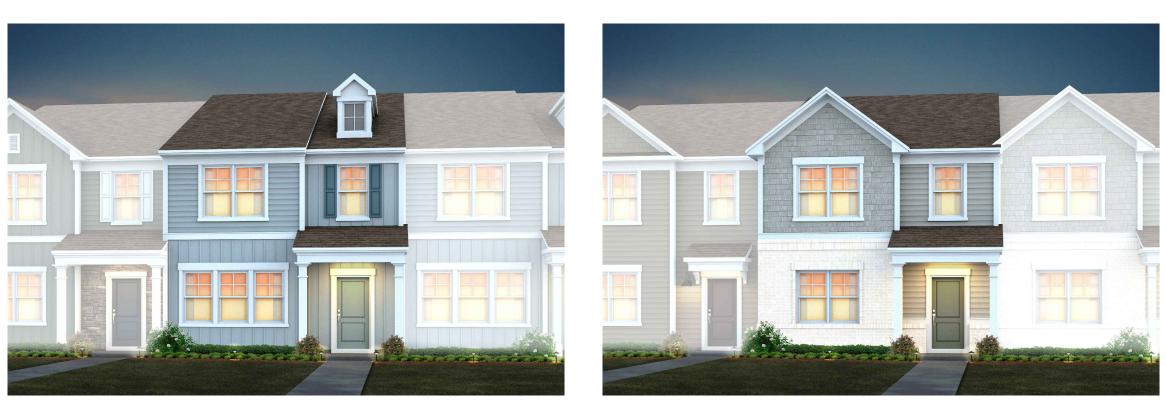






























FRONT DOOR EXAMPLES





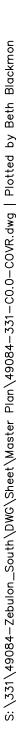






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ZEBULON SOUTH

Comprehensive Planned Development Document

715 S. Wakefield Street and S. Arendell Avenue Zebulon, North Carolina



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CIVIL ENGINEERING | ENVIRONMENTAL | SURVEYING | GIS | LANDSCAPE ARCHITECTURE | CONSTRUCTION SERVICES

Attachment 3 PD-2024-01

Zebulon South

Prepared For:

TOWN OF ZEBULON November 2022 August 2023 November 2023 January 2024 February 2024

Prepared By:

TIMMONS GROUP

5410 Trinity Road, Suite 102 Raleigh, NC 27607 301 Fayetteville Street, Suite 1400 Raleigh, NC 27601

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Developer:

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Timmons Group Project No. 49084

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Table of Contents

| 1.0 Introduction | 2 |
|---|----|
| 2.0 Vicinity Map | 3 |
| 3.0 Permitted Uses | 4 |
| 4.0 Design Controls | 4 |
| 5.0 Architectural Standards | 5 |
| 6.0 Parking and Loading | 9 |
| 7.0 Signs | 9 |
| 8.0 Infrastructure | 9 |
| 8.1 Public Water | 9 |
| 8.2 Sanitary Sewer | 9 |
| 8.3 Streets and Alleys | 10 |
| 8.4 Pedestrian Connectivity | 10 |
| 9.0 Stormwater Management | 10 |
| 10.0 Natural Resources and Environmental Data | 11 |
| 11.0 Pocket Parks and Open Space | 11 |
| 12.0 Homeowner's Association | 12 |
| 13.0 Residential Lot Landscaping | 12 |
| 14.0 Consistency with Comprehensive Plan and Land Use Map | 12 |
| 15.0 Compliance with the UDO | 13 |
| 16.0 Preliminary Residential Plan Review | 13 |
| 17.0 Additional Zoning Conditions | 13 |
| 18.0 Fire Station Land Dedication | 15 |
| 19.0 Transportation Impact Analysis Summary | 15 |

1.0 Introduction

Zebulon South is a proposed residential community utilizing the Town of Zebulon Planned Development Zoning as outlined in the Unified Development Ordinance (UDO). The assemblage is made up of three parcels with frontage on both S. Wakefield Street and S. Arendell Avenue. The total existing tract area is about 118.6 acres.

The development is bordered to the north by an existing solar farm. To the east across S. Arendell Avenue are existing single-family homes and a church. South and southeast of the property are existing single-family homes and agricultural fields as well as wooded properties. Existing single-family homes and agricultural fields are located west of the property.

Zebulon South PD will be a phased development of a high-quality master planned community that will provide a variety of housing types and amenities. By utilizing the PD zoning, the master plan containing site specific regulations will guide the development resulting in a well-integrated mix of housing types, lot sizes and densities with open space and preservation of environmentally sensitive areas. The flexibility offered by a PD zoning will result in a more efficient use of the land and network of utilities and streets.

The proposed community will consist of single family detached and attached residential development. The current zoning is R-2 and R-4. The Grow Zebulon Comprehensive Land Use Plan (LUP) designations are General Residential (GR) and Suburban Residential (SR) for the property. The LUP specifically identifies a PD as being a primary land use type in the SR and GR LUP classifications. The development will have an integrated mix of housing types, consistent with the LUP's recommendations for providing a diverse stock of residential choices. The diverse housing choices will promote varied price points, consistent with the LUP. The proposed zoning is Planned Development (PD) which is consistent with the LUP designations.

With over 50% open space (five times the minimum requirement) and at least 15% tree save (three times the minimum requirement), Zebulon South preserves a significant amount of environmentally sensitive areas in the design of the Master Plan, consistent with Land Use and Development Policy E, General Policies G1 and G6, Residential Policy R4, and Parks and Open Space Policy P5. The preserved areas, to a large degree, are located on the perimeter of the development, allowing for concentration of infrastructure improvements in the central area of the development. The Master Plan utilizes existing wetlands, open space and larger lots as buffer for the development, mitigating effects of the development on the surrounding community. Open space, common amenities and an integrated system of walking trails, including a publicly accessible greenway, support a high quality of life for the residents in and around the development. Open space shall exceed the Town of Zebulon minimum requirement for PD zoning.

The development includes the construction of a collector street connecting S. Wakefield Street and S. Arendell Street as shown on the Grow Zebulon Comprehensive Transportation Plan, at a location where it will clearly be a safe distance from the Perry Curtis/S. Arendell intersection, improving connectivity for the community. This is consistent with Land Use and Development Goal 3, Land Use and Development Policy G, General Policy G3, and Residential Policy R3. The internal street network includes three access points to existing public roads promoting access options for residents of the development. Finally, the proposed project will provide an additional benefit for all current and future area residents: dedication of land for a needed second fire station in the growing southern Zebulon. The location of the land dedication is shown on the master plan, is centrally located to serve growth that is already occurring in Southern Zebulon and will be an important step for the development of Zebulon's second fire station.

2.0 Vicinity Map

Zebulon South PD is located between S. Wakefield Street and S. Arendell Ave as shown on the vicinity map in Figure 1.



FIGURE 1

Zebulon South Timmons Group #49084 PPAB 10563380v1 Page 3 of 16 February 2024

3.0 Permitted Uses

Zebulon South PD proposes to allow the development of residential uses including Single Family Detached and Single Family Attached along with accessory uses as permitted in the R6 zoning district. Figure 2 below provides a listing of the proposed permitted uses. The uses are subject to the regulations of the Town of Zebulon UDO.

| Use Category | Specific Use | PD |
|------------------|---|----|
| Residential Uses | Single Family Detached and permitted accessory uses | Р |
| | Single Family Attached and permitted accessory uses | Р |

FIGURE 2

In addition, on the portion of the Master Plan designated as the "Dedication of Land for Fire Station," all uses permitted in R6 shall be permitted, including the "Fire/EMS/Police Station" use.

4.0 Design Controls

Development Area - 118.6 acres

Density:

| Maximum Density: | 2.95 dwelling units per acre |
|------------------|---|
| Units: | 350 dwelling units |
| | (maximum 210 SF detached & 140 SF attached) |

Building Height:

Maximum Building Height / # of stories: 50 feet / 3 stories

Building Setbacks:

| Single Family Detached Front Loaded: | | | | |
|--------------------------------------|-------------------------|---------|--|--|
| - | Front: | 20' | | |
| | Side: | 5' | | |
| | Corner Side: | 10' | | |
| | Rear: | 15' | | |
| Single Famil | y Detached Rear Loaded: | | | |
| | Front: | 10' max | | |
| | Side: | 3' | | |
| | Corner Side: | 10' | | |
| | Rear: | 20' | | |
| Single Family Attached Rear Loaded: | | | | |
| | Front: | 10' max | | |
| | Building Separation: | 10' | | |
| | Rear: | 20' | | |
| | | | | |

Attachment 3 PD-2024-01

Buffers:

| Streetscape Buffers: | 15' Type C Streetscape Buffer (Wakefield St) 15' Modified Type D* Streetscape Buffer (Arendell Ave) *For additional opacity, this buffer shall be comprised of 100% evergreen shrubs and 75% evergreen understory trees |
|----------------------|--|
| | |

Perimeter Buffers: 20' Type B Perimeter Buffer

Existing vegetation shall be utilized to the extent possible. All streetscape and perimeter buffers shall be provided in accordance with the Town of Zebulon UDO. Landscaped perimeter and street buffers shall include native and adaptive species only.

Disturbance within the buffer is only allowed as follows:

- a. Construction of driveways, public streets and walkways perpendicular to the buffer strip shall be allowed where such construction is necessary for safe ingress and egress to the site. The nature and limits of such construction must be designated on the approved master subdivision plan.
- b. Notwithstanding any other provision pertaining to buffers, City of Raleigh public utilities and easements shall be allowed, parallel and otherwise, within buffers, and the area within such easements shall still count towards buffer and undisturbed buffer calculations.

5.0 Architectural Standards

To encourage multiple architectural styles, buildings will be any variety of Craftsman, Traditional, Colonial, etc. While each of the architectural offerings proposed will have its own identity, a number of common threads will link the homes in the development, including color palettes, materials, roofing, and decorative garage doors. Elevations have been included in an effort to represent the bulk, massing, scale and architectural style of the development.

Requirements for All Homes:

Roofs:

Roof lines shall vary to reduce the scale of the structure and add visual interest. Roof shapes (flat, hip, mansard, gable, or shed for example) and material shall be architecturally compatible with façade elements and the rest of the structure. Shed roofs may be used on porches and dormers.

3-tab/235 shingles are not permitted.

Façades:

The principal structure's front façade must incorporate recesses and projections along at least 50% of the length of the façade. Windows, awnings, and porch area shall total at least 50% of the façade length abutting a public street.

Façades shall incorporate a repeating pattern of change in color, texture, and material modules.

No venting will be provided on any front facades except that when a bathroom is located on the front of the unit, a vent of a similar color to either the siding or the trim may be provided on the front of the unit.

Entryways:

Doors shall have built-in windows; alternatively, a solid door is allowed provided side lights (side windows) are installed immediately adjacent to the solid door. Double front doors are allowed as an option.

Front doors shall be illuminated.

Variations in color schemes and textures are encouraged in order to articulate entryways so as to give greater recognition to these features.

An option to include an overhang on rear exterior doors shall be provided. When this option is chosen by homeowner, the overhang shall extend at least 24 inches.

Windows:

All street-facing exterior windows shall have trim and screens. Trim shall be a minimum of 3 inches wide.

Materials and Color Palette:

Predominant exterior building materials shall be high quality materials including brick, wood, stone, fiber cement, and/or wood composite.

Vinyl siding is not permitted; however, vinyl windows, decorative elements and trim are permitted.

Trim color shall be distinct from façade color.

Front and side porches with open foundations shall have brick or stone piers and openings shall be fully screened with evergreen plantings.

A varied color palette shall be utilized on homes throughout the subdivision and shall include siding, trim, shutter, and accent colors complementing the siding colors.

Porch railings, if included on homes, shall be a complimentary color of the house and shall be made of either aluminum, or composite material.

Accessory buildings, if constructed, shall be of similar materials and colors as the primary single-family home.

All homes will have two or more of the following design features on the front façade (not including foundation):

- a. stone
- b. brick
- c. lap siding
- d. shakes
- e. board and batten
- f. window pediments
- g. recessed windows
- h. side and/or front window box bays
- i. roof gables
- j. roof dormers
- k. roofline cornices
- I. metal roofing as accent
- m. columns
- n. shutters
- o. other decorative features approved by the Planning Director

Screening:

All residential structures shall have screening by vinyl privacy fence installed on the sides or rear of the structure to prevent visibility of roll out refuse carts from the public right-of-way or adjacent properties.

Vegetative screening for HVAC units shall be provided.

Requirements for Single-family Detached Front Loaded Homes:

- 1. Each home shall have a minimum of one story and a maximum of three stories.
- 2. Each home may have a raised slab foundation. Raised slab foundation shall contain stone or brick.
- 3. Finished floor elevations shall comply with UDO Section 4.3.3.P.1.
- 4. UDO 4.3.3.P.2 Single-family detached dwellings shall be configured so that each side of the dwelling includes some form of ingress or egress capable of allowing emergency exit from or entrance into the dwelling. Windows, doors, or other wall penetrations shall be credited towards these standards. Skylights shall also be credited towards these standards in cases where there is sufficient access to the ground from the room.
- 5. Front porches shall extend beyond the front plane of the garage by a minimum of 12" on 25% of the homes constructed. Front Porches shall be allowed to extend beyond the minimum front setback a maximum of 10".
- 6. Garage doors must have windows, decorative details or carriage-style hardware.
- 7. Each garage will either have one light on each side or two lights above the garage door.
- 8. Eaves shall project at least 8 inches from the wall of the structure.
- 9. All gutter downspouts shall discharge to the side or rear of the structure.
- 10. The front elevation and all sides that abut a public street shall contain a minimum of 10% masonry (brick or stone) and shall contain a minimum of two siding materials (i.e. stone and hardiplank or brick and shake).

- 11.A minimum 18-inch masonry (brick or stone) water table on the front façade shall be provided.
- 12.On at least 30% of units, masonry (brick or stone) shall extend the full height of the ground floor.
- 13. Each front porch shall contain a covered stoop.
- 14.No single family detached home shall be constructed with a front elevation or color palette that is identical to the home on either side of it.

Requirements for Single-family Detached Rear Loaded Homes:

- 15.Each home shall have a minimum of one story and a maximum of three stories.
- 16.Each home may have a raised slab foundation or crawl space. Raised slab foundation shall contain stone or brick.
- 17. Finished floor elevations shall comply with UDO Section 4.3.3.P.1.
- 18.UDO 4.3.3.P.2 Single-family detached dwellings shall be configured so that each side of the dwelling includes some form of ingress or egress capable of allowing emergency exit from or entrance into the dwelling. Windows, doors, or other wall penetrations shall be credited towards these standards. Skylights shall also be credited towards these standards in cases where there is sufficient access to the ground from the room.
- 19. Eaves shall project at least 8 inches from the wall of the structure.
- 20.All gutter downspouts shall discharge to the side or rear of the structure.
- 21. The front elevation of each unit shall contain a minimum of 10% masonry (brick or stone) and shall contain a minimum of two siding materials (i.e. stone and hardiplank or brick and shake).
- 22.A minimum 24-inch masonry (brick or stone) water table on the front façade shall be provided.
- 23.All sides of a principal structure that face an abutting public street shall have architectural and decorative features as described above.
- 24.No single family detached home shall be constructed with a front elevation or color palette that is identical to the home on either side of it.

Requirements for Single-family Attached Rear Loaded Homes:

- 25. Each home shall have a minimum of two stories and a maximum of three stories.
- 26.Each home may have a raised slab foundation.
- 27. The front elevation and all sides that abut a public street shall contain a minimum of 10% masonry (brick or stone) and shall contain a minimum of two siding materials (i.e. stone and hardiplank or brick and shake).
- 28.No two consecutive units within a single building shall contain the exact same front elevation regarding materials or color palette.
- 29.All sides of a principal structure that face an abutting public street shall have architectural and decorative features as described above.
- 30. The building façade cannot be a single mass; it must be broken up by home articulations of at least 12 inches, at minimum, between every two homes.
- 31. The roofline of each attached building cannot be a single mass; it must be broken up either horizontally and/or vertically between, at a minimum every two homes.

Requirements for Amenities:

A mail kiosk shall be located adjacent to the clubhouse and pool. The kiosk shelter shall be designed with similar architectural style, materials and color palette as the homes in the neighborhood. Cluster mailboxes shall meet the requirements of Section 6.12.7 of the Town of Zebulon UDO.

The clubhouse shall match residential buildings with regard to style, materials and color palette.

Architectural Guidelines:

We commit to the architectural requirements in Section 5.2.4 of the UDO. We will work with Town Planning and Building staff to provide additional architectural features with the exception of Section 5.2.4.E.3.e. Garage doors will not be required to be located at least two or more feet behind the front porch or the primary entrance to the dwelling.

6.0 Parking and Loading

All parking and loading areas shall comply with applicable requirements of the Town of Zebulon UDO Section 5.8. Guest parking shall be constructed for each phase to meet the requirements of that phase, and the total number of guest parking spaces shall exceed the minimum requirement by 76 spaces, which is almost double the requirement.

7.0 Signs

All signage shall comply with applicable standards and requirements of the Town of Zebulon UDO Section 5.11.

8.0 Infrastructure

8.1 Public Water

Public water will be provided via extensions of the existing City of Raleigh water system. Existing water is located in S. Arendell Avenue closer to the Town of Zebulon near Temple Johnson Road. Water infrastructure will be extended from the current terminus and along the site frontage of S. Arendell Avenue and S. Wakefield Street. Infrastructure shall be extended throughout the site as required for development to provide public water to all lots.

8.2 Sanitary Sewer

Public sanitary sewer will be provided via extensions of the existing City of Raleigh sanitary system. A pump station will be constructed on site. Existing gravity sanitary sewer is located north of the site closer to the Town of Zebulon near Temple Johnson Road. The forcemain connection will be made to this existing gravity sewer. The gravity sanitary sewer infrastructure will be extended throughout the site as required.

Page 9 of 16 February 2024



8.3 Streets and Alleys

All streets shall be in conformance with the Town of Zebulon Transportation Plan and shall be constructed to Town of Zebulon standards and specifications. The project proposes an 70' right-of-way two-lane collector street with on-street parking protected by bump-outs and 10' multiuse paths on either side for additional pedestrian and cyclist safety, and has has been shown on the Master Plan connecting S. Wakefield Street and S. Arendell Avenue. This section will create the connectivity envisioned in the CTP, with a cross section that will fit in the proposed neighborhood. The collector street construction shall follow the phasing of the project. Each section shall be constructed within the phase which it is located.

The ultimate cross section of S. Arendell Avenue is an 80' right-of-way 2-lane median divided roadway. The ultimate cross section of S. Wakefield Street starts as an 80' right-of-way 2-lane median divided roadway that transitions on the south side of the proposed collector street to a 100' right-of way 4-lane median divided roadway. This project shall construct half of the cross-section along the property frontage. The Parks and Recreation Master Plan proposes a greenway along each of these roads. These greenways shall be incorporated as a 10' mixed use path along the roadway in place of a traditional 5' sidewalk.

Alleys shall be located within a 20' right-of-way with 10' of asphalt pavement width.

8.4 Pedestrian Connectivity

Zebulon South has over 6 miles of greenways, multiuse paths, trails, and sidewalks. Sidewalks shall be provided on both sides of all streets throughout Zebulon South PD. Alleys shall not have sidewalks. Multiuse paths will also be provided on Wakefield St and S Arendell Ave, and will connect the greenway to Wakefield St. Multiuse paths are provided on both sides of Road A. The neighborhood sections shall also be connected for pedestrians by several 6' paved private trails, which will include at least three (3) exercise stations along the trails.

The public greenway shown on the Town's Comprehensive Transportation Plan shall be constructed through the site along the north side of the development. A private trail shall connect the sidewalk system to the public greenway approximately as shown on the Master Plan.

9.0 Stormwater Management

The proposed development will meet all applicable requirements and standards as outlined in the Town of Zebulon Street and Storm Drainage Standard and Specifications Manual. Zebulon South PD will meet all stormwater quantity and quality reduction requirements. Proposed stormwater control measures (SCMs) will typically consist of wet ponds and other approved measures. SCMs will be located within open space areas and be maintained by the HOA. At least one stormwater control pond shall contain a fountain. At least seventy-five percent (75%) of any required plants in the Stormwater Control Measure ponds, excluding grasses, shall be pollinator plants such as native milkweeds and other nectar-rich flowers.

10.0 Natural Resources and Environmental Data

The development site consists mostly of agricultural fields along with wooded areas surrounding the streams, wetlands and existing pond.

Existing streams and wetlands have been delineated and buffered as required by Town, State and Federal agencies. The site is located within the Neuse River Basin. Any impacts requiring permits shall be obtained and permitted through the Town of Zebulon, NC Division of Water Resources and US Army Corps of Engineers as applicable.

No special flood hazard areas are located onsite per FEMA FIRM Map 3720270500k & 3720270400L dated 7/19/2022.

11.0 Pocket Parks and Open Space

Active and passive open spaces and recreational features will provide the residents with excellent on-site amenities. An integrated system of walking trails traverses open space and environmentally sensitive areas providing a unique amenity for the development. The development provides over eleven acres of open space, including over five acres of active open space. An on-site swimming pool and clubhouse provide pedestrian accessible amenities for the residents of the development.

Pool:

• Minimum 1,000 square foot water surface area

Clubhouse:

• No meeting space, bathrooms and changing rooms only

Tot Lot:

- Minimum 600 square feet including ASTM fall zones
- IPEMA Certified Playground Equipment
- Target age: 2-12 years

Yard Games:

- At least one amenity area shall provide a yard game, such as cornhole or outdoor ping pong
- Target age: 12+

Dog Park:

- Minimum 6,000 square feet of fenced area
- Fence shall be a minimum of 4' tall galvanized or vinyl-coated chain link fence
- Shall include a minimum of two benches, one trash can and one dog waste station

Pocket Park:

- Minimum of 8,000 square feet of area for multi-purpose play
- May include benches, paths, trashcans and enhanced landscaping
- Located adjacent to the greenway, along the Arendell/Wakefield St connector
- Pollinator Garden

Private Trails:

• Minimum 6' wide paved trail connecting from sidewalk system to Public Greenway as illustrated on Master Plan. At least two (2) dog waste stations and three (3) exercise stations will be provided along trails.

12.0 Homeowner's Association

Prior to the issuance of the first certificate of occupancy for the development, a Homeowner's Association (HOA) shall be formed to govern the affairs of the development. The HOA shall be responsible for maintaining the common areas of the development including any shared stormwater facilities, landscaping, hardscape structures (such as signage, irrigation, lighting, and fountains) and recreation amenities. The Homeowners Association shall appoint one resident to the advisory board at 25% resident occupied, one resident at 50% occupied and one resident at 75% occupied.

13.0 Residential Lot Landscaping

Individual residential lots shall be landscaped per Town of Zebulon UDO for foundation plantings and site landscaping.

Foundation plantings consisting of evergreen shrubs or decorative grasses with a minimum heigh of 18 inches shall be located within 10 feet of any foundation wall visible from a public street excluding alleys. Shrubs shall maintain a maximum on-center placement of three feet.

Site landscaping consisting of one canopy tree for every 2,000 square feet of lot area is required. Canopy trees may be located anywhere within the residential lot except where limited by easements, sight distance triangles or buffer areas.

HVACs and ground-based mechanical equipment shall be screened utilizing evergreen shrubs on sides visible from a public street.

14.0 Consistency with Comprehensive Plan and Land Use Map

Zebulon South PD is consistent with the Town of Zebulon Comprehensive Plan and Land Use Map goals and objectives. The development is located in SR and GR land use categories where PD zoning is a recommended land use type particularly where a mix of housing types and varying densities is proposed. Proposing both single-family detached and attached product supports the Town's desire for a variety of housing types and price points. This draws new residents and provides additional housing choices for existing residents.

The site design incorporates in a variety of lot sizes supporting the goal of increasing a diverse housing stock for the Town. The variety ensures additional housing choices as well as a variety of price points.

Providing more concentrated development while preserving environmentally sensitive areas and perimeter buffers provides a transition to the existing single-family homes and agricultural properties adjacent to the development.

The integrated system of streets, sidewalks, trails and greenways provide a cohesive pedestrian and vehicular network adhering to the Town's Comprehensive Transportation Plan and provided a thoughtfully planned neighborhood.

15.0 Compliance with the UDO

This Master Plan shall be the primary governing document for the development of Zebulon South PD. All standards and regulations in this Master Plan shall control over general standards of the UDO. Provided, however, that if a specific regulation is not addressed in this Master Plan, UDO regulations shall control. Zebulon South PD will comply with all other relevant portions of the Town of Zebulon Unified Development Ordinance.

16.0 Preliminary Residential Plan Review

Pursuant to UDO Section 3.5.5.B.4, the applicant requests an exemption from subsequent residential preliminary plan review. This PD includes a master plan that is detailed and meets the requirements for a residential preliminary plan. Therefore, upon approval of this PD, the applicant shall be exempt from subsequent residential preliminary plan review.

17.0 Additional Zoning Conditions

In addition to conditions contained throughout the visual and written document, additional written voluntary conditions have been offered to ensure a quality development.

- 1. Uses shall be limited to single family detached, single family attached, and accessory uses as permitted in the R6 zoning district.
- 2. Minimum driveway stem length shall be 20'.
- 3. Single family detached rear load lots shall have a minimum lot size of 4,800 sf.
- 4. Single family detached front load lots shall have a minimum lot size of 6,000 sf.
- 5. Single family attached lots shall have a minimum lot size of 1,260 sf.
- 6. The minimum lot width for front loaded lots shall be 50' reduced from 70'.

- 7. All single family detached rear loaded homes shall have a sidewalk connection from the front door or porch to the public sidewalk.
- 8. The clubhouse and pool shall be completed before the 150th Certificate of Occupancy for any dwelling is issued.
- 9. Zebulon South will apply a maximum 35% impervious requirement for the development as a whole (based on total acreage).
- 10. The applicant commits to provide a minimum 15% Tree Save, three times the minimum requirement.
- 11. All planned improvements to roadways and right-of-way owned and maintained by the NC Department of Transportation (NCDOT), including improvements that require off-site property acquisition and/or easements, are subject to NCDOT approval during subsequent phases of development. If any improvements are not approved by NCDOT, alternative designs may be administratively approved by Town staff.
- 12. If a bus pickup location is approved by Wake County Public Schools in the neighborhood, one bust stop area, including a shelter, a bench, a trash can, and at least 5 bicycle spaces shall be provided with the second phase of development.
- 13. To support community gatherings and active neighborhoods, the applicant commits to providing one neighborhood congregation area, to include:
 - a. a minimum of two (2) larger parking spaces designed for food trucks or delivery vehicles (mobile vendors), with an electrical outlet available;
 - b. one (1) covered seating area with at least 10 designated public seating spaces will be provided adjacent to the Mobile Vendor spaces.
 - c. at least one (1) outdoor grill will be provided adjacent to the covered seating area,

This area may be classified as active open space under UDO § 5.7.

- 14. The development shall include a minimum of three (3) affordable single-family detached ownership units (the "Affordable Units"), which shall be located on different streets in the neighborhood. The Developer shall vet qualified buyers for the Affordable Units and shall ensure, in the first sale of the Affordable Units, that they are affordable households earning no more than eighty percent (80%) of the Area Median Income (AMI). Prior to Subdivision closeout, the Developer shall submit documentation of compliance with this zoning condition. Following the first sale of each of the Affordable Units, Developer shall have no further obligations under this condition.
- 15. Final alignment of the greenway will be reviewed and approved by TRC during construction drawings.
- 16. Unless not approved by Wake County, the grave site(s) located at 0 N Arendell Ave (PIN 2705513114) shall be relocated prior to approval of construction drawings.

18.0 Fire Station Land Dedication

The owner shall designate, for the benefit of the Town, a minimum of 2 acres of land (including any existing and future right-of-way) located on Hwy 96/Arendell Ave abutting the property lines of Wake County PINs 2705410911 (Deed book 16651/page 25) and 2705520074 (Deed book 9289/page 1838) for the future development of a Town of Zebulon Fire Station (the "Fire Station Land"). The location of the Fire Station Land is identified on the Master Plan, and the surveyed boundary line will be identified and approved by the Town prior to Final Plat approval for the phase of development adjacent to the Fire Station Land. After approval of the boundary line, but no later than the approval of the Final Plat for the phase of development adjacent to the Fire Station Land, the developer shall dedicate to the Town an easement (or superior title) by deed or other instrument, approved by the Town Attorney as to form, for the Fire Station Land. The actual design, construction, and installation of the fire station and associated amenities, or other permitted public use as determined by the Town, shall be done by the Town, or another public entity and are not commitments of this zoning case. This zoning condition may also be satisfied by the Town Manager, or his/her designee, stating, in writing, that the Town does not wish to accept the land for the development of a fire station or other public use.

19.0 Transportation Impact Analysis Summary

A Traffic Impact Analysis (TIA) was conducted by the Timmons Group in accordance with the Zebulon (Town) Unified Development Ordinance (UDO) and the North Carolina Department of Transportation (NCDOT) capacity analysis guidelines. A full copy of the TIA was submitted for review and approval with the PD submittal, and a voluntary additional analysis incorporating anticipated traffic from the nearby recently-approved Chamblee Lake PD was submitted for review and approval with the applicant's final master plan submittal. The listed recommended improvements are subject to further review and final approval by NCDOT.

<u>Study Area</u>

The study area for the TIA was determined through coordination with the Town and NCDOT and consists of the following existing intersections:

- NC 96 and Site Access #1
- NC 96 and Site Access #2
- S Wakefield St and Site Access # 3
- NC 97 and S Wakefield St
- NC 97 and NC 96
- NC 96 and W Barbee St
- NC 96 and Perry Curtis Rd

Recommended Improvements

Based on the analysis of the TIA (including improvements to be installed by the Chamblee Lake development), the following improvements have been recommended to be constructed by the developer to both mitigate traffic impacts by the proposed development.

NC 96 and Site Access #1

- Site Access 1 to include a two-lane cross-section with one eastbound egress lane and one westbound ingress lane.
- Provide stop control on the eastbound approach of the site drive.
- Construct a southbound right-turn lane on NC 96 with a minimum of 50 feet of fullwidth storage and appropriate deceleration and taper.
- Construct a northbound left-turn lane on NC 96 with a minimum of 100 feet of fullwidth storage and appropriate deceleration and taper.

NC 96 and Site Access #2

- Site Access 2 to include a two-lane cross-section with one eastbound egress lane and one westbound ingress lane.
- Provide stop control on the eastbound approach of the site drive.
- Construct a southbound right-turn lane on NC 96 with a minimum of 50 feet of fullwidth storage and appropriate deceleration and taper.
- Construct a northbound left-turn lane on NC 96 with a minimum of 100 feet of fullwidth storage and appropriate deceleration and taper.

Wakefield St and Site Access #3

- Site Access 3 to include a two-lane cross-section with one westbound egress lane and one eastbound ingress lane.
- Provide stop control on the westbound approach of the site drive.
- Construct a southbound left-turn lane on Wakefield Street with a minimum of 50 feet of full-width storage and appropriate deceleration and taper.

ZEBULON

MUNICIPAL UTILITY ALLOCATION POLICY

Statement of Purpose and Goals

Introduction

Drinking water supplies throughout the greater City of Raleigh distribution system are finite, subject to disruption by drought and/or other calamity and Zebulon's allocation is contractually limited. The Town staff, the Planning Board, and the Board of Commissioners have given a great deal of thought and study as to the best utilization of this valuable resource to benefit current and future citizens.

The Town of Zebulon's municipal water and sewer capacity is a valuable resource that must be conserved and apportioned to new development projects that promote the Town's policy of ensuring a diversified tax base and housing supply. Such an allocation policy will tend to promote diversity of housing available to a wide cross section of citizens of diverse socio-economic backgrounds and promote economic viability and sustainability by providing for retail and other commercial development within the Town of Zebulon.

In order to preserve and enhance property values, manage its limited water supply as a vital natural resource, promote economic development, and incentivize smart growth practices, the allocation of Zebulon's potable water capacity shall hereafter be in accordance with this policy.

Land Use and the Tax Base

The local government expense of providing fire and police protection, schools, parks, social services, water and sewage systems and other essential public services to residential neighborhoods is generally greater than the ad valorem tax revenue generated by such neighborhoods. On the other hand, the cost of providing services to commercial and industrial development is generally less than the tax revenue accruing to the local government. Having a predominantly residential tax base would require the Town of Zebulon over time to assess a higher tax levy to raise funds to provide essential services or to reduce the level of public services provided. This is one reason among many why local governments including Zebulon strive to achieve a balance of both residential and non-residential growth.

Zebulon's historical development is transitioning from industrial to residential, leading to a current tax base of approximately 40% residential and 60% commercial/industrial. The following table shows Zebulon's tax base over the past five years ¹

| Fiscal Year | Commercial | Residential |
|-------------|------------|-------------|
| 2021-2022 | 60% | 40% |
| 2020-2021 | 65% | 35% |
| 2019-2020 | 72% | 28% |
| 2018-2019 | 73% | 27% |
| 2017-2018 | 71% | 29% |

Zebulon Tax Base (Past Five Years)

¹ "Tax Base Components | Wake County Government," Wake County North Carolina,

https://www.wakegov.com/departments-government/tax-administration/data-files-statistics-and-reports/tax-base-components

As shown in the table above, the residential tax base has steadily increased proportionally over the past five years. This trend in the tax base data, combined with the vested planned residential development in the coming years, demonstrates the need for the Town to address this shift through policy. The Zebulon Board of Commissioners believes that it is fiscally responsible and otherwise in the public interest to promote and encourage non-residential development in the jurisdiction as an alternative to rapid residential development to keep the ratio between the two development types well balanced. A goal of maintaining a tax base of 60% residential and 40% commercial/industrial is hereby established.

Development Goals for the Full Build-Out of Zebulon

Communities without a wide variety of housing types and styles also put pressure on the Wake County Public School System which remains committed to having students of a wide range of socio-economic backgrounds attend each local school. In addition to the goal of maintaining a balanced tax base, the Town of Zebulon is committed to achieving a balance of housing types within its jurisdiction.

This commitment is consistent with both the Town's Strategic Plan and Comprehensive Plan. The *Town of Zebulon: Vision 2030 Strategic Plan* lists "Growing Smart" as one of its three focus areas, calling for the planning of appropriate land uses and affordability of the community. The *Grow Zebulon Comprehensive Land Use Plan* identifies six guiding principles for the town. Two of those principles are "Zebulon will be BALANCED" and "Zebulon will be PRUDENT." A balance should be achieved for the Town's tax base, its land uses, and its housing types to allow for an affordable community with employment and business opportunities that will help the community prosper. The achievement of balance in Zebulon will contribute to the Town being prudent. As stated previously, a local government's cost of providing services to commercial properties is generally less than that of residential properties. Having a balanced tax base that is not proportionally over-saturated with residential properties will contribute to keeping the Town financially sound.

Below are three development goals that are integral to the utility allocation policy and the future of the Town. These development goals apply to the entire, future Zebulon jurisdiction including the ETJ, short-range and long-range urban service areas.

GOAL #1: Maintain 60%-40% ratio of residential to non-residential tax values.

| Upon Adoption-January 2021 | |
|---------------------------------------|--|
| 60% Residential - 40% Non-Residential | |

GOAL #2: Residential Housing Percentage Breakdown SFD|TH|MF - 75%|10%|15% (Note - Duplex counted as MF)

| Upon Adoption-January 2021 | |
|----------------------------|--|
| 80.5% 0.5% 19% | |

GOAL #3: Encourage Mixed Use Development to improve pedestrian connectivity to non-residential activity.

Policy and Procedures

Water Allocation

All existing parcels of real property within the corporate limits of Zebulon, regardless of proposed acreage, shape, or location as of the adoption of this ordinance are entitled to **115 gallons per day** of water allocation to build and sustain a single family or a limited business or commercial use. No additional water allocation will be awarded for proposed development except in accordance with the requirements of this policy.

Wastewater Connection

All projects considered for utility allocation must provide a wastewater system connection with adequate receiving capacity, as determined by the Wake County Health Department and/or City of Raleigh Public Utilities Department and approved by the Town of Zebulon Planning Director.

General Conditions & Requirements

- All proposed projects must be within the existing corporate limits or have filed a valid and complete petition for Voluntary Annexation.
- All proposed projects under consideration must have a complete application submitted for the appropriate Master Plan, Subdivision, Site Plan, Special Use Permit, Conditional Zoning Request, Zoning Compliance Permit, Building Permit, or any other necessary approval.
- All projects are subject to a Utility Allocation or Developer's Agreement approved by the Town's Board of Commissioners. If the Developer/Applicant fails to meet all terms of that agreement the unused allocation will be reclaimed, no new building permits will be issued, and no new connections to the water or wastewater systems will be permitted. Active building permits will have certificates of occupancy held until mitigating measures are agreed to by all parties.
- Projects with proven vested rights upon adoption of this ordinance will be permitted to finish their projects as previously approved.
- Public water may be utilized for irrigation purposes so long as the Primary Use associated with the site has previously gained water allocation through the Town.
- Any third parties who buy land to build upon are bound by the approved Utility Allocation Agreement or Development Agreement for that property. If the agreement is not fulfilled, the above terms and conditions still apply regardless of who owns the land.

Compliance Required

This policy allocates municipal water in gallons per day for new development proposals, master plans, site plans, building plans, and/or structures seeking construction approval. Each phase of a phased development must comply with the terms and development schedule of an approved Utility Allocation Agreement before the next phase can begin or the development risks loss of previously reserved allocation.

Previously dedicated but unused allocation can be reclaimed by the Town's Board of Commissioners for:

- (1) the lack of compliance with any existing Utility Allocation or Developer's Agreement;
- (2) violation of applicable town policy provision, ordinance standard, condition of approval;
- (3) violation of federal or state regulation; or
- (4) other good cause.

Attachment 4 PD-2024-01

Utility Allocation Application Process

Upon receiving a new development proposal requesting water capacity, the Planning Staff shall direct the Developer/Applicant to demonstrate the project's qualifications. A Developer/Applicant shall state on the appropriate application, and stipulate within an approved Utility Allocation Agreement, the use or uses proposed to be built as part of the project along with the construction design and materials. Town action on the request will be deferred until the application is complete and the requested information has been provided.

Proposed projects shall complete the UTILITY ALLOCATION WORKSHEET according to its instructions to determine the total number of points achieved. The Utility Allocation Application package will be reviewed for completeness and compliance by the Technical Review Committee (TRC) in conjunction with the applicable development approval for the subject property (conditional rezoning, planned development, site plan, etc.).

Qualification for water allocation is judged by:

- The level of developer investment
- Anticipated increases in the Town's ad valorem tax base
- Construction and dedication of public infrastructure
- Provision of employment opportunities for Zebulon citizens
- Provisions of diversified housing stock
- Preservation of open space
- Protection of existing tree canopy
- Conservation of existing habitat
- The provision of recreational amenities for current or future Zebulon residents

Projects must be awarded 60 TOTAL POINTS or more to merit water allocation.

Points are awarded in two categories, BASE POINTS and BONUS POINTS. BONUS POINTS are broken down into six categories.

- 1. Nonconformity Abatement and Public Infrastructure Improvements
- 2. Green Development Standards
- 3. Gateway and Transit Improvements
- 4. Amenities
- 5. Affordable Housing
- 6. Other

Unless a project can gain all necessary BONUS POINTS from a single improvement identified in the approved list, improvements must be made from at least two of the categories of BONUS POINTS.

All features and/or improvements that earn a projects BONUS POINTS must be clearly shown on a development plan for each application type.



Expiration of Allocation Award

A developer/applicant who has secured allocation according to this policy and hasn't progressed in construction plan approval, building permit approval, or on-site construction for a period of 12 months will lose the award of allocation without benefit.

Annual Review of Policy & Appeals

This policy shall be reviewed in January of each year and, when appropriate, readjusted by the Town's Board of Commissioners. The Town's overall progress on policy goals will be considered and the multipliers and/or point thresholds readjusted accordingly.

Appeals of any provision of this ordinance shall be decided upon by the Town's Board of Commissioners upon receiving a recommendation from the Planning Board.

BASE POINTS: List of Preferred Land Uses and Required Characteristics:

The uses listed below have been determined to be the most desirable and important uses for the Town of Zebulon to promote and maintain economic and housing diversity. Only projects that completely meet the stated performance characteristics will be considered for utility allocation. Please select one of the following Base Point classifications.

| 60 Base Points | Single Family Homes (Expedited Subdivision or Recombination) Newly constructed Single Family Homes built upon new lots created via the minor subdivision, exempt subdivision, expedited subdivision (3 or fewer lots) or recombination process. |
|----------------|---|
| 60 Base Points | Change of Use This category captures renovation, rehabilitation, up-fit or retrofit of existing buildings or portions of buildings that pre-date this policy and require a code summary sheet, change in building occupancy, certificate of occupancy, building permit and/or building inspections and do not increase the utility demand from the previous use of the building. |
| 45 Base Points | Business Office/Finance/ Insurance / Professional Services Center - Large Qualifying projects must exceed 100,000 square feet of heated floor space and create at least 150 employment positions that exceed the average annual Wake County salary according to Wake County Economic Development or the Employment Security Commission. Employees perform professional, scientific, and technical services for others. Such services require a high degree of expertise and training and provide high salaried employment opportunities. Examples include software engineering, legal, medical, accounting, consulting, architectural, biomedical, chemical, research and development, and administrative services. Finance or Insurance Centers shall also pool financial risks by underwriting insurance and annuities. Some establishments support employee benefit programs. Examples include bank or credit union headquarters, brokerages, investments, insurance, financing, and data processing establishments. |
| 45 Base Points | Manufacturing/Industrial Employment Center Manufacturing or Industrial establishments in this category exceed 200,000 square feet of floor space located in plants, factories, or mills and employ power- |

| | - |
|----------------|--|
| | driven machines and materials-handling equipment. They may also employ workers who assemble or create new products by hand, without the characteristic machinery-intensive enterprise. Many manufacturing establishments process products of agriculture, forestry, fishing, mining, or quarrying as well as products of other manufacturing establishments. Most manufacturing establishments have some form of captive services (e.g., research and development, and administrative operations, such as accounting, payroll, or management) in conjunction on-site. |
| 45 Base Points | Governmental Uses/Public Administration This category encompasses centers for all government functions; it includes federal, state, and local government agencies that administer, oversee, and manage public programs and budgets and have executive, legislative, or judicial authority. Establishments develop policy, create laws, adjudicate civil and criminal legal cases, and provide for public safety and national defense. |
| 40 Base Points | Single Use Retail Newly constructed single use, stand-alone building used primarily for retail, restaurant, or similar commercial use. |
| 40 Base Points | Hotels, Motels, or other Accommodation Service Establishments This category serves lodging and short-term accommodations for travelers. They may offer a wide range of services, from overnight sleeping space to full-service hotel suites. They may offer these services in conjunction with other activities, such as entertainment or recreation. Stays in these establishments are generally less than one month. This classification does not include boarding or rooming houses. |
| 40 Base Points | Arts/Entertainment/Museums These establishments operate facilities or provide services for a variety of cultural, entertainment, and performing art functions. Establishments include those that produce, promote, or participate in live performances, events, or exhibits intended for public viewing; those that preserve and exhibit objects and sites of historical, cultural, or educational interest; and those that operate facilities or provide services to serve activities associated with the aforementioned. |
| 40 Base Points | Amusement, Sports or Recreational Establishment Establishments in this category operate either indoor or outdoor facilities offering family activities (i.e. sports, recreation, or amusement) and provide services, such as facilitating amusement in places operated by others, operating recreational sports groups and leagues. Examples include golf courses, indoor sports venues, bowling alleys, miniature golf courses, athletic clubs, skating rinks and arcades. This category may be used in conjunction with a commercial or residential development as a mixed use development. |
| 40 Base Points | Mixed Use Development (Transit Oriented) Newly constructed or substantially rehabilitated collection of vertically mixed retail, office and residential uses in multi-story buildings centered within a one- half mile radius of an existing rail or bus transit station or the intersection of |

| | Horton Street and North Arendell Avenue in Downtown Zebulon. In order to qualify as mixed use, developments must dedicate at least one-third of the total heated square footage to residential use and the remainder to a mix of retail and office uses. All three use types must be represented and at least 10% of the heated square footage must be dedicated to street level, storefront retail uses. |
|----------------|---|
| 40 Base Points | Mixed Use Development (Urban Infill) Newly constructed or substantially rehabilitated collection of mixed retail, office and residential uses in a multi-story building on a previously developed parcel within the corporate limits. In order to qualify as mixed use, developments must dedicate at least one-third of the total heated square footage to residential use and the remainder to a mix of retail and office uses. All three use types must be represented and at least 10% of the heated square footage must be dedicated to street level, storefront retail uses. |
| 40 Base Points | Mixed Use Development (Greenfield) Newly constructed collection of mixed retail, office and residential uses in a multi- story building or buildings on a previously undeveloped parcel. In order to qualify as mixed use, developments must dedicate at least one-third of the total heated square footage to residential use and the remainder to a mix of retail and office uses. All three use types must be represented and at least 10% of the heated square footage must be dedicated to street level, storefront retail uses. |
| 35 Base Points | Housing Services for the Elderly Establishments This category offers housing services for the aged, not requiring a license from the North Carolina Department of Health and Human Services, such as independent retirement housing, multi-unit assisted housing with services (MAHS), and continuing care retirement centers. All facilities must provide, but not necessarily be limited to, the following services/facilities: On-site laundry facilities, on site management, guaranteed transportation services at least four days per week, on-site exercise facilities, on-site computer access, and a clubhouse/common lounge area for all residents. |
| 35 Base Points | Mixture of Use Development (Retail/Office-Institutional/Commercial) Newly constructed collection of horizontally arranged uses including retail, office- institutional and commercial within a master planned project on a previously undeveloped parcel or parcels totaling at least 10 acres. Mixture of use projects must include at least two (2) use types with at least 25% of the space devoted to each use type included in the development. |
| 30 Base Points | Retail/Commercial Center Newly constructed center of at least 50,000 square feet, typically containing an anchor such as a grocery store and other smaller spaces and/or outparcels for subordinate uses. Uses are entirely consumer-driven and include all manner of retail, service and office possibilities. |
| 30 Base Points | Business Office/Finance/ Insurance / Professional Services Center – Medium Qualifying projects must exceed 50,000 square feet of heated floor space and create at least 75 employment positions that exceed the average annual Wake County salary according to Wake County Economic Development or the |

| | Employment Security Commission. Employees perform professional, scientific, and technical services for others. Such services require a high degree of expertise and training and provide high salaried employment opportunities. Examples include software engineering, legal, medical, accounting, consulting, architectural, biomedical, chemical, research and development, and administrative services. Finance or Insurance Centers shall also pool financial risks by underwriting insurance and annuities. Some establishments support employee benefit programs. Examples include bank or credit union headquarters, brokerages, investments, insurance, financing, and data processing establishments. |
|----------------|---|
| 30 Base Points | Business Office/Finance/ Insurance / Professional Services Center – Small Qualifying projects 50,000 square feet of heated floor space or less. Employees perform professional, scientific, and technical services for others. Such services require a high degree of expertise and training and provide high salaried employment opportunities. Examples include software engineering, legal, medical, accounting, consulting, architectural, biomedical, chemical, research and development, and administrative services. Finance or Insurance Centers shall also pool financial risks by underwriting insurance and annuities. Some establishments support employee benefit programs. Examples include bank or credit union headquarters, brokerages, investments, insurance, financing, and data processing establishments. |
| 30 Base Points | Multi-Tenant Retail Center Newly constructed center 50,000 square feet or less, typically containing a more than one tenant space within a single structure. Uses are entirely consumer- driven and include all manner of retail, service and office possibilities. |
| 30 Base Points | Single Use Office Newly constructed single use, stand-alone building used primarily for office and professional. |
| 30 Base Points | Bungalow Court or Pocket Neighborhood Newly constructed Bungalow Court or Pocket Neighborhood per the standards of the Unified Development Ordinance. |
| 30 Base Points | Distribution/Trucking Center Newly constructed center of at least 500,000 square feet where products and resources are transported to and delivered from via truck or rail. |
| 25 Base Points | Warehouse Newly constructed center of at least 500,000 square feet where products and resources are stored. |
| 25 Base Points | Religious Institutions Any facility such as a church, temple, synagogue, mosque or monastery used for worship by a non-profit organization and their customarily related uses. |
| 20 Base Points | Intensive Industrial Uses: Uses classified as Special Land Uses within the Industrial Classification. |
| | |

| 20 Base Points | Multi-Family Residential & Condo Units | |
|------------------------|--|--|
| 20 Base Points | Major Subdivision 4- 25 Lots Any subdivision of land of four (4) – 25 Lots. | |
| 10 Base Points | Major Subdivision 26 lots or more10Any subdivision of land of 26 or more lots.10 | |
| Board Determination | All Other Uses Not CategorizedThis category of use captures all other uses not categorized elsewhere.Allocations for such uses are left to the discretion of the Town's Board ofCommissioners upon recommendation of the Planning Board and acted on a case- by-case basis. | |

BONUS POINTS

Proposed projects can gain BONUS POINTS by agreeing to provide any of the following items over and above the UDO or Standard Specification requirements for their development proposal.

NOTE: No bonus points are given for UDO requirements.

CATEGORY 1 – Non-Conformity Abatement and Public Infrastructure Improvements

| Section 1A - Abatement of Nonconformities | (Max - 3 points) |
|--|------------------|
| Abatement of any existing non-conforming structures | 3 |
| Abatement of any existing non-conforming use of land | 2 |
| Abatement of any existing non-conforming lots | 1 |

| Section 1B - Roadway Infrastructure Not Warranted by TIA/UDO/CTP | (Max - 10 points) |
|---|-------------------|
| Construction of full cross section of existing off-site public street | 5 |
| Nearby intersection improvements | 5 |
| Traffic signal improvements | 4 |
| Signage or striping improvements | 1 |

| Section 1C - Off-Site Public Greenway Improvements | (Max - 10 points) |
|---|-------------------|
| Construct more than 4000 linear feet of 10-foot-wide path | 10 |
| Construct more than 3000 linear feet of 10-foot-wide path | 8 |
| Construct more than 2000 linear feet of 10-foot-wide path | 6 |
| Construct more than 1000 linear feet of 10-foot-wide path | 4 |
| Construct 500 to 1000 linear feet of 10-foot-wide path | 2 |

| Section 1D – Off-Site Bike-Ped Improvements | (Max – 5 points) |
|---|------------------|
| Construction of off-site sidewalk improvements (Subject to Approval) | TRC 2 |
| Construction of off-site bike lane improvements (Subject to Approval) | TRC 3 |

CATEGORY 2. Green Development Standards/ Building & Site Design

| Section 2 | (Max - 10 points) | |
|-----------|-----------------------------------|--------|
| Requirem | | |
| | One point per acre up to 10 acres | 1 - 10 |

| Section 2B - Parking | | (Max – 15 points) |
|----------------------|--|-------------------|
| | Structured Parking Facilities - must reduce footprint by 20% | 10 |
| | EV Charging Stations (two-port) | 5 |
| | Provision of on-street public parking (1 point per stall up to 10 Max) | 1 - 10 10 |

Attachment 4 PD-2024-01

| Section 2C - Stormwater SCM's | (Max – 10 points) |
|---|-------------------|
| Stormwater - Restored Riparian Buffer | 10 |
| Construct a fountain or other stormwater amenity within the BMP/SCM | 4 4 |
| (as approved by Staff) | |
| Stormwater - Landscaped Green Roof | 5 |
| Stormwater - Underground capture system for on-site irrigation | 5 |
| Stormwater - Bioretention | 5 |
| Stormwater - Wetland | 5 5 |
| Exclusive use of porous pavement in parking areas where suitable | 2 |

| Section 2D - Building/Site Design | (Max - 20 points) |
|--|-------------------|
| Compliance with residential design guidelines per Section 5.2 of the UDO | ¹⁰ 10 |
| Non-Residential building design that incorporates an active upper story. | 5 |
| Pedestrian oriented and walkable site design which promotes alternatives to vehicular travel within the development. (Subject to TRC Approval) | 5 |

| Section 2E - Infill/Redevelopment | (Max – 16 points) |
|--|-------------------|
| Development or Redevelopment within DTC | 10 |
| Development or Redevelopment within DTP | 6 |
| Redevelopment of previously vacant building space over 20,000 square feet | 6 |
| Redevelopment of previously vacant building space under 20,000 square feet | 5 |

| Section 2 | 2F - Historic Preservation | |
|-----------|--|----|
| | Historic Structure Preservation via Deed Restriction (Determined by TRC) | 10 |
| | Restoration of Historic Structure (Must be approved by TRC) | 5 |

| Section 2G – LEED Certification | (Max – 10 points) |
|---|-------------------|
| LEED Certification for Neighborhood Development (LEED ND) | 10 |
| Platinum LEED Certification | 10 |
| Gold LEED Certification | 8 |
| Silver LEED Certification | 6 |
| Bronze LEED Certification | 4 |
| LEED Certified Certification | 2 |

CATEGORY 3 – Outdoor Enhancement and Transit Improvements

| Section 3 | 8A – Outdoor Enhancement | (Max – 12 points) |
|-----------|--|-------------------|
| | Construction of a Parkway Street Section on a Collector level street | 5 |

Attachment 4 PD-2024-01

| Construction or Preservation of Gateway Landscaping or Structure (Subject to Comprehensive Plan Consistency and TRC approval) | 5 | |
|--|---|---|
| Outdoor Display of Public Art (Subject to TRC Approval) | 4 | |
| Public Facing Outdoor Mural (Subject to TRC Approval) | 4 | |
| Maintenance of Roadside Gateway Plant Bed (requires maintenance agreement) | 3 | |
| Planting Pollinator Garden (225 Square Foot Minimum) | 3 | 2 |
| Exclusive use of xeriscaping techniques and drought tolerant species | 3 | 5 |
| Enhanced Roadside Landscaping (Subject to TRC Approval) | 2 | |
| Enhanced Buffer Landscaping (Subject to TRC Approval) | 2 | |
| Construction of a Parkway Street Section on a Local level street | 2 | |
| Installation of Native Shade Tree Species (per Tree up to 10 Trees) | 1 | 9 |

| Section 3B – Transit (Pursuant to location being adjacent to a planned or active transit route) | (Max - 8 points) |
|---|------------------|
| Provision of more than 50 designated Park & Ride Stalls | 8 |
| Provision of 25 designated Park & Ride Stalls | 5 |
| Provision of 10 designated Park & Ride Stalls | 3 |
| Provision of mass transit easement w/ structure (bus stop with shelter & bench) | 2 |

CATEGORY 4 - Amenities

| Section 4A - Private Greenway | (Max - 3 points) |
|--|------------------|
| Construction of more than 3000 linear feet private greenway meeting Town of Zebulon standards | 3 |
| Construction of more than 2000 linear feet of private greenway meeting Town of Zebulon standards | ² 2 |
| Construction of more than 1000 linear feet of private greenway meeting Town of Zebulon standards | 1 |

| Section 4B – Pool (Combinations may be approved by TRC) | (Max - 8 points) |
|---|------------------|
| Olympic Pool and Aquatic Center | 8 |
| Junior Olympic Pool | 5 |
| Lap Pool (four lane minimum) | 3 |
| Resort Style Pool | 2 2 |
| Any Other Pool | 1 |

| Section 4C - Outdoor Deck/Patio (Max - 3 point | | | |
|--|-----|--|--|
| Deck/Patio - More than 3000 square feet | 3 | | |
| Deck/Patio - More than 2000 square feet | 2 | | |
| Deck/Patio - More than 1000 square feet | 1 1 | | |

| Section 4D - Pool Amenities (Max - 2 points) | | |
|--|-----------------------------|------------------|
| | Section 4D - Pool Amenities | (Max - 2 points) |

| Jacuzzi/Hot Tub/Whirlpool | 2 |
|---------------------------------|---|
| Water Playground with apparatus | 2 |
| Sauna/Steam room | 2 |

| Section 4E - Clubhouse | (Max - 10 points) |
|---|-------------------|
| Commercial Coffee Shop with at least 10 designated public seating | 10 |
| spaces. | |
| With full kitchen and over 4000 square feet of meeting space | 10 |
| With full kitchen and less than 4000 square feet of meeting space | 9 |
| Meeting space without kitchen more than 3500 square feet | 8 |
| Meeting space without kitchen 2500 - 3499 square feet | 7 |
| Meeting Space without kitchen 1500 - 2499 square feet | 5 |
| Meeting Space without kitchen less than 1500 square feet | 4 |
| No meeting space, bathrooms and changing rooms only | 3 <mark>3</mark> |
| Outdoor Kitchen or Grills | 2 2 |

| Section 4F - Additional Active Recreation | (Max - 10 points) |
|--|-------------------|
| Gymnasium (regulation size indoor basketball court) | 10 |
| Baseball/Softball Field (regulation size) | 5 |
| Football/Soccer Field (regulation size) | 5 |
| Skate Park | 5 |
| Tennis Courts (two regulation courts, fenced) | 5 |
| Multi-Use Hardcourt (two regulation basketball courts, street hockey, fenced) | 5 |
| Pickleball Court (three regulation courts, fenced) | 5 |
| Pocket Park – 5,000 square feet | 3 <mark>3</mark> |
| IPEMA Certified Playground Equipment | 4 4 |
| Lighted Field of Play for nighttime use | 3 |
| Electronic Scoreboard or Covered Dugouts or Bleachers | 3 |
| Community Garden – 15-foot by 15-foot, with water access and potting shed. | 3 |

| Section 4G – Additional Urban Open Space Enhancements (Within Non Residential Zoning Districts) | (Max – 10 points) |
|--|-------------------|
| Fountain | 2 |
| Canopy Including Fixed Permanent Seating | 2 |
| Drinking Fountain with Pet Fountain | 2 |
| Permanent Game Tables | 1 |
| Permanent Tables with Shade Cover | 1 |
| All Weather Bulletin Board | 1 |
| Covered or Internal Bicycle Parking | 1 |
| Artist-Design Bicycle Racks | 1 |
| Little Free Library | 1 |
| Drinking Fountain | 1 |
| Public Work Bike Stand With Tools | 1 |

CATEGORY 5 – Affordable Housing

| Inclusion of developmen 80% of the A | (Max – 10 Points | |
|--|------------------------|----|
| | 15% Affordable Housing | 10 |
| | 10% Affordable Housing | 5 |

CATEGORY 6 – Other

(Max 5 Points)

| Integrated public safety operation systems (EX. Flock Safety or others | 3 |
|--|---|
| as approved by the Police Department) | |
| Smart Waste and Recycling Stations | 2 |

Total 68



| Date: | January 23, 2024 |
|----------|--|
| То: | Adam Culpepper, Senior Planner, Town of Zebulon Andrew Suriano, Managing Partner, Deacon Development Group Beth Blackmon, Senior Project Manager, Timmons Group Jeff Hochanadel, Principal, Timmons Group Ashley Honeycutt Terrazas, Associate, Parker Poe |
| From: | Sravya Suryadevara, PE, Traffic Engineering Director, WSP USA Inc. |
| Subject: | Zebulon South Supplemental Traffic Impact Analysis Review |

Per your request, WSP has performed a review of the Zebulon South development traffic impact study resubmitted by Timmons Group, dated January 2024 and the supplemental memo, also dated January 2024. We have the following comments:

- Based on the updated site plan, the number of units has changed for the site since the TIA was completed. Please add a note in the body of the TIA report discussing this change and confirming that the analysis is still valid because it is more conservative than the current site plan.
- Please confirm site access locations in the Build Synchro files matches the site plan. If Site Access 2 is within • 165 feet of Perry Curtis Road, this access will need to be right-in/right-out only. This is based on the 2003 NCDOT's Policy on Street and Driveway Access.
- For tables 3-1, 3-2, and 5-1 in the TIA and tables 1 and 2 in the supplemental, please provide a footnote to • describe the meaning of the "#" symbol in the queue lengths.
- For tables 3-1, 3-2, 5-1, and 5-2 in the TIA and tables 1 and 2 in the supplemental, please designate which • intersections are unsignalized/signalized to aid in the differentiation of queues which are in feet and queues which are number of cars.
- For tables 3-1, 3-2, 5-1, and 5-2 in the TIA and tables 1 and 2 in the supplemental, please add units for ٠ queues.
- Include NCDOT comments from July 2022 referenced in section 6 in the appendix of the TIA if available. •
- Please add a complete list of recommended improvements to the supplemental memo for clarity, even ٠ though the recommendations do not change from the TIA.
- The following comment responses were provided by Timmons Group based on the initial submittal review. • Please add these explanations in the body of the TIA report to provide a full picture of the analysis methodology:
 - Include discussion on why count data was not balanced between intersections and why Perry Curtis 0 Road volumes were used for site access 1 and 2.

TG Response: Traffic volumes were not balanced to the presence of commercial site driveways and various side streets. To provide the most accurate analyses, corridor volumes were not balanced. Site Access 1 and 2 volumes were balanced with Perry Curtis due to the driveways' proximities.

Please provide justification for the 3% growth rate used for background volume development. 0

TG Response: The 3% growth rate is based on published AADTs.



In the Build scenario turn lane analysis, it's mentioned that both S Wakefield Street and NC 96 will have
 2026 AADTs higher than 4,000 vpd. Please clarify if this is based on the existing AADT value and an assumption of growth or if this is based on the existing AADT including an assumed growth rate.

TG Response: NC-96's AADT currently exceeds 4,000 VPD. Per future projections, this value is not projected to decrease. S Wakefield Street AADT projections are based on recent AADT counts (grown at 3% annually to 2026) and 30% of daily site trips on S Wakefield Street north of Site Access 3.

• Site access roads are listed as needing 100-feet of IPS. Please define IPS as internal protected stem in the text and reference the standards that guide this recommendation.

TG Response: IPS was defined as "internal protected stem" in the updated TIA. IPS requirements are defined in the NCDOT's Driveway Manual.

• The alignment of the S Wakefield Street/Morphius Bridge and Pully Gordon Road intersection is not ideal for safe operations, but no improvements are required currently.

We do not anticipate any of the above comments to impact the analysis or recommendations in the TIA. If you have any questions about this review, please do not hesitate to contact me at (984) 389-2944 or sravya.suryadevara@wsp.com.



January 2nd, 2023

Michael Clark Planning Director Town of Zebulon 1003 N. Arendell Avenue Zebulon, NC 27597 919-823-1808 mclark@townofzebulon.org

RE: Zebulon South Memo

Dear Mr. Clark,

This memorandum is a supplement to the Zebulon South Traffic Impact Analysis (TIA). The TIA was initially scoped with the Town of Zebulon (Town) and NCDOT in March 2022. Originally sealed June 28th, 2022, the NCDOT provided final comments on July 26th, 2022. On November 27th, 2023, WSP provided Town comments to Timmons Group. The TIA was updated and resubmitted (sealed January 2nd, 2023). At the time of scoping, there were no approved area developments that would contribute trips during the Background or Build analyses. In the interim, the Chamblee Lake Planned Development TIA was approved. Due to traffic concerns expressed by Town Council and area citizens, the project team determined that additional analyses should be conducted including the proposed Chamblee Lake Planned Development. This memorandum's purpose is to determine 1) the Zebulon South Development site trip impacts to study area intersections (including the Chamblee Lake Planned Development), and 2) if improvement recommendations are changed from the original TIA.

The following intersections were analyzed:

- NC-97 (Gannon Ave) / SR-2349 (South Wakefield Street);
- NC-97 (Gannon Ave) / NC-96 (Arendell Ave);
- NC-96 (Arendell Ave) / SR-2348 (West Barbee Street);
- NC-96 (Arendell Ave) / Site Access 1*;
- NC-96 (Arendell Ave) / Site Access 2*;
- NC-96 (Arendell Ave) / SR-2347 (Perry Curtis Road); and
- SR-2349 (South Wakefield Street) / Site Access 3*.
- * Build conditions only

Upgraded 2026 Background and Build + Improvement AM and PM peak hour capacity analyses were performed including the Chamblee Lake Planned Development site trips. As discussed below, it was determined that with the Chamblee Lake Planned Development site trip addition, all study area intersection approaches are projected to perform acceptably. Therefore, no additional improvement recommendations were necessary.

5410 Trinity Rd. Suite 102 | Raleigh, NC 27607

Development | Residential | Infrastructure | Technology

Site



2026 Background

Table 1 below summarizes the intersection LOS and delay based on existing intersection geometry (see **Figure A**) and the 2026 Background traffic volumes (see **Figure D**). 2026 Background volumes were calculated by summing 2026 ambient traffic volumes (**Figure B** – Zebulon South TIA **Figure 3-1**) and Chamblee Lake Planned Development traffic volumes (**Figure C**^ and **Appendix B**). The corresponding SYNCHRO outputs are located in **Appendix A**. As shown in **Table 1**, all intersection approaches are projected to operate acceptably during both 2026 Background peak hours. Optimized timings were used for all signalized intersection analyses (adhering to NCDOT minimum cycle length requirements).

^ For purposes of analysis (and to be more conservative), it was assumed that all traffic projected along Horton Street turned right onto NC-96 south of NC-97. Traffic was then split between northbound left and through at NC-97. All traffic wishing to travel west (towards US-264) will likely utilize NC-39.



| 2026 Backgrou | | | | | | | | | PM PEAK |
|---|---|---------------------------------|-------|---------------------------------|-------|------------------------------|-------------------------|--|--|
| | | AM PEAK HOUR | | PM PEAK HOUR | | | Turn | AM PEAK HOUR | HOUR |
| Intersection | Approach / Overall | Delay ¹ (sec/veh) | LOS 1 | Delay ¹ (sec/veh) | LOS 1 | Movement | Lane Storage (ft) | *95th Percentile Queue Length | *95th Percentile Queue Length |
| 1: S Wakefield Street & NC-97 | | | | | - | EB Left | 125 | 10 | 11 |
| (Gannon Avenue) | Eastbound | 22.4 | С | 31.1 | С | EB Thru/Right | | 380 | #744 |
| | C12 | | | | | EB Approach | | | |
| | | | | | | WB Left | 125 | 63 | 72 |
| | Westbound | 13.9 | в | 11.3 | в | WB Thru/Right | | 265 | 237 |
| | | | | | | WB Approach | | × | |
| | Northbound | 31.8 | С | 46.4 | D | NB Left/Thru/Right | | 199 | 169 |
| | Northbound | 31.8 | C | 40.4 | U | NB Approach | | | - |
| | Southbound | 23.4 | с | 31.2 | с | SB Left/Thru/Right | | 35 | 63 |
| | Southbound | 23.4 | C | 31.2 | C | SB Approach | | () | |
| | Overall | 20.2 | С | 25.1 | С | Overall | | | |
| 2: NC-96 (Arendell Avenue) & NC- | | | | | | EB Left | 200 | 46 | 73 |
| 97 (Gannon Avenue) | Eastbound | 33.2 | с | 38.0 | D | EB Thru | | 367 | #336 |
| | Luscoound | 33.2 | ~ | 50.0 | 0 | EB Right | 100 | 69 | 100 |
| | | | | | | EB Approach | | () () | - |
| | Westbound | 23.6 | с | 28.4 | с | WB Left | 350 | #238 | #276 |
| | | | | | | WB Thru/Right | | 166 | 348 |
| | | | | | | WB Approach | | 877 | |
| | Northbound | 40.1 | D | | | NB Left | 125 | 124 | 96 |
| | | | | 38.7 | D | NB Thru/Right | | #352 | #462 |
| | | | | | - | NB Approach | | - | |
| | Southbound | 29.7 | С | 27.8 | | SB Left | 250 | 36 | #77 |
| | | | | | С | SB Thru/Right | | 203 | 230 |
| | | | | | | SB Approach | | () | |
| | Overall | 31.5 | С | 33.2 | С | Overall | | | |
| 3: NC-96 (Arendell Avenue) & Barbee Street | Eastbound | 14.9 | В | 22.1 | с | EB Left/Thru/Right | | 0.5 | 1.9 |
| barbee Street | | COLUMN . | | | 1001 | EB Approach | | | |
| | Westbound | 14.1 | В | 16.7 | С | WB Left/Thru/Right | | 0.1 | 0.2 |
| | | | | | | WB Approach | | | |
| | Northbound | 1.0 | A | 1.0 | A | NB Left/Thru/Right | | 0.1 | 0.1 |
| | - | | | | - | NB Approach | | | - |
| | Southbound | 0.2 | A | 0.1 | A | SB Left/Thru/Right | | 0 | 0 |
| 6: NC-96 (Arendell Avenue) & Perry | | | | | - | SB Approach | | | |
| Curtis Road | Westbound Northbound | 11.1 | В | 11.1 | В | WB Left/Right | | 0.6 | 0.4 |
| Jurus Nudu | | | | | - | WB Approach | | 0 | 0 |
| | | 0.0 | A | 0.0 | Α | NB Thru/Right NB Approach | | 0 | |
| | | 0.000 | | | | SB Left/Thru | | 0.1 | 0.4 |
| | Southbound | 1.8 | Α | 2.6 | Α | SB Approach | | 0.1 | 0.4 |

Table 1: Intersection Approach Level of Service and Delay2026 Background Traffic Volumes

¹ Overall intersection LOS and delay not reported for TWSC intersections.

* - 95th percentile queues for unsignalized intersections reported in number of vehicles.

2026 Build + Improvements

The Zebulon South trip generation and distribution are located in the Zebulon South TIA (see **Section 4** and **Figure 4-2**, respectively). 2026 Build traffic volumes (see **Figure E**) were calculated by summing the 2026 Background traffic volumes (**Figure D**) and projected Zebulon South site trips (**Figure 4-2** – Zebulon South TIA). **Table 2** below summarizes the intersection LOS and delay based on the future lane configuration (see **Figure F**) and 2026 Build traffic volumes (see **Figure E**). The corresponding SYNCHRO outputs are located in **Appendix A**. As shown in **Table 2**, all intersection approaches are projected to operate acceptably during the 2026 Build + Improvements AM and PM peak hours. Optimized timings were used for all signalized intersection analyses (adhering to NCDOT minimum cycle length requirements). Because all approaches are projected to operate acceptably, no additional study area intersection improvements are recommended.



Table 2: Intersection Approach Level of Service and Delay 2026 Build + Improvements Traffic Volumes AM PEAK PM PEAK HOUR HOUR AM PEAK HOUR PM PEAK HOUR Turn

| | 2020/02/2020/2020 | 100 A. 100 A. 100 A. | _ | 12.000 | _ | | Tum | HOUK | HOUR |
|---|-----------------------|----------------------|-------|-----------|----------|--------------------|-----------------|------------|---------------------|
| Intersection | Approach / | | | | | Movement | Lane | *95th | *95th |
| | Overall | Delay 1 | LOS 1 | Delay 1 | LOS 1 | | Storage (ft) | Percentile | Percentile Queue |
| | | (sec/veh) | | (sec/veh) | | | (10) | Queue | Length |
| 1: S Wakefield Street & NC-97 | | | | | | EB Left | 125 | 10 | 11 |
| (Gannon Avenue) | Eastbound | 25.1 | с | 41.0 | D | EB Thru/Right | 125 | 405 | #825 |
| | Lasubound | 23.1 | - | 41.0 | | EB Approach | | | #025 |
| | | | | | | | 105 | | |
| | | 15.0 | | | | WB Left | 125 | 68 | 90 |
| | Westbound | 15.9 | В | 12.9 | В | WB Thru/Right | | 287 | 244 |
| | | | | | | WB Approach | | - | - |
| | Northbound | 35.1 | D | 54.8 | D | NB Left/Thru/Right | | #273 | #232 |
| | | 100 | | | | NB Approach | | | |
| | Southbound | 23.0 | с | 30.9 | с | SB Left/Thru/Right | | 35 | 63 |
| | Southooding | 23.0 | | 30.5 | | SB Approach | | | |
| and the second se | Overall | 23.0 | С | 31.9 | С | Overall | | | |
| 2: NC-96 (Arendell Avenue) & NC- | 1.1.1.1.1.1 | | | | | EB Left | 200 | 48 | 77 |
| 97 (Gannon Avenue) | 5-11-11-1 | 20.5 | | 47.4 | | EB Thru | | #439 | #389 |
| | Eastbound | 38.5 | D | 47.1 | D | EB Right | 100 | 76 | 121 |
| | | | | | | EB Approach | | | |
| | | | | | | WB Left | 350 | #268 | #321 |
| | Westbound | 27.9 | C | 33.2 | с | WB Thru/Right | | 185 | 385 |
| | | | - | | - | WB Approach | | | - |
| | | | | | | NB Left | 125 | 134 | 106 |
| | Northbound | 41.6 | D | 40.2 | D | | 125 | | #494 |
| | worthbound | 41.0 | U | 40.2 | | NB Thru/Right | | #407 | |
| | | | | | | NB Approach | - | | |
| | 121.0001.000 | 12222 | 1.2 | 1000 | | SB Left | 250 | 36 | #83 |
| | Southbound | 27.7 | С | 28.0 | С | SB Thru/Right | | 200 | 243 |
| | | | | | | SB Approach | · · · · · · · · | | |
| | Overall | 34.8 | С | 37.4 | D | Overall | | | |
| 3: NC-96 (Arendell Avenue) & | Eastbound | 17.0 | c | 30.0 | D | EB Left/Thru/Right | | 0.6 | 2.8 |
| Barbee Street | Eastouunu | 17.0 | | 50.0 | | EB Approach | | | |
| | Westbound | | с | 20.2 | | WB Left/Thru/Right | | 0.1 | 0.2 |
| | westbound | 16.1 | | 20.2 | С | WB Approach | | | |
| | Northborrd | | | | | NB Left/Thru/Right | | 0.1 | 0.2 |
| | Northbound | 0.9 | A | 1.0 | A | NB Approach | | | |
| | and the second second | 10000 | | 200 | | SB Left/Thru/Right | | 0 | 0 |
| | Southbound | 0.2 | A | 0.1 | A | SB Approach | | | |
| 4: NC-96 (Arendell Avenue) & Site | | | | | | EB Left/Right | | 0.4 | 0.3 |
| Access 1 | Eastbound | 13.1 | В | 16.0 | C | EB Approach | | | |
| | | | | | | NB Left/Thru | | 0 | 0 |
| | Northbound | 0.1 | A | 0.3 | A | | | - | |
| | | | | | | NB Approach | | | |
| | | | | | | SB Thru | | 0 | 0 |
| | Southbound | 0.0 | A | 0.0 | A | SB Right | 50 | 0 | 0 |
| | | | | | | SB Approach | | | |
| 5: NC-96 (Arendell Avenue) & Site | Eastbound | 12.3 | в | 14.9 | в | EB Left/Right | | 0.4 | 0.3 |
| Access 2 | Custosand | | | 1 | - | EB Approach | | | |
| | Northbound | 0.1 | A | 0.6 | A | NB Left/Thru | | 0 | 0.1 |
| | Northbound | 0.1 | ~ | 0.0 | ~ | NB Approach | | | |
| | | | | | | SB Thru | | 0 | 0 |
| | Southbound | 0.0 | A | 0.0 | A | SB Right | 50 | 0 | 0 |
| | | | | | | SB Approach | _ | | |
| 6: NC-96 (Arendell Avenue) & Perry | | | | | | WB Left/Right | | 0.7 | 0.5 |
| Curtis Road | Westbound | 11.3 | в | 11.3 | В | WB Approach | | | |
| | | | | | | NB Thru/Right | | 0 | 0 |
| | Northbound | 0.0 | A | 0.0 | A | NB Approach | | | |
| | | | - | | | | | 0.1 | 0.4 |
| | Southbound | 1.9 | A | 2.6 | A | SB Left/Thru | | | |
| 2. C Mahadada Charles Contra | | | - | | - | SB Approach | | | |
| 7: S Wakefield Street & Site Access 3 | Westbound | estbound 10.1 | в | 9.9 | A | WB Left/Right | | 0.3 | 0.2 |
| | westbound | | - | | | WB Approach | | - | |
| | Northbound | 0.0 | A | 0.0 | A | NB Thru/Right | | 0 | 0 |
| | Northbound | 0.0 | | 0.0 | <u> </u> | NB Approach | | | |
| | | | | | SB Left | 50 | 0 | 0.1 | |
| | Southbound | 1.1 | A | 1.8 | A | SB Thru | | 0 | 0 |
| | | | | | | SB Approach | | | |
| | | | | | _ | | - | | |

¹ Overall intersection LOS and delay not reported for TWSC intersections.

* - 95th percentile queues for unsignalized intersections reported in number of vehicles.

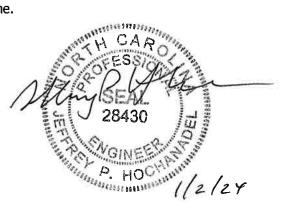


Conclusions

Per the provided analyses, it was determined that inclusion of Chamblee Lake Planned Development site trips does not result in changes to original Zebulon South TIA recommendations.

Should you have any questions regarding this memorandum, please do not hesitate to contact me.

Sincerely,



Jeffrey P. Hochanadel, PE, PTOE Principal | North Carolina Transportation Group Leader



LIST OF TABLES

Table 1 – Intersection Approach Level of Service and Delay –2026 Background Traffic Volumes

Table 2 – Intersection Approach Level of Service and Delay –

2026 Build + Improvements Traffic Volumes

LIST OF FIGURES

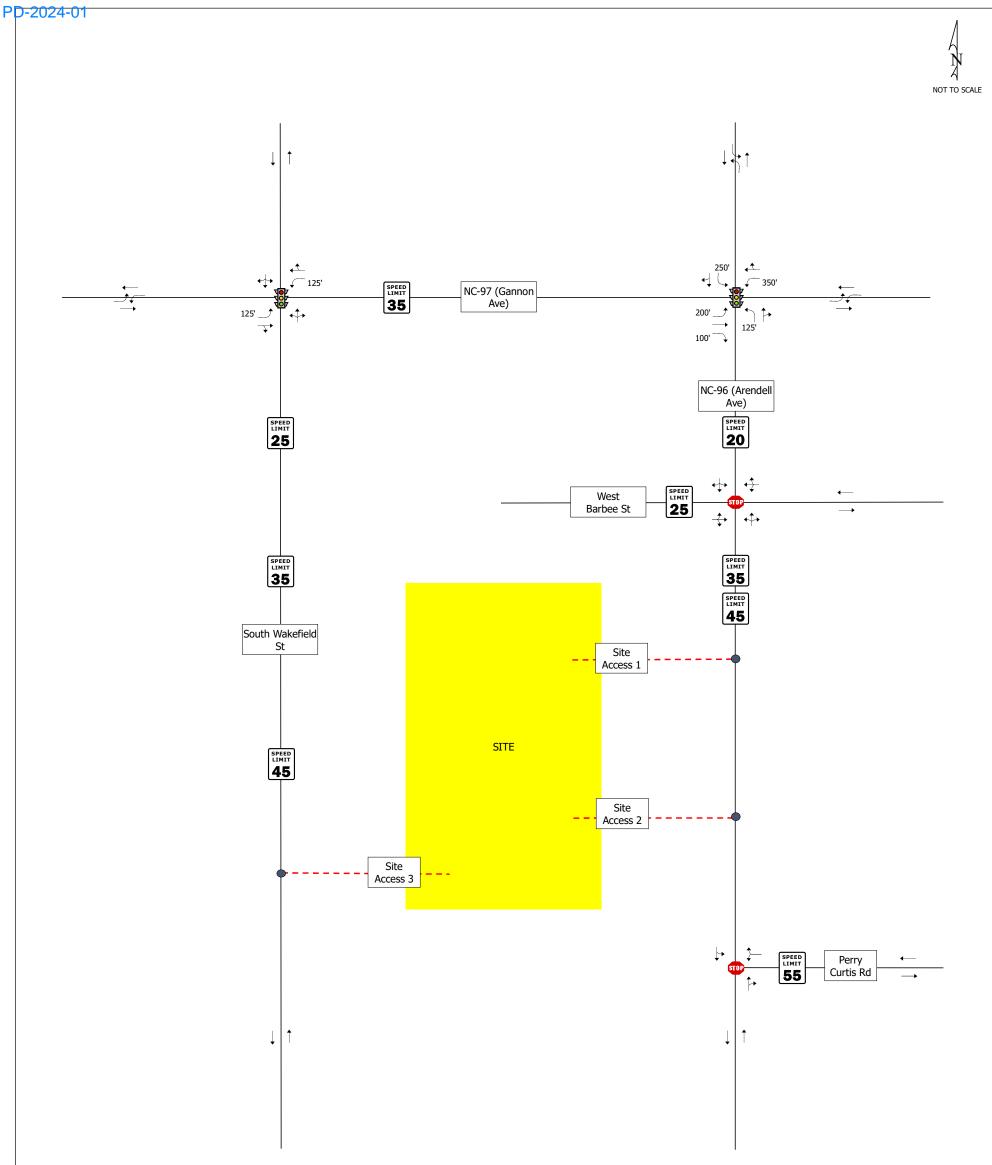
- FIGURE A EXISTING LANE CONFIGURATION
- FIGURE B 2026 AMBIENT TRAFFIC VOLUMES
- FIGURE C CHAMBLEE LAKE PLANNED DEVELOPMENT TRAFFIC VOLUMES
- FIGURE D 2026 BACKGROUND TRAFFIC VOLUMES
- FIGURE E 2026 BUILD TRAFFIC VOLUMES
- FIGURE F FUTURE LANE CONFIGURATION

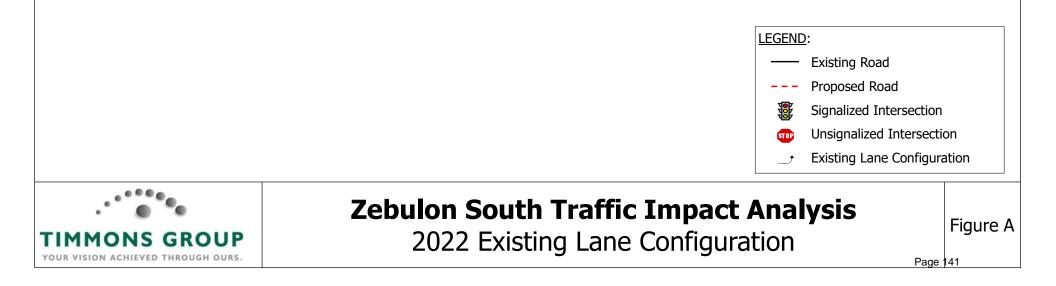
APPENDICES

Appendix A – Synchro Output

Appendix B – Chamblee Lake Planned Development



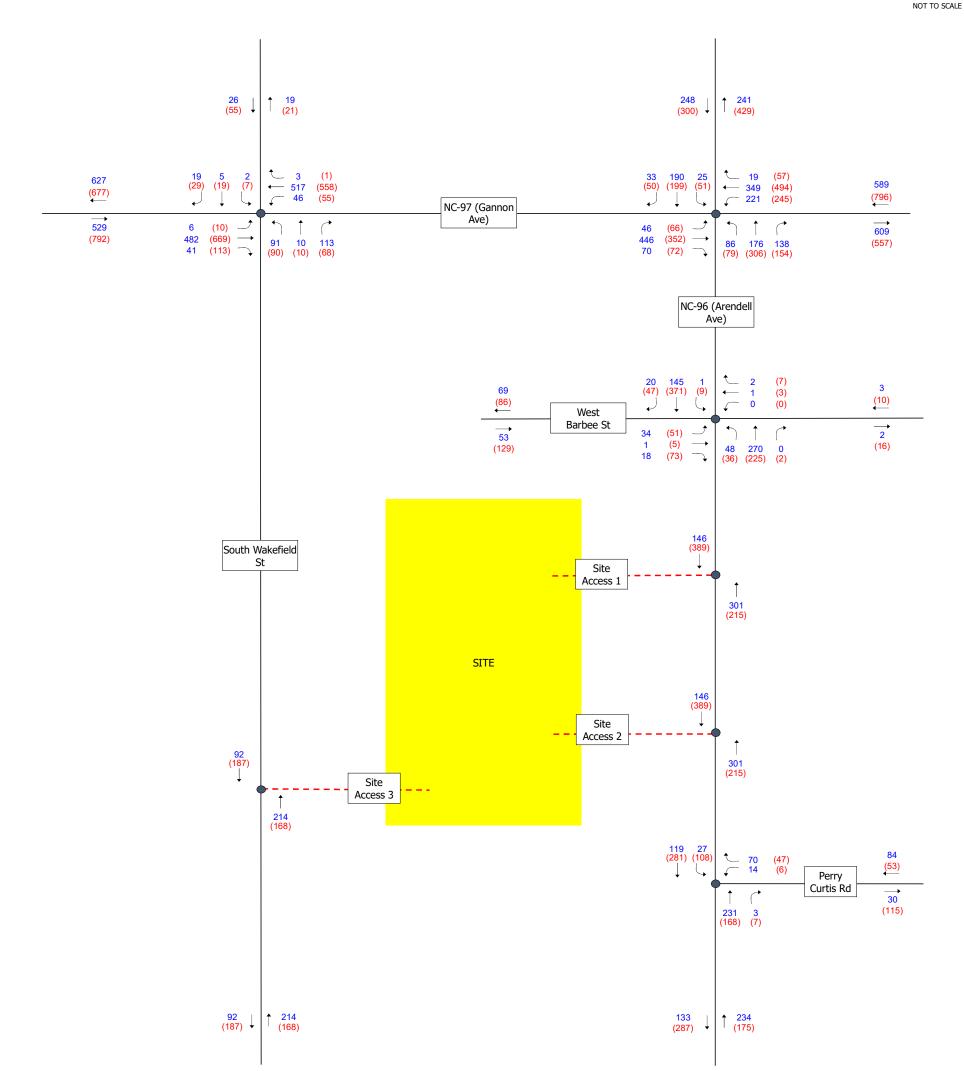


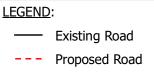


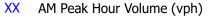
Attachment 6



A N A







(XX) PM Peak Hour Volume (vph)



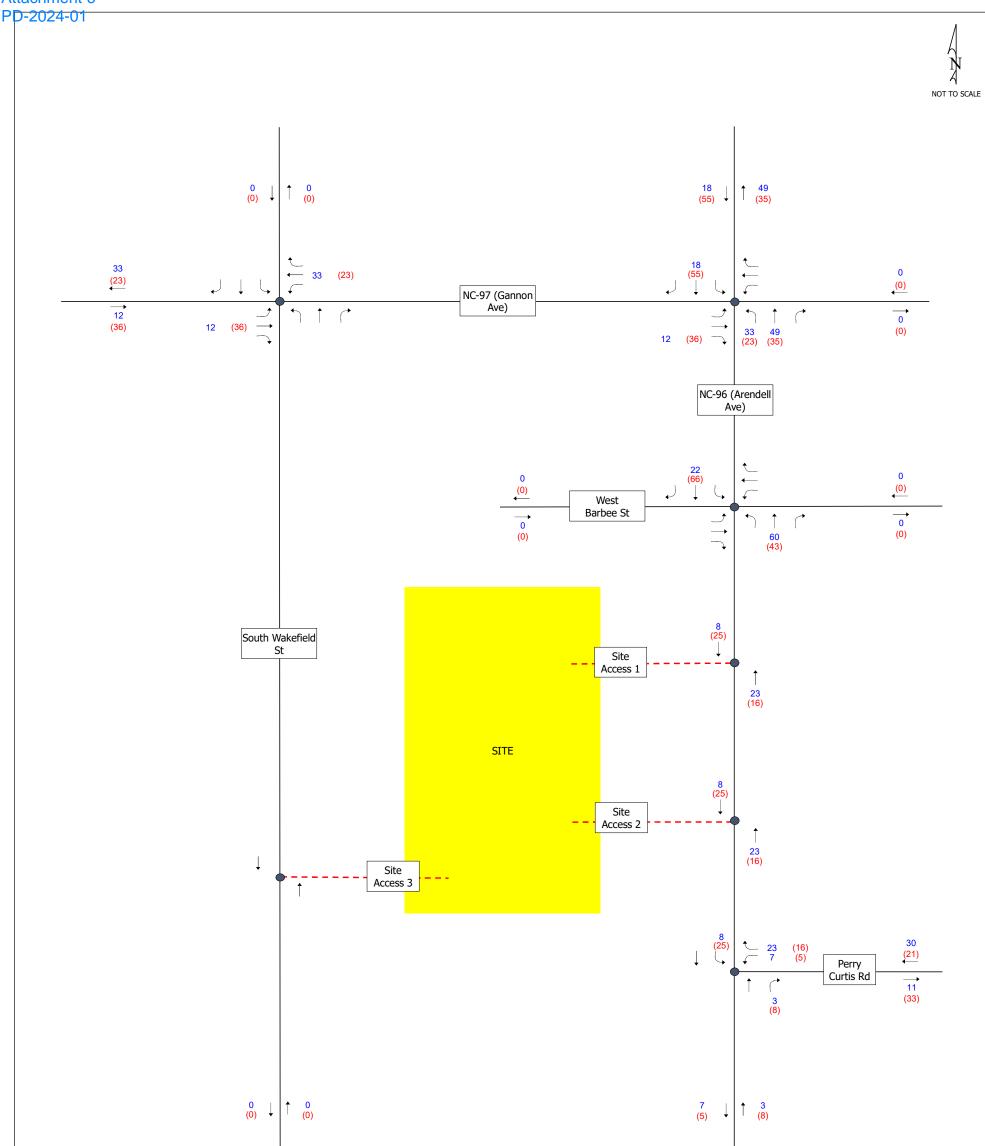
Zebulon South Traffic Impact Analysis

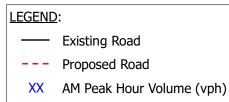
2026 Ambient Traffic Volumes

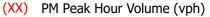
Figure B

Page 142

Attachment 6







TIMMONS GROUP YOUR VISION ACHIEVED THROUGH OURS.

Zebulon South Traffic Impact Analysis

Chamblee Lake Planned Development Traffic Volumes Page 143

Figure C

Attachment 6



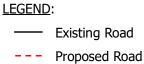
A N A

NOT TO SCALE

26 (55) ↓ 19 (21) 266 (355) ↓ 1 290 (464) (1) (581) (55) ↓ 19 (57) ↓ 349 (494) ↓ 221 (245) 19 5 (29) (19) ↓ ↓ $\begin{array}{c} & 3 \\ \leftarrow & 550 \\ \hline & 46 \end{array}$ 2 (7) 660 589 (700) (796) NC-97 (Gannon 541 (828) Ave) 119 225 138 (102) (341) (154)
 ●
 ↓
 ↓

 91
 10
 113

 (90)
 (10)
 (68)
 609 6 494 41 46 446 82 (10) (705) (113) (66) (352) (108) $\overrightarrow{}$ $\overrightarrow{}$ (557) NC-96 (Arendell Ave) 20 167 1 (47) (437) (9) (7) (3) (0) 2 1 0 69 (86) € 3 (10) West 48 330 0 (36) (268) (2) <mark>2</mark> (16) Barbee St (51) → (5) → (73) → 53 34 1 18 (129) 154 (414) ↓ South Wakefield St Site Access 1 Î 324 (231) SITE 154 | (414) ↓ Site Access 2 Î <mark>92</mark> (187) <mark>324</mark> (231) Site Access 3 Î 214 (168) 119 35 (281) (133) ↓ ↓ 114 (74) ↓ € € 93 21 (63) (11) Perry Curtis Rd <mark>41</mark> (148) Ť (231 6 (168) (15) ↑ <mark>214</mark> (168) 140 (292) 92 (187) ↓ 1 237 (183) Ļ





(XX) PM Peak Hour Volume (vph)



Zebulon South Traffic Impact Analysis

2026 Background Traffic Volumes

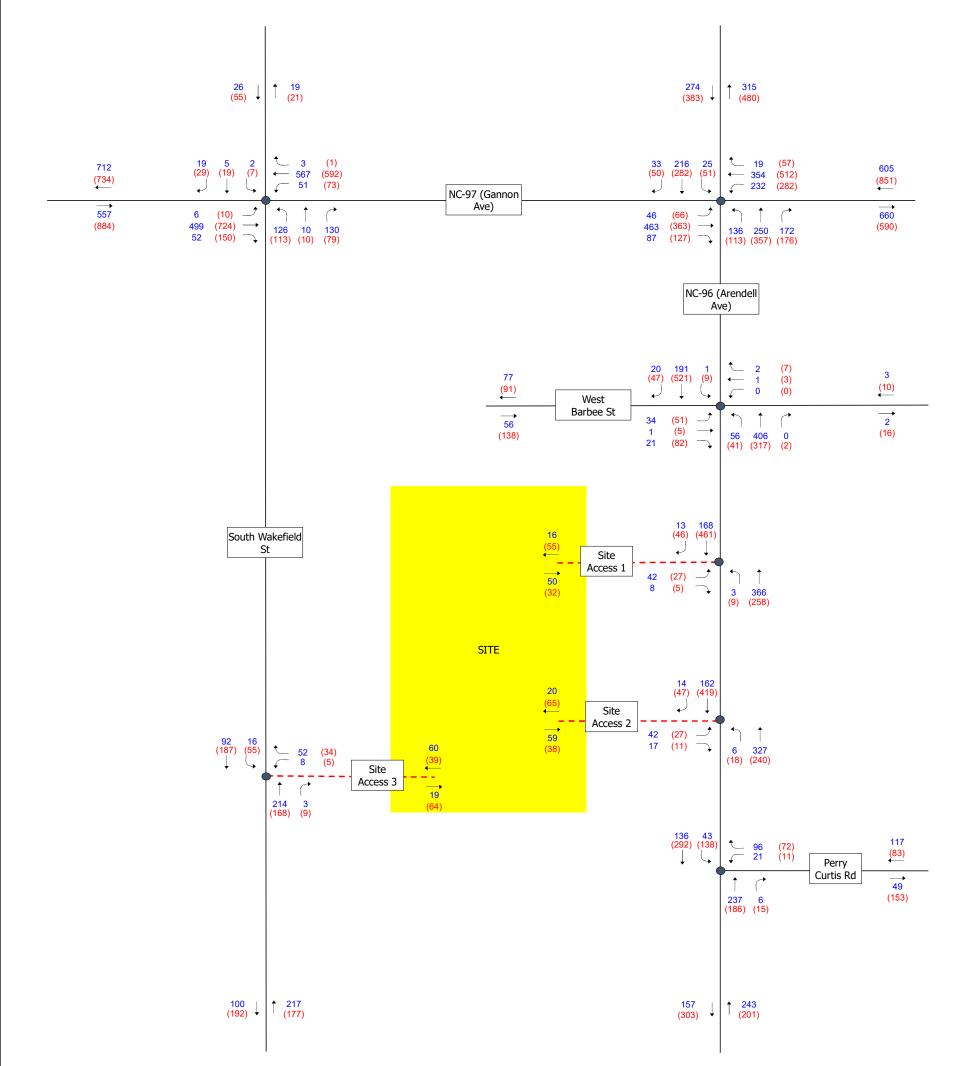
Figure D

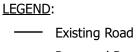
Page 144

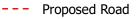
Attachment 6



A N A NOT TO SCALE







- XX AM Peak Hour Volume (vph)
- (XX) PM Peak Hour Volume (vph)



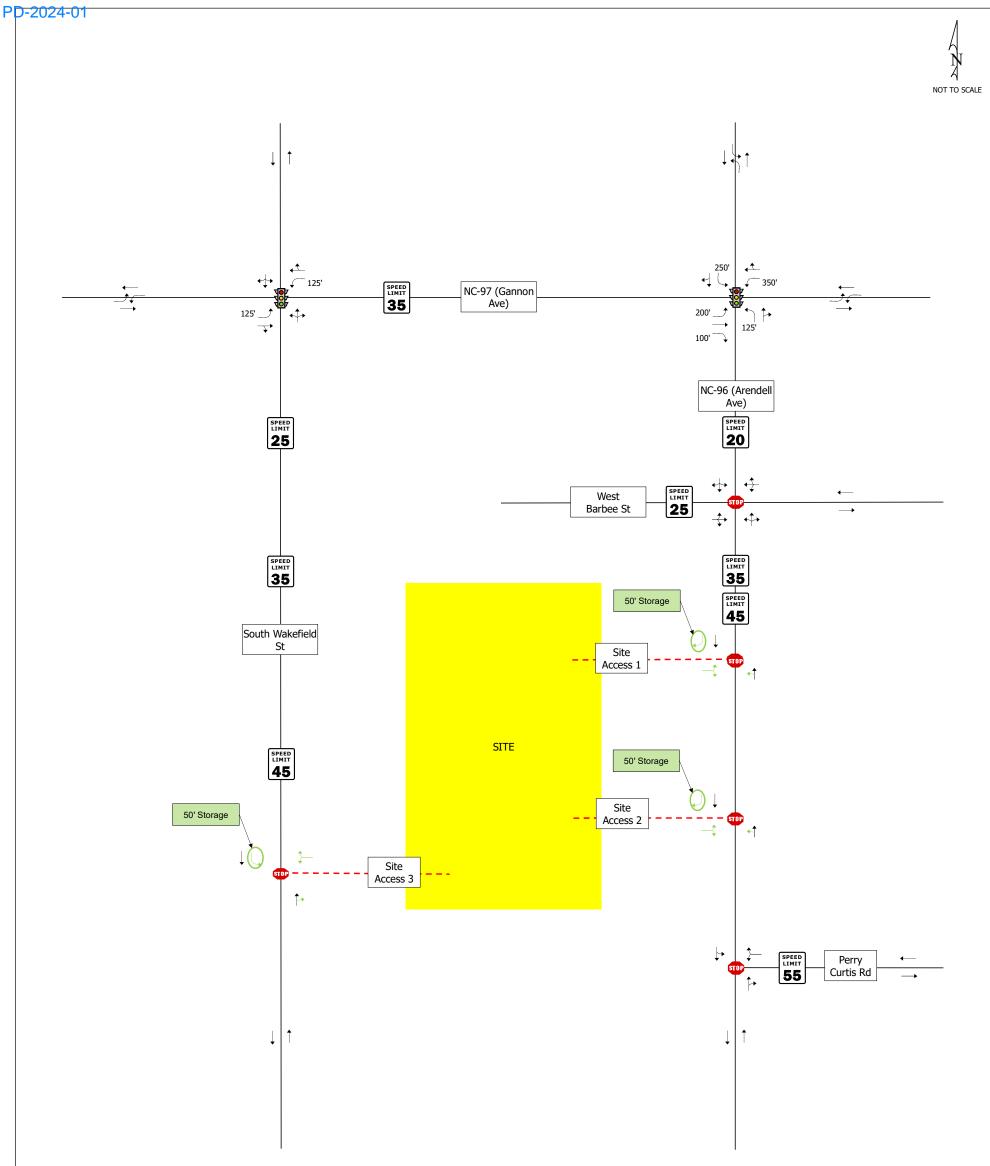
Zebulon South Traffic Impact Analysis

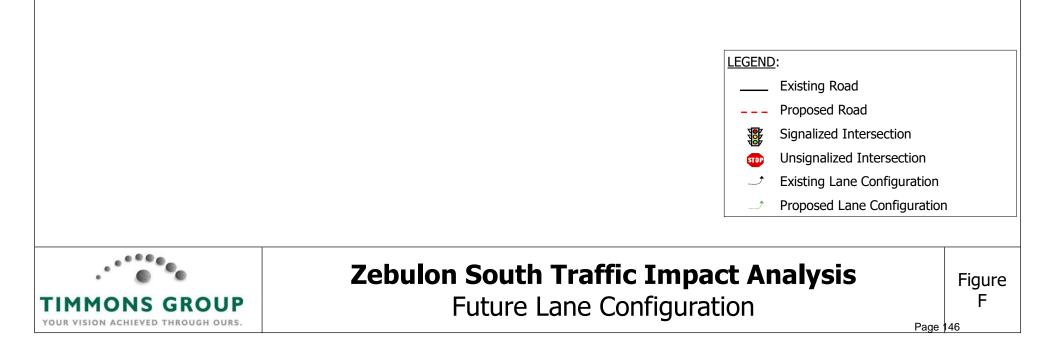
2026 Build Traffic Volumes

Figure E

Page 145







Appendix A – Synchro Output

2026 Background Traffic Volumes

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/14/2023

| | ≯ | - | 7 | 4 | + | • | 1 | Ť | 1 | 4 | Ŧ | ~ |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | f) | | 2 | et. | | | \$ | | | \$ | |
| Traffic Volume (vph) | 6 | 494 | 41 | 46 | 550 | 4 | 91 | 10 | 113 | 4 | 5 | 19 |
| Future Volume (vph) | 6 | 494 | 41 | 46 | 550 | 4 | 91 | 10 | 113 | 4 | 5 | 19 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.988 | | | 0.999 | | | 0.929 | | | 0.909 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.979 | | | 0.994 | |
| Satd. Flow (prot) | 1778 | 1850 | 0 | 1770 | 1861 | 0 | 0 | 1669 | 0 | 0 | 1675 | 0 |
| Flt Permitted | 0.432 | | | 0.950 | | | | 0.848 | | | 0.952 | |
| Satd. Flow (perm) | 809 | 1850 | 0 | 1770 | 1861 | 0 | 0 | 1445 | 0 | 0 | 1604 | 0 |
| Right Turn on Red | | | No | | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 7 | 549 | 46 | 51 | 611 | 4 | 101 | 11 | 126 | 4 | 6 | 21 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 7 | 595 | 0 | 51 | 615 | 0 | 0 | 238 | 0 | 0 | 31 | 0 |
| Enter Blocked Intersection | No |
| Lane Alignment | Left | Left | Right |
| Median Width(ft) | | 12 | | | 12 | | | 0 | | | 0 | |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | Yes | | | | | | | |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 | | 9 | 15 | | 9 | 15 | | 9 | 15 | | 9 |
| Turn Type | Perm | NA | | Prot | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 8 | | | 4 | |
| Permitted Phases | 2 | | | | | | 8 | | | 4 | | |
| Detector Phase | 2 | 2 | | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | | | | | | | | | | | | |
| Minimum Initial (s) | 10.0 | 10.0 | | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 17.0 | 17.0 | | 14.0 | 17.0 | | 14.0 | 14.0 | | 14.0 | 14.0 | |
| Total Split (s) | 47.0 | 47.0 | | 14.0 | 61.0 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Total Split (%) | 52.2% | 52.2% | | 15.6% | 67.8% | | 32.2% | 32.2% | | 32.2% | 32.2% | |
| Maximum Green (s) | 40.0 | 40.0 | | 7.0 | 54.0 | | 22.0 | 22.0 | | 22.0 | 22.0 | |
| Yellow Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | | -2.0 | -2.0 | | | -2.0 | | | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | | None | Min | | None | None | | None | None | |
| Act Effct Green (s) | 28.8 | 28.8 | | 10.3 | 35.5 | | | 17.7 | | | 17.7 | |
| Actuated g/C Ratio | 0.45 | 0.45 | | 0.16 | 0.55 | | | 0.27 | | | 0.27 | |
| v/c Ratio | 0.02 | 0.72 | | 0.18 | 0.60 | | | 0.60 | | | 0.07 | |
| Control Delay | 13.3 | 22.6 | | 35.1 | 12.1 | | | 31.8 | | | 23.4 | |

2026 Background AM Peak Hour Timmons Group

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/14/2023

| | ٨ | → | 7 | 4 | + | * | 1 | t | 1 | 4 | ŧ | ~ |
|------------------------------|-------------|------------|----------|----------|----------|------------|-----|------|-----|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 13.3 | 22.6 | | 35.1 | 12.1 | | | 31.8 | | | 23.4 | |
| LOS | В | С | | D | В | | | С | | | С | |
| Approach Delay | | 22.4 | | | 13.9 | | | 31.8 | | | 23.4 | |
| Approach LOS | | С | | | В | | | С | | | С | |
| Queue Length 50th (ft) | 2 | 221 | | 21 | 142 | | | 94 | | | 10 | |
| Queue Length 95th (ft) | 10 | 380 | | 63 | 265 | | | 199 | | | 35 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 575 | 1317 | | 281 | 1518 | | | 612 | | | 680 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.01 | 0.45 | | 0.18 | 0.41 | | | 0.39 | | | 0.05 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 64 | .6 | | | | | | | | | | | |
| Natural Cycle: 60 | | | | | | | | | | | | |
| Control Type: Actuated-Ur | coordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.72 | | | | | | | | | | | | |
| Intersection Signal Delay: | | | | | | n LOS: C | | | | | | |
| Intersection Capacity Utiliz | ation 65.7% | 6 | | IC | CU Level | of Service | ЭC | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| Splits and Phases: 1: S | Wakefield S | Street & N | IC-97 (G | annon Av | (enue) | | | | | | | |

Splits and Phases: 1: S Wakefield Street & NC-97 (Gannon Avenue)

| √ Ø1 | <u>→</u> _{Ø2} | Ø4 | |
|----------------|------------------------|-------|--|
| 14 s | 47 s | 29 s | |
| ← Ø6 | | ≪¶ øs | |
| 61s | | 29 s | |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/14/2023

| | ٦ | + | * | 1 | + | * | 1 | t | 1 | 1 | Ŧ | ~ |
|--|-------|------------|------------|------------|------------|---------|-------|------------|-------|----------|------------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 3 | 1 | 1 | 5 | ţ, | | ň | ţ, | | 3 | ţ, | •= |
| Traffic Volume (vph) | 46 | 446 | 82 | 221 | 349 | 19 | 119 | 225 | 138 | 25 | 208 | 33 |
| Future Volume (vph) | 46 | 446 | 82 | 221 | 349 | 19 | 119 | 225 | 138 | 25 | 208 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | 1000 | 2% | | 1000 | -2% | 1000 | 1000 | -2% | 1000 | 1000 | 2% | 1000 |
| Storage Length (ft) | 200 | 270 | 100 | 350 | 270 | 0 | 125 | 270 | 0 | 250 | 270 | 0 |
| Storage Lanes | 1 | | 1 | 1 | | 0 | 120 | | 0 | 1 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | U | 25 | | 0 | 25 | | U |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 0.850 | 1.00 | 0.992 | 1.00 | 1.00 | 0.943 | 1.00 | 1.00 | 0.979 | 1.00 |
| Flt Protected | 0.950 | | 0.000 | 0.950 | 0.552 | | 0.950 | 0.540 | | 0.950 | 0.575 | |
| Satd. Flow (prot) | 1752 | 1844 | 1567 | 1787 | 1866 | 0 | 1787 | 1774 | 0 | 1752 | 1805 | 0 |
| Flt Permitted | 0.522 | 1044 | 1007 | 0.950 | 1000 | 0 | 0.463 | 1114 | U | 0.259 | 1000 | U |
| Satd. Flow (perm) | 963 | 1844 | 1567 | 1787 | 1866 | 0 | 871 | 1774 | 0 | 478 | 1805 | 0 |
| Right Turn on Red | 303 | 1044 | No | 1707 | 1000 | No | 071 | 1114 | No | 470 | 1005 | No |
| Satd. Flow (RTOR) | | | INU | | | NU | | | NU | | | NU |
| Link Speed (mph) | | 35 | | | 35 | | | 20 | | | 35 | |
| Link Distance (ft) | | 1453 | | | 677 | | | 1822 | | | 478 | |
| Travel Time (s) | | 28.3 | | | 13.2 | | | 62.1 | | | 9.3 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 0.90 | 496 | 0.90 91 | 246 | 388 | 21 | 132 | 250 | 153 | 28 | 231 | 0.90 |
| | 51 | 490 | 91 | 240 | 300 | 21 | 152 | 250 | 155 | 20 | 231 | 37 |
| Shared Lane Traffic (%) Lane Group Flow (vph) | 51 | 496 | 91 | 246 | 409 | 0 | 132 | 403 | 0 | 28 | 268 | 0 |
| , | No | | No | | | 0 No | No | | No | Zo No | | 0 |
| Enter Blocked Intersection | Left | No | | No Left | No Left | | | No | | | No Left | No |
| Lane Alignment | Leit | Left 12 | Right | Leit | 12 | Right | Left | Left 12 | Right | Left | 12 | Right |
| Median Width(ft) | | | | | | | | | | | | |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | 4.04 | Yes | 4.04 | 0.00 | Yes | 0.00 | 0.00 | 0.00 | 0.00 | 4.04 | Yes | 4.04 |
| Headway Factor | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 | | 9 | 15 | NIA | 9 | 15 | NIA | 9 | 15 | NIA | 9 |
| Turn Type | Perm | NA | Perm | Prot | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | 0 | 2 | • | 1 | 6 | | • | 8 | | | 4 | |
| Permitted Phases | 2 | • | 2 | | • | | 8 | 0 | | 4 | 4 | |
| Detector Phase | 2 | 2 | 2 | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | 10.0 | 40.0 | 40.0 | 7.0 | 40.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 24.0 | 24.0 | 24.0 | 14.0 | 17.0 | | 24.0 | 24.0 | | 14.0 | 14.0 | |
| Total Split (s) | 37.0 | 37.0 | 37.0 | 22.0 | 59.0 | | 31.0 | 31.0 | | 31.0 | 31.0 | |
| Total Split (%) | 41.1% | 41.1% | 41.1% | 24.4% | 65.6% | | 34.4% | 34.4% | | 34.4% | 34.4% | |
| Maximum Green (s) | 30.0 | 30.0 | 30.0 | 15.0 | 52.0 | | 24.0 | 24.0 | | 24.0 | 24.0 | |
| Yellow Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | | -2.0 | -2.0 | | -2.0 | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| Lead/Lag | Lag | Lag | Lag | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 2.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | Min | None | Min | | None | None | | None | None | |
| Walk Time (s) | 7.0 | 7.0 | 7.0 | | | | 7.0 | 7.0 | | | | |
| Flash Dont Walk (s) | 10.0 | 10.0 | 10.0 | | | | 10.0 | 10.0 | | | | |
| · · · | • | | | | | | 0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | 0 | 0 | | | | U | 0 | | | | |

2026 Background AM Peak Hour Timmons Group

Zebulon South TIA

| 2: NC-96 (| (Arendell Avenue) |) & NC-97 (| (Gannon Avenue) |) |
|------------|-------------------|-------------|-----------------|---|
|------------|-------------------|-------------|-----------------|---|

12/14/2023

| Lane GroupEBLActuated g/C Ratio0.33v/c Ratio0.16Control Delay21.4Queue Delay0.0Total Delay21.4LOSCApproach DelayApproach LOSQueue Length 50th (ft)20Queue Length 95th (ft)46Internal Link Dist (ft)200Base Capacity (vph)391Starvation Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:Area Type:OtherCycle Length: 90Actuated Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | EBT 0.33 0.81 36.7 D 33.2 C 245 367 1373 749 0 0 0 0 0 0 | EBR 0.33 0.17 21.0 0.0 21.0 C 35 69 100 637 0 0 0 0 | WBL 0.19 0.72 45.9 0.0 45.9 D 129 #238 350 385 0 0 0 0 | WBT 0.59 0.37 10.1 0.0 10.1 B 23.6 C 109 166 597 1311 0 0 0 | WBR | NBL 0.28 0.53 35.5 0.0 35.5 D 61 124 125 287 0 0 | NBT 0.28 0.80 41.7 0.0 41.7 D 40.1 D 203 #352 1742 585 0 0 0 | NBR | SBL 0.28 0.21 28.7 0.0 28.7 C 12 36 250 158 0 | SBT 0.28 0.52 29.8 0.0 29.8 C 29.7 C 123 203 398 596 0 | SBR |
|---|--|---|--|--|------------|--|---|-----|---|--|-----|
| v/c Ratio0.16Control Delay21.4Queue Delay0.0Total Delay21.4LOSCApproach DelayApproach LOSQueue Length 50th (ft)20Queue Length 95th (ft)46Internal Link Dist (ft)200Base Capacity (vph)391Starvation Cap Reductn0Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:Area Type:OtherCycle Length: 90Actuated Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.8111.5Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 0.81 36.7 0.0 36.7 D 33.2 C 245 367 1373 749 0 0 0 0 | 0.17 21.0 0.0 21.0 C 35 69 100 637 0 0 0 0 | 0.72 45.9 0.0 45.9 D 129 #238 350 385 0 0 | 0.37 10.1 0.0 10.1 B 23.6 C 109 166 597 1311 0 0 | | 0.53 35.5 0.0 35.5 D 61 124 125 287 0 | 0.80 41.7 0.0 41.7 D 40.1 D 203 #352 1742 585 0 | | 0.21 28.7 0.0 28.7 C 12 36 250 158 | 0.52 29.8 0.0 29.8 C 29.7 C 123 203 398 596 | |
| Control Delay21.4Queue Delay0.0Total Delay21.4LOSCApproach DelayApproach LOSQueue Length 50th (ft)20Queue Length 95th (ft)46Internal Link Dist (ft)200Base Capacity (vph)391Starvation Cap Reductn0Spillback Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 36.7 0.0 36.7 D 33.2 C 245 367 1373 749 0 0 0 0 | 21.0 0.0 21.0 C 35 69 100 637 0 0 0 0 | 45.9 0.0 45.9 D 129 #238 350 385 0 0 | 10.1 0.0 10.1 B 23.6 C 109 166 597 1311 0 0 | | 35.5 0.0 35.5 D 61 124 125 287 0 | 41.7 0.0 41.7 D 40.1 D 203 #352 1742 585 0 | | 28.7 0.0 28.7 C 12 36 250 158 | 29.8 0.0 29.8 C 29.7 C 123 203 398 596 | |
| Queue Delay0.0Total Delay21.4LOSCApproach DelayApproach LOSQueue Length 50th (ft)20Queue Length 95th (ft)46Internal Link Dist (ft)200Base Capacity (vph)391Starvation Cap Reductn0Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 0.0 36.7 D 33.2 C 245 367 1373 749 0 0 0 | 0.0 21.0 C 35 69 100 637 0 0 0 | 0.0 45.9 D 129 #238 350 385 0 0 | 0.0 10.1 B 23.6 C 109 166 597 1311 0 0 | | 0.0 35.5 D 61 124 125 287 0 | 0.0 41.7 D 40.1 D 203 #352 1742 585 0 | | 0.0 28.7 C 12 36 250 158 | 0.0 29.8 C 29.7 C 123 203 398 596 | |
| Total Delay21.4LOSCApproach DelayApproach LOSQueue Length 50th (ft)20Queue Length 95th (ft)46Internal Link Dist (ft)Turn Bay Length (ft)200Base Capacity (vph)391Starvation Cap Reductn0Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle Length: 80.8Natural Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 36.7 D 33.2 C 245 367 1373 749 0 0 0 | 21.0 C 35 69 100 637 0 0 0 | 45.9 D 129 #238 350 385 0 0 | 10.1 B 23.6 C 109 166 597 1311 0 0 | | 35.5 D 61 124 125 287 0 | 41.7 D 40.1 D 203 #352 1742 585 0 | | 28.7 C 12 36 250 158 | 29.8 C 29.7 C 123 203 398 596 | |
| LOS C Approach Delay Approach LOS Queue Length 50th (ft) 20 Queue Length 50th (ft) 46 Internal Link Dist (ft) 700 Base Capacity (vph) 391 Starvation Cap Reductn 0 Spillback Cap Reductn 0 Storage Cap Reductn 0 Storage Cap Reductn 0 Reduced v/c Ratio 0.13 Intersection Summary Area Type: Other Cycle Length: 90 Actuated Cycle Length: 80.8 Natural Cycle: 65 Control Type: Actuated-Uncoordinate Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | D 33.2 C 245 367 1373 749 0 0 0 | C 35 69 100 637 0 0 0 | D 129 #238 350 385 0 0 | B 23.6 C 109 166 597 1311 0 0 | | D 61 124 125 287 0 | D 40.1 D 203 #352 1742 585 0 | | C 12 36 250 158 | C 29.7 C 123 203 398 596 | |
| Approach DelayApproach LOSQueue Length 50th (ft)20Queue Length 95th (ft)46Internal Link Dist (ft)Turn Bay Length (ft)200Base Capacity (vph)391Starvation Cap Reductn0Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle Length: 80.8Natural Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 33.2 C 245 367 1373 749 0 0 0 | 35 69 100 637 0 0 0 | 129 #238 350 385 0 0 | 23.6 C 109 166 597 1311 0 0 | | 61 124 125 287 0 | 40.1 D 203 #352 1742 585 0 | | 12 36 250 158 | 29.7 C 123 203 398 596 | |
| Approach LOS Queue Length 50th (ft) 20 Queue Length 95th (ft) 46 Internal Link Dist (ft) 200 Base Capacity (vph) 391 Starvation Cap Reductn 0 Spillback Cap Reductn 0 Storage Cap Reductn 0 Storage Cap Reductn 0 Reduced v/c Ratio 0.13 Intersection Summary Area Type: Other Cycle Length: 90 Actuated Cycle Length: 80.8 Natural Cycle: 65 Control Type: Actuated-Uncoordinate Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | C 245 367 1373 749 0 0 0 | 69 100 637 0 0 0 | #238 350 385 0 0 | C 109 166 597 1311 0 0 | | 124 125 287 0 | D 203 #352 1742 585 0 | | 36 250 158 | C 123 203 398 596 | |
| Queue Length 50th (ft)20Queue Length 95th (ft)46Internal Link Dist (ft)700Turn Bay Length (ft)200Base Capacity (vph)391Starvation Cap Reductn0Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle Length: 80.8Natural Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 245 367 1373 749 0 0 0 | 69 100 637 0 0 0 | #238 350 385 0 0 | 109 166 597 1311 0 0 | | 124 125 287 0 | 203 #352 1742 585 0 | | 36 250 158 | 123 203 398 596 | |
| Queue Length 95th (ft)46Internal Link Dist (ft)700Turn Bay Length (ft)200Base Capacity (vph)391Starvation Cap Reductn0Storage Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle Length: 80.8Natural Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 367 1373 749 0 0 0 | 69 100 637 0 0 0 | #238 350 385 0 0 | 166 597 1311 0 0 | | 124 125 287 0 | #352 1742 585 0 | | 36 250 158 | 203 398 596 | |
| Internal Link Dist (ft)Turn Bay Length (ft)200Base Capacity (vph)391Starvation Cap Reductn0Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle Length: 80.8Natural Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 1373 749 0 0 0 | 100 637 0 0 0 | 350 385 0 0 | 597 1311 0 0 | | 125 287 0 | 1742 585 0 | | 250 158 | 398 596 | |
| Turn Bay Length (ft)200Base Capacity (vph)391Starvation Cap Reductn0Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 900Actuated Cycle Length: 80.80Natural Cycle: 650.13Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.810.81Intersection Signal Delay: 31.50.81 | 749 0 0 0 | 637 0 0 0 | 385 0 0 | 1311 0 0 | | 287 0 | 585 0 | | 158 | 596 | |
| Base Capacity (vph)391Starvation Cap Reductn0Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle Length: 80.8Natural Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 0 0 0 | 637 0 0 0 | 385 0 0 | 0 0 | | 287 0 | 0 | | 158 | | |
| Starvation Cap Reductn0Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle Length: 80.8Natural Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 0 0 0 | 0 0 0 | 0 0 | 0 0 | | 0 | 0 | | | | |
| Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle Length: 80.8Natural Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 0 0 | 0 | 0 | 0 | | | - | | 0 | 0 | |
| Storage Cap Reductn0Reduced v/c Ratio0.13Intersection SummaryArea Type:OtherCycle Length: 90Actuated Cycle Length: 80.8Natural Cycle: 65Control Type: Actuated-UncoordinateMaximum v/c Ratio: 0.81Intersection Signal Delay: 31.5Intersection Capacity Utilization 78.5 | 0 | 0 | | | | 0 | 0 | | | | |
| Reduced v/c Ratio 0.13 Intersection Summary Area Type: Other Cycle Length: 90 Actuated Cycle Length: 80.8 Natural Cycle: 65 Control Type: Actuated-Uncoordinate Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | | | 0 | ٥ | | - | 0 | | 0 | 0 | |
| Intersection Summary Area Type: Other Cycle Length: 90 Actuated Cycle Length: 80.8 Natural Cycle: 65 Control Type: Actuated-Uncoordinate Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | 0.66 | • • • • | | 0 | | 0 | 0 | | 0 | 0 | |
| Area Type: Other Cycle Length: 90 Actuated Cycle Length: 80.8 Natural Cycle: 65 Control Type: Actuated-Uncoordinate Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | | 0.14 | 0.64 | 0.31 | | 0.46 | 0.69 | | 0.18 | 0.45 | |
| Cycle Length: 90 Actuated Cycle Length: 80.8 Natural Cycle: 65 Control Type: Actuated-Uncoordinate Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | | | | | | | | | | | |
| Actuated Cycle Length: 80.8 Natural Cycle: 65 Control Type: Actuated-Uncoordinate Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | | | | | | | | | | | |
| Natural Cycle: 65 Control Type: Actuated-Uncoordinate Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | | | | | | | | | | | |
| Control Type: Actuated-Uncoordinate Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | | | | | | | | | | | |
| Maximum v/c Ratio: 0.81 Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | | | | | | | | | | | |
| Intersection Signal Delay: 31.5 Intersection Capacity Utilization 78.5 | ed | | | | | | | | | | |
| Intersection Capacity Utilization 78.5 | | | | | | | | | | | |
| | | | | tersection | | | | | | | |
| | % | | IC | CU Level | of Service | e D | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | |
| # 95th percentile volume exceeds | | | ay be long | ger. | | | | | | | |
| Queue shown is maximum after t | wo cycles. | | | | | | | | | | |
| Splits and Phases: 2: NC-96 (Arer | ndell Aven | ue) & NC | -97 (Gan | non Aver | iue) | | | | | | |
| 1 01 | A | | | | | | Ø4 | | | | |
| | Ø2 | | | | | 31 | | | | | |
| 22.3 D/ | | | | | | 51 | <u>.</u> | | | | |
| Ø6 | S | | | | | 11 | Tø8 | | | | |

| Intersection | |
|------------------|-----|
| Int Delay, s/veh | 2.2 |

| Int Delay, 5/Ven | 2.2 | | | | | | | | | | | | |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|--|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | | \$ | | | \$ | | | \$ | | | \$ | | |
| Traffic Vol, veh/h | 34 | 4 | 18 | 4 | 4 | 4 | 48 | 330 | 4 | 4 | 167 | 20 | |
| Future Vol, veh/h | 34 | 4 | 18 | 4 | 4 | 4 | 48 | 330 | 4 | 4 | 167 | 20 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | |
| RT Channelized | - | - | None | |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - | |
| Veh in Median Storage | ,# - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 38 | 4 | 20 | 4 | 4 | 4 | 53 | 367 | 4 | 4 | 186 | 22 | |
| | | | | | | | | | | | | | |

| Major/Minor | Minor2 | | | Vinor1 | | | Major1 | | N | lajor2 | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|-----|--------|---|---|--|
| Conflicting Flow All | 684 | 682 | 197 | 692 | 691 | 369 | 208 | 0 | 0 | 371 | 0 | 0 | |
| Stage 1 | 205 | 205 | - | 475 | 475 | - | - | - | - | - | - | - | |
| Stage 2 | 479 | 477 | - | 217 | 216 | - | - | - | - | - | - | - | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - 1 | 2.218 | - | - | |
| Pot Cap-1 Maneuver | 363 | 372 | 844 | 358 | 368 | 677 | 1363 | - | - | 1188 | - | - | |
| Stage 1 | 797 | 732 | - | 570 | 557 | - | - | - | - | - | - | - | |
| Stage 2 | 568 | 556 | - | 785 | 724 | - | - | - | - | - | - | - | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | |
| Mov Cap-1 Maneuver | · 343 | 352 | 844 | 332 | 348 | 677 | 1363 | - | - | 1188 | - | - | |
| Mov Cap-2 Maneuver | · 343 | 352 | - | 332 | 348 | - | - | - | - | - | - | - | |
| Stage 1 | 758 | 729 | - | 542 | 530 | - | - | - | - | - | - | - | |
| Stage 2 | 532 | 529 | - | 759 | 721 | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | |

| Approach | EB | WB | NB | SB | |
|----------------------|------|------|----|-----|--|
| HCM Control Delay, s | 14.9 | 14.1 | 1 | 0.2 | |
| HCM LOS | В | В | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1V | VBLn1 | SBL | SBT | SBR | |
|-----------------------|-------|-----|-----|--------|-------|-------|-----|-----|--|
| Capacity (veh/h) | 1363 | - | - | 425 | 407 | 1188 | - | - | |
| HCM Lane V/C Ratio | 0.039 | - | - | 0.146 | 0.033 | 0.004 | - | - | |
| HCM Control Delay (s) | 7.7 | 0 | - | 14.9 | 14.1 | 8 | 0 | - | |
| HCM Lane LOS | А | А | - | В | В | А | А | - | |
| HCM 95th %tile Q(veh) | 0.1 | - | - | 0.5 | 0.1 | 0 | - | - | |

Zebulon South TIA 6: NC-96 (Arendell Avenue) & Perry Curtis Road

| Intersection | |
|--------------|--|
| | |

| Int Delay, s/veh | 3.1 | | | | | |
|------------------------|--------|------|------|------|------|------|
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | Þ | | | र्स |
| Traffic Vol, veh/h | 21 | 93 | 231 | 6 | 35 | 119 |
| Future Vol, veh/h | 21 | 93 | 231 | 6 | 35 | 119 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 23 | 103 | 257 | 7 | 39 | 132 |

| Major/Minor | Minor1 | Ν | /lajor1 | Ν | /lajor2 | |
|----------------------|--------|-------|---------|-------|---------|-----|
| Conflicting Flow All | 471 | 261 | 0 | 0 | 264 | 0 |
| Stage 1 | 261 | - | - | - | - | - |
| Stage 2 | 210 | | - | - | - | - |
| Critical Hdwy | 6.42 | | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | | - | - | - | - |
| Follow-up Hdwy | | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuve | | | - | - | 1300 | - |
| Stage 1 | 783 | | - | - | - | - |
| Stage 2 | 825 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuv | | | - | - | 1300 | - |
| Mov Cap-2 Maneuv | | | - | - | - | - |
| Stage 1 | 783 | - | - | - | - | - |
| Stage 2 | 799 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, | s 11.1 | | 0 | | 1.8 | |
| HCM LOS | В | | | | | |
| | | | | | | |
| Minor Lane/Major M | lvmt | NBT | NBRW | BLn1 | SBL | SBT |
| Capacity (veh/h) | | - | - | 717 | 1300 | - |
| HCM Lane V/C Rati | 0 | - | - (| 0.177 | 0.03 | - |
| HCM Control Delay | (s) | - | - | 11.1 | 7.9 | 0 |

HCM Lane LOS

HCM 95th %tile Q(veh)

-

-

-

-

В

0.6

А

0.1

А

-

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/05/2023

| | ٠ | + | + | 4 | ← | • | 1 | t | 1 | 4 | ţ | ~ |
|------------------------------------|------------|----------|-------|------------|---------|-------|------------|---------|-------|------------|------------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ٦ | 4Î | | ٦ | ţ, | | | 4 | | | \$ | |
| Traffic Volume (vph) | 10 | 705 | 113 | 55 | 581 | 4 | 90 | 10 | 68 | 7 | 19 | 29 |
| Future Volume (vph) | 10 | 705 | 113 | 55 | 581 | 4 | 90 | 10 | 68 | 7 | 19 | 29 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.979 | | | 0.999 | | | 0.945 | | | 0.929 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.974 | | | 0.993 | |
| Satd. Flow (prot) | 1778 | 1833 | 0 | 1770 | 1861 | 0 | 0 | 1689 | 0 | 0 | 1710 | 0 |
| Flt Permitted | 0.418 | | | 0.950 | | | | 0.815 | | | 0.956 | |
| Satd. Flow (perm) | 783 | 1833 | 0 | 1770 | 1861 | 0 | 0 | 1413 | 0 | 0 | 1646 | 0 |
| Right Turn on Red | | | No | | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 11 | 783 | 126 | 61 | 646 | 4 | 100 | 11 | 76 | 8 | 21 | 32 |
| Shared Lane Traffic (%) | | | | | | - | - | | | - | • • | |
| Lane Group Flow (vph) | 11 | 909 | 0 | 61 | 650 | 0 | 0 | 187 | 0 | 0 | 61 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | | 12 | | | 12 | | | 0 | | | 0 | |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | 0.00 | Yes | 0.00 | 1 00 | Yes | 1 00 | 4 00 | 4 00 | 4 00 | 1.04 | 4.04 | 1.04 |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 Dorm | NIA | 9 | 15 Drot | NA | 9 | 15 Dorm | NA | 9 | 15 Dorm | NIA | 9 |
| Turn Type Protected Phases | Perm | NA 2 | | Prot 1 | NA 6 | | Perm | NA 8 | | Perm | NA | |
| | 0 | 2 | | 1 | 0 | | 0 | 0 | | 1 | 4 | |
| Permitted Phases Detector Phase | 2 | 2 | | 1 | 6 | | 8 8 | 8 | | 4 | 4 | |
| Switch Phase | Z | 2 | | 1 | 0 | | 0 | 0 | | 4 | 4 | |
| Minimum Initial (s) | 10.0 | 10.0 | | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 17.0 | 17.0 | | 14.0 | 17.0 | | 14.0 | 14.0 | | 14.0 | 14.0 | |
| Total Split (s) | 52.0 | 52.0 | | 14.0 | 66.0 | | 24.0 | 24.0 | | 24.0 | 24.0 | |
| Total Split (%) | 57.8% | 57.8% | | 15.6% | 73.3% | | 26.7% | 26.7% | | 26.7% | 24.0 | |
| Maximum Green (s) | 45.0 | 45.0 | | 7.0 | 59.0 | | 17.0 | 17.0 | | 17.0 | 17.0 | |
| Yellow Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | | -2.0 | -2.0 | | 2.0 | -2.0 | | 2.0 | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | |
| Lead/Lag | Lag | Lag | | Lead | 0.0 | | | 0.0 | | | 0.0 | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | | None | Min | | None | None | | None | None | |
| Act Effct Green (s) | 46.7 | 46.7 | | 9.2 | 57.2 | | 110110 | 16.2 | | 110110 | 16.2 | |
| Actuated g/C Ratio | 0.56 | 0.56 | | 0.11 | 0.68 | | | 0.19 | | | 0.19 | |
| v/c Ratio | 0.03 | 0.89 | | 0.31 | 0.51 | | | 0.68 | | | 0.19 | |
| Control Delay | 10.7 | 31.4 | | 42.3 | 8.4 | | | 46.4 | | | 31.2 | |
| | | U | | .2.0 | 0.1 | | | | | | 0 <u>C</u> | |

2026 Background PM Peak Hour Timmons Group

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/05/2023

| | ٠ | - | 7 | 1 | + | * | 1 | t | 1 | 4 | ţ | 4 |
|-------------------------------|-------------|------------|----------|------------|----------|------------|-----|------|------|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 10.7 | 31.4 | | 42.3 | 8.4 | | | 46.4 | | | 31.2 | |
| LOS | В | С | | D | А | | | D | | | С | |
| Approach Delay | | 31.1 | | | 11.3 | | | 46.4 | | | 31.2 | |
| Approach LOS | | С | | | В | | | D | | | С | |
| Queue Length 50th (ft) | 3 | 452 | | 32 | 153 | | | 97 | | | 29 | |
| Queue Length 95th (ft) | 11 | #744 | | 72 | 237 | | | 169 | | | 63 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 448 | 1051 | | 194 | 1385 | | | 327 | | | 381 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.02 | 0.86 | | 0.31 | 0.47 | | | 0.57 | | | 0.16 | |
| Intersection Summary | | | | | | | | | | | | |
| | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 83. | 6 | | | | | | | | | | | |
| Natural Cycle: 80 | | | | | | | | | | | | |
| Control Type: Actuated-Uno | coordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.89 | | | | | | | | | | | | |
| Intersection Signal Delay: 2 | | | | | | n LOS: C | | | | | | |
| Intersection Capacity Utiliza | ation 70.4% | 6 | | IC | CU Level | of Service | ЭC | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | | | ueue ma | iy be long | ger. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 1: S V | Wakefield S | Street & N | IC-97 (G | annon Av | /enue) | | | | | | | |
| | | | | | | | | | 1 | | | |
| ▼Ø1 | Ø2 | | | | | | | 12.0 | • Ø4 | | | |

| Ø1 | | Ø4 |
|------|------|--------------|
| 14 s | 52 s | 24 s |
| ←Ø6 | | ∞1 Ø8 |
| 66 s | | 24 s |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/05/2023

| | ٠ | + | 1 | 4 | Ŧ | * | 1 | t | 1 | 1 | Ļ | ~ |
|----------------------------|--------|-------|--------|-------|-------|--------|--------|-------|--------|--------|-------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ٦ | 1 | 1 | ٦ | ţ, | | ۲ | Þ | | 5 | ţ, | |
| Traffic Volume (vph) | 66 | 352 | 108 | 245 | 494 | 57 | 102 | 341 | 154 | 51 | 254 | 50 |
| Future Volume (vph) | 66 | 352 | 108 | 245 | 494 | 57 | 102 | 341 | 154 | 51 | 254 | 50 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | 1000 | 2% | 1000 | 1000 | -2% | 1000 | 1000 | -2% | 1000 | 1000 | 2% | 1000 |
| Storage Length (ft) | 200 | 270 | 100 | 350 | 270 | 0 | 125 | 270 | 0 | 250 | 270 | 0 |
| Storage Lanes | 1 | | 1 | 1 | | 0 | 120 | | 0 | 1 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | Ŭ | 25 | | Ű | 25 | | Ű |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 0.850 | 1.00 | 0.985 | 1.00 | 1.00 | 0.953 | 1.00 | 1.00 | 0.975 | 1.00 |
| Flt Protected | 0.950 | | 0.000 | 0.950 | 0.000 | | 0.950 | 0.000 | | 0.950 | 0.070 | |
| Satd. Flow (prot) | 1752 | 1844 | 1567 | 1787 | 1853 | 0 | 1787 | 1793 | 0 | 1752 | 1798 | 0 |
| Flt Permitted | 0.433 | 1044 | 1007 | 0.950 | 1000 | Ū | 0.418 | 1150 | 0 | 0.170 | 1100 | U |
| Satd. Flow (perm) | 799 | 1844 | 1567 | 1787 | 1853 | 0 | 786 | 1793 | 0 | 313 | 1798 | 0 |
| Right Turn on Red | 100 | 1044 | No | 1101 | 1000 | No | 100 | 1755 | No | 010 | 1750 | No |
| Satd. Flow (RTOR) | | | NO | | | NO | | | NO | | | NO |
| Link Speed (mph) | | 35 | | | 35 | | | 20 | | | 35 | |
| Link Distance (ft) | | 1453 | | | 677 | | | 1822 | | | 478 | |
| Travel Time (s) | | 28.3 | | | 13.2 | | | 62.1 | | | 9.3 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 73 | 391 | 120 | 272 | 549 | 63 | 113 | 379 | 171 | 57 | 282 | 56 |
| Shared Lane Traffic (%) | 75 | 551 | 120 | 212 | 545 | 00 | 115 | 515 | 17.1 | 51 | 202 | 50 |
| Lane Group Flow (vph) | 73 | 391 | 120 | 272 | 612 | 0 | 113 | 550 | 0 | 57 | 338 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | Leit | 12 | rtight | Leit | 12 | rtight | Len | 12 | rtight | Leit | 12 | rugin |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | Yes | | | 10 | | | Yes | |
| Headway Factor | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 1.01 | 1.01 | 9 | 15 | 0.99 | 0.99 | 15 | 0.99 | 0.99 | 1.01 | 1.01 | 9 |
| Turn Type | Perm | NA | Perm | Prot | NA | 9 | Perm | NA | 9 | Perm | NA | 9 |
| Protected Phases | Feilli | 2 | Feilii | 1 | 6 | | Feilii | 8 | | Feilli | 4 | |
| Permitted Phases | 2 | 2 | 2 | 1 | 0 | | 8 | 0 | | 4 | 4 | |
| Detector Phase | 2 | 2 | 2 | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | 2 | Z | Z | I | 0 | | 0 | 0 | | 4 | 4 | |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| | 24.0 | 24.0 | 24.0 | 14.0 | | | | 24.0 | | 14.0 | 14.0 | |
| Minimum Split (s) | | | | 22.0 | 17.0 | | 24.0 | | | | 38.0 | |
| Total Split (s) | 30.0 | 30.0 | 30.0 | | 52.0 | | 38.0 | 38.0 | | 38.0 | | |
| Total Split (%) | 33.3% | 33.3% | 33.3% | 24.4% | 57.8% | | 42.2% | 42.2% | | 42.2% | 42.2% | |
| Maximum Green (s) | 23.0 | 23.0 | 23.0 | 15.0 | 45.0 | | 31.0 | 31.0 | | 31.0 | 31.0 | |
| Yellow Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | | -2.0 | -2.0 | | -2.0 | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| Lead/Lag | Lag | Lag | Lag | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | 0.0 | | | 0.0 | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 2.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | Min | None | Min | | None | None | | None | None | |
| Walk Time (s) | 7.0 | 7.0 | 7.0 | | | | 7.0 | 7.0 | | | | |
| Flash Dont Walk (s) | 10.0 | 10.0 | 10.0 | | | | 10.0 | 10.0 | | | | |
| Pedestrian Calls (#/hr) | 0 | 0 | 0 | | | | 0 | 0 | | | | |
| Act Effct Green (s) | 22.2 | 22.2 | 22.2 | 16.2 | 43.5 | | 29.5 | 29.5 | | 29.5 | 29.5 | |

2026 Background PM Peak Hour Timmons Group

Zebulon South TIA

| 2: NC-96 | (Arendell Avenue) |) & NC-97 (| (Gannon Avenue) |) |
|----------|-------------------|-------------|-----------------|---|
|----------|-------------------|-------------|-----------------|---|

12/05/2023

| | 1 | → | 7 | 1 | + | * | 1 | Ť | 1 | 1 | ŧ | ~ |
|-------------------------------|-------------|------------|----------|-----------|------------|------------|------|------|-----|------|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBF |
| Actuated g/C Ratio | 0.27 | 0.27 | 0.27 | 0.19 | 0.52 | | 0.35 | 0.35 | | 0.35 | 0.35 | |
| v/c Ratio | 0.34 | 0.79 | 0.29 | 0.78 | 0.63 | | 0.41 | 0.86 | | 0.52 | 0.53 | |
| Control Delay | 31.3 | 42.4 | 27.5 | 51.2 | 18.3 | | 26.5 | 41.3 | | 41.7 | 25.4 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 31.3 | 42.4 | 27.5 | 51.2 | 18.3 | | 26.5 | 41.3 | | 41.7 | 25.4 | |
| LOS | С | D | С | D | В | | С | D | | D | С | |
| Approach Delay | | 38.0 | | | 28.4 | | | 38.7 | | | 27.8 | |
| Approach LOS | | D | | | С | | | D | | | С | |
| Queue Length 50th (ft) | 33 | 203 | 53 | 149 | 235 | | 47 | 281 | | 25 | 147 | |
| Queue Length 95th (ft) | 73 | #336 | 100 | #276 | 348 | | 96 | #462 | | #77 | 230 | |
| Internal Link Dist (ft) | | 1373 | | | 597 | | | 1742 | | | 398 | |
| Turn Bay Length (ft) | 200 | | 100 | 350 | | | 125 | | | 250 | | |
| Base Capacity (vph) | 244 | 564 | 479 | 371 | 1065 | | 317 | 724 | | 126 | 726 | |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Reduced v/c Ratio | 0.30 | 0.69 | 0.25 | 0.73 | 0.57 | | 0.36 | 0.76 | | 0.45 | 0.47 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: 0 | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 83.2 | 2 | | | | | | | | | | | |
| Natural Cycle: 75 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | coordinated | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.86 | | | | | | | | | | | | |
| Intersection Signal Delay: 33 | 3.2 | | | In | tersection | n LOS: C | | | | | | |
| Intersection Capacity Utiliza | ation 87.6% | , 0 | | IC | CU Level | of Service | eΕ | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume e | exceeds ca | apacity, q | ueue ma | y be long | ger. | | | | | | | |
| Queue shown is maximu | ım after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 2: NC | -96 (Arend | ell Aveni | ie) & NC | -97 (Gan | non Aver | nue) | | | | | | |
| | | | | | | | | | | | | |
| ▼ Ø1 22 s | 30 s | Ø2 | | | | 38 s | Ø4 | | | | | |

¶ø8

Ø6

| Intersection | | | | | | | |
|------------------|--|--|--|--|--|--|--|
| Int Delay, s/veh | | | | | | | |

| Int Delay, s/veh | 3.7 | | | | | | | | | | | | |
|------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|--|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | | \$ | | | \$ | | | \$ | | | \$ | | |
| Traffic Vol, veh/h | 51 | 5 | 73 | 4 | 4 | 7 | 36 | 268 | 4 | 9 | 437 | 47 | |
| Future Vol, veh/h | 51 | 5 | 73 | 4 | 4 | 7 | 36 | 268 | 4 | 9 | 437 | 47 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None | |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - | |
| Veh in Median Storage | e, # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 57 | 6 | 81 | 4 | 4 | 8 | 40 | 298 | 4 | 10 | 486 | 52 | |

| Major/Minor | Minor2 | | I | Minor1 | | | Major1 | | Μ | lajor2 | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|-----|--------|---|---|--|
| Conflicting Flow All | 918 | 914 | 512 | 956 | 938 | 300 | 538 | 0 | 0 | 302 | 0 | 0 | |
| Stage 1 | 532 | 532 | - | 380 | 380 | - | - | - | - | - | - | - | |
| Stage 2 | 386 | 382 | - | 576 | 558 | - | - | - | - | - | - | - | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - 2 | 2.218 | - | - | |
| Pot Cap-1 Maneuver | 252 | 273 | 562 | 238 | 264 | 740 | 1030 | - | - | 1259 | - | - | |
| Stage 1 | 531 | 526 | - | 642 | 614 | - | - | - | - | - | - | - | |
| Stage 2 | 637 | 613 | - | 503 | 512 | - | - | - | - | - | - | - | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | |
| Mov Cap-1 Maneuver | 235 | 257 | 562 | 192 | 249 | 740 | 1030 | - | - | 1259 | - | - | |
| Mov Cap-2 Maneuver | 235 | 257 | - | 192 | 249 | - | - | - | - | - | - | - | |
| Stage 1 | 506 | 520 | - | 612 | 585 | - | - | - | - | - | - | - | |
| Stage 2 | 596 | 584 | - | 421 | 506 | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | |

| Approach | EB | WB | NB | SB | |
|----------------------|------|------|----|-----|--|
| HCM Control Delay, s | 22.1 | 16.7 | 1 | 0.1 | |
| HCM LOS | С | С | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1 | VBLn1 | SBL | SBT | SBR | |
|-----------------------|-------|-----|-----|-------|-------|-------|-----|-----|--|
| Capacity (veh/h) | 1030 | - | - | 352 | 324 | 1259 | - | - | |
| HCM Lane V/C Ratio | 0.039 | - | - | 0.407 | 0.051 | 0.008 | - | - | |
| HCM Control Delay (s) | 8.6 | 0 | - | 22.1 | 16.7 | 7.9 | 0 | - | |
| HCM Lane LOS | А | А | - | С | С | А | А | - | |
| HCM 95th %tile Q(veh) | 0.1 | - | - | 1.9 | 0.2 | 0 | - | - | |

Zebulon South TIA <u>6: NC-96 (Arendell Avenue) & Perry Curtis Road</u>

| 12/05/2023 |
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|------------|

| Intersection | | | | | | | |
|------------------------|--------|------|------|------|------|------|---|
| Int Delay, s/veh | 2.8 | | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT | Γ |
| Lane Configurations | Y | | ħ | | | 4 | 1 |
| Traffic Vol, veh/h | 11 | 63 | 168 | 15 | 133 | 281 | 1 |
| Future Vol, veh/h | 11 | 63 | 168 | 15 | 133 | 281 | 1 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |) |
| Sign Control | Stop | Stop | Free | Free | Free | Free | 9 |
| RT Channelized | - | None | - | None | - | None | 9 |
| Storage Length | 0 | - | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |) |
| Grade, % | 0 | - | 0 | - | - | 0 |) |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |) |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 12 | 70 | 187 | 17 | 148 | 312 | 2 |
| | | | | | | | |

| Major/Minor | Minor1 | Ν | /lajor1 | 1 | Major2 | | | | | |
|----------------------|--------|------|---------|--------------|--------|-----|------|------|--|--|
| Conflicting Flow All | 804 | 196 | 0 | 0 | 204 | 0 | | | | |
| Stage 1 | 196 | - | - | - | - | - | | | | |
| Stage 2 | 608 | - | - | - | - | - | | | | |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - | | | | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | | | | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | | | | |
| Follow-up Hdwy | 3.518 | | - | - | 2.218 | - | | | | |
| Pot Cap-1 Maneuver | | 845 | - | - | 1368 | - | | | | |
| Stage 1 | 837 | - | - | - | - | - | | | | |
| Stage 2 | 543 | - | - | - | - | - | | | | |
| Platoon blocked, % | | | - | - | | - | | | | |
| Mov Cap-1 Maneuve | | 845 | - | - | 1368 | - | | | | |
| Mov Cap-2 Maneuve | | - | - | - | - | - | | | | |
| Stage 1 | 837 | - | - | - | - | - | | | | |
| Stage 2 | 472 | - | - | - | - | - | | | | |
| | | | | | | | | | | |
| Approach | WB | | NB | | SB | | | | | |
| HCM Control Delay, s | s 11.1 | | 0 | | 2.6 | | | | | |
| HCM LOS | В | | | | | | | | | |
| | | | | | | | | | | |
| Miner Lene (Meier Mu | une t | NDT | | <u>1</u> ا م | CDI | ODT | | | | |
| Minor Lane/Major Mv | mt | NBT | NBRW | | SBL | SBT | | | | |
| Capacity (veh/h) | | - | - | 670 | 1368 | - | | | | |
| HCM Lane V/C Ratio | | - | - | | 0.108 | - | | | | |
| HCM Control Delay (| S) | - | - | 11.1 | 8 | 0 | | | | |
| HCM Lane LOS | | - | - | В | A | А | | | | |

0.4

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0.4

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HCM 95th %tile Q(veh)

2026 Build + Improvement Traffic Volumes

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/05/2023

| | ٠ | + | * | 4 | Ļ | • | 1 | t | 1 | 1 | ţ | ~ |
|----------------------------------|-------|---------|-------|------------|---------|-------|-------|----------|-------|-------|----------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ٦ | ĥ | | 5 | ţ, | | | 4 | | | \$ | |
| Traffic Volume (vph) | 6 | 499 | 52 | 51 | 567 | 4 | 126 | 10 | 130 | 4 | 5 | 19 |
| Future Volume (vph) | 6 | 499 | 52 | 51 | 567 | 4 | 126 | 10 | 130 | 4 | 5 | 19 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | Ŭ | 25 | | • | 25 | | | 25 | | Ū |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.986 | | | 0.999 | | | 0.934 | | | 0.909 | |
| Flt Protected | 0.950 | 0.000 | | 0.950 | 0.000 | | | 0.977 | | | 0.994 | |
| Satd. Flow (prot) | 1778 | 1846 | 0 | 1770 | 1861 | 0 | 0 | 1674 | 0 | 0 | 1675 | 0 |
| Flt Permitted | 0.405 | 1010 | Ū | 0.950 | 1001 | v | Ū | 0.833 | Ū | v | 0.951 | Ŭ |
| Satd. Flow (perm) | 758 | 1846 | 0 | 1770 | 1861 | 0 | 0 | 1428 | 0 | 0 | 1602 | 0 |
| Right Turn on Red | 100 | 1040 | No | 1110 | 1001 | No | U | 1420 | No | U | 1002 | No |
| Satd. Flow (RTOR) | | | NO | | | NO | | | 110 | | | NO |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 0.90 | 554 | 58 | 0.90 57 | 630 | 4 | 140 | 11 | 144 | 0.90 | 0.30 | 21 |
| Shared Lane Traffic (%) | 1 | 554 | 50 | 57 | 030 | 4 | 140 | 11 | 144 | 4 | 0 | 21 |
| Lane Group Flow (vph) | 7 | 612 | 0 | 57 | 634 | 0 | 0 | 295 | 0 | 0 | 31 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | | Left | Left | | Left | Left | |
| Median Width(ft) | Leit | 12 | Right | Leit | 12 | Right | Leit | Len 0 | Right | Leit | Len 0 | Right |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | Yes | | | 10 | | | 10 | |
| • | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 9 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) Turn Type | | NA | 9 | | NA | 9 | | NA | 9 | | NA | 9 |
| 21 21 | Perm | NA 2 | | Prot 1 | NA 6 | | Perm | NA 8 | | Perm | | |
| Protected Phases | 0 | Z | | I | 0 | | 0 | Ö | | 4 | 4 | |
| Permitted Phases | 2 | 0 | | 4 | c | | 8 | 0 | | 4 | 4 | |
| Detector Phase | 2 | 2 | | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | 40.0 | 40.0 | | 7.0 | 40.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Initial (s) | 10.0 | 10.0 | | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 17.0 | 17.0 | | 14.0 | 17.0 | | 14.0 | 14.0 | | 14.0 | 14.0 | |
| Total Split (s) | 46.0 | 46.0 | | 14.0 | 60.0 | | 30.0 | 30.0 | | 30.0 | 30.0 | |
| Total Split (%) | 51.1% | 51.1% | | 15.6% | 66.7% | | 33.3% | 33.3% | | 33.3% | 33.3% | |
| Maximum Green (s) | 39.0 | 39.0 | | 7.0 | 53.0 | | 23.0 | 23.0 | | 23.0 | 23.0 | _ |
| Yellow Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | | -2.0 | -2.0 | | | -2.0 | | | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | | None | Min | | None | None | | None | None | |
| Act Effct Green (s) | 30.6 | 30.6 | | 10.1 | 37.4 | | | 21.0 | | | 21.0 | |
| Actuated g/C Ratio | 0.44 | 0.44 | | 0.14 | 0.54 | | | 0.30 | | | 0.30 | |
| | | 0 70 | | 0.00 | 0.64 | | | 0.68 | | | 0.06 | |
| v/c Ratio | 0.02 | 0.76 | | 0.22 | 0.64 | | | 35.1 | | | 23.0 | |

2026 Build + IMP AM Peak Hour Timmons Group

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/05/2023

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|-------------------------------|-------------|------------|----------|------------|---------|------------|-----|------|-----|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBF |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 14.0 | 25.3 | | 37.3 | 14.0 | | | 35.1 | | | 23.0 | |
| LOS | В | С | | D | В | | | D | | | С | |
| Approach Delay | | 25.1 | | | 15.9 | | | 35.1 | | | 23.0 | |
| Approach LOS | | С | | | В | | | D | | | С | |
| Queue Length 50th (ft) | 2 | 262 | | 26 | 182 | | | 131 | | | 11 | |
| Queue Length 95th (ft) | 10 | 405 | | 68 | 287 | | | #273 | | | 35 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 483 | 1177 | | 257 | 1463 | | | 575 | | | 646 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.01 | 0.52 | | 0.22 | 0.43 | | | 0.51 | | | 0.05 | |
| Intersection Summary | | | | | | | | | | | | |
| | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 69. | 7 | | | | | | | | | | | |
| Natural Cycle: 65 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | coordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.76 | | | | | | | | | | | | |
| Intersection Signal Delay: 2 | | | | | | n LOS: C | | | | | | |
| Intersection Capacity Utiliza | ation 72.9% | 0 | | IC | U Level | of Service | ЭC | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | | | ueue ma | iy be long | jer. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 1: S V | Vakefield S | Street & N | IC-97 (G | annon Av | (enue) | | | | | | | |
| | 3 | | | | | | 8 | ~ | | | | |

| √ Ø1 | | Ø4 |
|----------------|------|------|
| 14 s | 46 s | 30 s |
| ← Ø6 | | ≪ ø8 |
| 60 s | | 30 s |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/05/2023

| | ٦ | + | 7 | 1 | Ŧ | * | 1 | t | 1 | 1 | ţ | 4 |
|---|--------------|-------------|------------|-----------|-----------|---------|--------------|--------------|--------------|----------|-------------|------------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 1 | 1 | 1 | 5 | 1 | TIBI(| 5 | 1 | NB IX | 5 | 1 | |
| Traffic Volume (vph) | 46 | 463 | 87 | 232 | 354 | 19 | 136 | 250 | 172 | 25 | 216 | 33 |
| Future Volume (vph) | 46 | 463 | 87 | 232 | 354 | 19 | 136 | 250 | 172 | 25 | 216 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | 1500 | 2% | 1500 | 1500 | -2% | 1500 | 1500 | -2% | 1500 | 1500 | 2% | 1500 |
| Storage Length (ft) | 200 | 2 /0 | 100 | 350 | -270 | 0 | 125 | -270 | 0 | 250 | 270 | 0 |
| Storage Lanes | 200 | | 100 | 1 | | 0 | 125 | | 0 | 200 | | 0 |
| Taper Length (ft) | 25 | | 1 | 25 | | 0 | 25 | | 0 | 25 | | 0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 0.850 | 1.00 | 0.992 | 1.00 | 1.00 | 0.939 | 1.00 | 1.00 | 0.980 | 1.00 |
| Fit Protected | 0.950 | | 0.000 | 0.950 | 0.332 | | 0.950 | 0.959 | | 0.950 | 0.900 | |
| Satd. Flow (prot) | 1752 | 1844 | 1567 | 1787 | 1866 | 0 | 1787 | 1767 | 0 | 1752 | 1807 | 0 |
| Flt Permitted | 0.520 | 1044 | 1307 | 0.950 | 1000 | U | 0.466 | 1707 | 0 | 0.205 | 1007 | 0 |
| Satd. Flow (perm) | 959 | 1844 | 1567 | 1787 | 1866 | 0 | 877 | 1767 | 0 | 378 | 1807 | 0 |
| Right Turn on Red | 909 | 1044 | No | 1707 | 1000 | No | 011 | 1707 | No | 570 | 1007 | No |
| Satd. Flow (RTOR) | | | INU | | | INU | | | INU | | | INU |
| Link Speed (mph) | | 35 | | | 35 | | | 20 | | | 35 | |
| , | | 1453 | | | 677 | | | 1822 | | | 478 | |
| Link Distance (ft) Travel Time (s) | | 28.3 | | | 13.2 | | | 62.1 | | | 9.3 | |
| Peak Hour Factor | 0.90 | 20.3 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 02.1 | 0.90 | 0.90 | 9.3 0.90 | 0.90 |
| | 0.90 | 0.90 514 | 0.90 97 | 258 | 393 | 21 | 151 | 278 | 191 | 28 | 240 | 0.90 37 |
| Adj. Flow (vph) | JI | 514 | 97 | 200 | 393 | 21 | 151 | 210 | 191 | 20 | 240 | 37 |
| Shared Lane Traffic (%) | 51 | 514 | 97 | 258 | 414 | 0 | 151 | 469 | 0 | 28 | 277 | 0 |
| Lane Group Flow (vph) Enter Blocked Intersection | No | No | 97 No | Z50 No | 414 No | 0 No | No | 409 No | No | Zo No | No | 0 No |
| | Left | Left | | Left | Left | | Left | Left | | Left | Left | |
| Lane Alignment | Leit | 12 | Right | Leit | 12 | Right | Leit | 12 | Right | Leit | 12 | Right |
| Median Width(ft) Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| | | 16 | | | 16 | | | 16 | | | 16 | |
| Crosswalk Width(ft) Two way Left Turn Lane | | Yes | | | Yes | | | 10 | | | Yes | |
| | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Headway Factor | 1.01 | 1.01 | 9 | 15 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | Perm | NA | Perm | Prot | NA | 9 | Perm | NA | 9 | Perm | NA | 9 |
| Turn Type Protected Phases | Feilii | 2 | Feilii | 1 | 6 | | Feilii | NA 8 | | Feilli | 4 | |
| Permitted Phases | 2 | 2 | 2 | I | 0 | | 0 | 0 | | 4 | 4 | |
| Detector Phase | 2 | 2 | 2 | 1 | 6 | | 8 8 | 8 | | 4 | 4 | |
| Switch Phase | 2 | 2 | 2 | I | 0 | | 0 | 0 | | 4 | 4 | |
| | 10.0 | 10.0 | 10.0 | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Initial (s) | 24.0 | 24.0 | 24.0 | 14.0 | 10.0 | | | 24.0 | | 14.0 | 14.0 | |
| Minimum Split (s) | 24.0 35.0 | | | 21.0 | 56.0 | | 24.0 34.0 | 24.0 34.0 | | | 34.0 | |
| Total Split (s) | | 35.0 | 35.0 | | | | 37.8% | | | 34.0 | | |
| Total Split (%) | 38.9% | 38.9% | 38.9% | 23.3% | 62.2% | | | 37.8% | | 37.8% | 37.8% | |
| Maximum Green (s) | 28.0 | 28.0 | 28.0 | 14.0 | 49.0 | | 27.0 | 27.0 | | 27.0 | 27.0 | |
| Yellow Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | | -2.0 | -2.0 | | -2.0 | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| Lead/Lag | Lag | Lag | Lag | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 2.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | Min | None | Min | | None | None | | None | None | |
| Walk Time (s) | 7.0 | 7.0 | 7.0 | | | | 7.0 | 7.0 | | | | |
| Flash Dont Walk (s) | 10.0 | 10.0 | 10.0 | | | | 10.0 | 10.0 | | | | |
| Pedestrian Calls (#/hr) | 0 | 0 | 0 | 45.0 | 47.0 | | 0 | 0 | | 00.0 | 00.0 | |
| Act Effct Green (s) | 27.4 | 27.4 | 27.4 | 15.3 | 47.8 | | 26.3 | 26.3 | | 26.3 | 26.3 | |

2026 Build + IMP AM Peak Hour Timmons Group

Zebulon South TIA

| 2: NC-96 | (Arendell Avenue) |) & NC-97 (| (Gannon Avenue) |) |
|----------|-------------------|-------------|-----------------|---|
|----------|-------------------|-------------|-----------------|---|

12/05/2023

| | ٠ | → | 7 | 1 | + | • | 1 | Ť | 1 | 4 | ţ | ~ |
|-------------------------------|-------------|------------|----------|-----------|-------------|------------|-------------|------|-----|------|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBF |
| Actuated g/C Ratio | 0.33 | 0.33 | 0.33 | 0.18 | 0.57 | | 0.31 | 0.31 | | 0.31 | 0.31 | |
| v/c Ratio | 0.16 | 0.86 | 0.19 | 0.79 | 0.39 | | 0.55 | 0.85 | | 0.24 | 0.49 | |
| Control Delay | 22.9 | 43.1 | 22.5 | 53.8 | 11.8 | | 33.9 | 44.1 | | 28.7 | 27.6 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 22.9 | 43.1 | 22.5 | 53.8 | 11.8 | | 33.9 | 44.1 | | 28.7 | 27.6 | |
| LOS | С | D | С | D | В | | С | D | | С | С | |
| Approach Delay | | 38.5 | | | 27.9 | | | 41.6 | | | 27.7 | |
| Approach LOS | | D | | | С | | | D | | | С | |
| Queue Length 50th (ft) | 20 | 267 | 39 | 142 | 122 | | 70 | 244 | | 12 | 125 | |
| Queue Length 95th (ft) | 48 | #439 | 76 | #268 | 185 | | 134 | #407 | | 36 | 200 | |
| Internal Link Dist (ft) | | 1373 | | | 597 | | | 1742 | | | 398 | |
| Turn Bay Length (ft) | 200 | | 100 | 350 | | | 125 | | | 250 | | |
| Base Capacity (vph) | 346 | 666 | 566 | 344 | 1146 | | 306 | 617 | | 132 | 631 | |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Reduced v/c Ratio | 0.15 | 0.77 | 0.17 | 0.75 | 0.36 | | 0.49 | 0.76 | | 0.21 | 0.44 | |
| Intersection Summary | | | | | | | | | | | | |
| 31 | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 84. | 2 | | | | | | | | | | | |
| Natural Cycle: 70 | | | | | | | | | | | | |
| Control Type: Actuated-Uno | coordinate | t | | | | | | | | | | |
| Maximum v/c Ratio: 0.86 | | | | | | | | | | | | |
| Intersection Signal Delay: 3 | | | | | Itersection | | | | | | | |
| Intersection Capacity Utiliza | ation 83.4% | 0 | | IC | CU Level | of Service | θE | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | | | ueue ma | y be long | ger. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 2: NC | C-96 (Areno | lell Aveni | ue) & NC | -97 (Gan | non Aver | nue) | | | | | | |
| · . | | | , | 1 | | - / | | 4 | | | | |
| € Ø1 21 s | 35 s | Ø2 | | | | | ∳ Ø 34 s | 4 | | | | |
| 215 | 35 5 | | | | | | JT5 | | | | | |

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3: NC-96 (Arendell Avenue) & Barbee Street

| Intersection | | | | | | | | | | | | | |
|------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|--|
| Int Delay, s/veh | 2.2 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | | \$ | | | \$ | | | \$ | | | \$ | | |
| Traffic Vol, veh/h | 34 | 4 | 21 | 4 | 4 | 4 | 56 | 406 | 4 | 4 | 191 | 20 | |
| Future Vol, veh/h | 34 | 4 | 21 | 4 | 4 | 4 | 56 | 406 | 4 | 4 | 191 | 20 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None | |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - | |
| Veh in Median Storage | e, # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 38 | 4 | 23 | 4 | 4 | 4 | 62 | 451 | 4 | 4 | 212 | 22 | |
| | 00 | - | 20 | - | | | 02 | JU | | | 212 | 22 | |

| Major/Minor | Minor2 | | ſ | Minor1 | | l | Major1 | | ſ | Major2 | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|---|--------|---|---|--|
| Conflicting Flow All | 812 | 810 | 223 | 822 | 819 | 453 | 234 | 0 | 0 | 455 | 0 | 0 | |
| Stage 1 | 231 | 231 | - | 577 | 577 | - | - | - | - | - | - | - | |
| Stage 2 | 581 | 579 | - | 245 | 242 | - | - | - | - | - | - | - | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - | 2.218 | - | - | |
| Pot Cap-1 Maneuver | 298 | 314 | 817 | 293 | 310 | 607 | 1333 | - | - | 1106 | - | - | |
| Stage 1 | 772 | 713 | - | 502 | 502 | - | - | - | - | - | - | - | |
| Stage 2 | 499 | 501 | - | 759 | 705 | - | - | - | - | - | - | - | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | |
| Mov Cap-1 Maneuve | r 278 | 293 | 817 | 267 | 290 | 607 | 1333 | - | - | 1106 | - | - | |
| Mov Cap-2 Maneuve | 278 | 293 | - | 267 | 290 | - | - | - | - | - | - | - | |
| Stage 1 | 724 | 710 | - | 471 | 471 | - | - | - | - | - | - | - | |
| Stage 2 | 460 | 470 | - | 730 | 702 | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | |
| Approach | EB | | | WB | | | NB | | | SB | | | |

| Approach | EB | WB | NB | SB | |
|----------------------|----|------|-----|-----|--|
| HCM Control Delay, s | 17 | 16.1 | 0.9 | 0.2 | |
| HCM LOS | С | С | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR E | BLn1V | VBLn1 | SBL | SBT | SBR |
|-----------------------|-------|-----|-------|-------|-------|-------|-----|-----|
| Capacity (veh/h) | 1333 | - | - | 365 | 339 | 1106 | - | - |
| HCM Lane V/C Ratio | 0.047 | - | - | 0.18 | 0.039 | 0.004 | - | - |
| HCM Control Delay (s) | 7.8 | 0 | - | 17 | 16.1 | 8.3 | 0 | - |
| HCM Lane LOS | А | А | - | С | С | Α | А | - |
| HCM 95th %tile Q(veh) | 0.1 | - | - | 0.6 | 0.1 | 0 | - | - |

| Intersection | | | | | | |
|------------------------|--------|------|------|------|------|------|
| Int Delay, s/veh | 1.2 | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y | | | 4 | • | 1 |
| Traffic Vol, veh/h | 42 | 8 | 4 | 366 | 168 | 13 |
| Future Vol, veh/h | 42 | 8 | 4 | 366 | 168 | 13 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | 50 |
| Veh in Median Storage | e, # 0 | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 9 | 4 | 407 | 187 | 14 |
| | | | | | | |

| Major/Minor | Minor2 | N | Major1 | N | lajor2 | | |
|----------------------|--------|-------|--------|-------|--------|-----|--|
| Conflicting Flow All | 602 | 187 | 201 | 0 | - | 0 | |
| Stage 1 | 187 | - | - | - | - | - | |
| Stage 2 | 415 | - | - | - | - | - | |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | |
| Follow-up Hdwy | | 3.318 | 2.218 | - | - | - | |
| Pot Cap-1 Maneuver | | 855 | 1371 | - | - | - | |
| Stage 1 | 845 | - | - | - | - | - | |
| Stage 2 | 666 | - | - | - | - | - | |
| Platoon blocked, % | | | | - | - | - | |
| Mov Cap-1 Maneuve | | 855 | 1371 | - | - | - | |
| Mov Cap-2 Maneuve | | - | - | - | - | - | |
| Stage 1 | 842 | - | - | - | - | - | |
| Stage 2 | 666 | - | - | - | - | - | |
| | | | | | | | |
| Approach | EB | | NB | | SB | | |
| HCM Control Delay, | | | 0.1 | | 0 | | |
| HCM LOS | В | | | | | | |
| | | | | | | | |
| Minor Lane/Major Mv | /mt | NBL | NBTE | EBLn1 | SBT | SBR | |
| Capacity (veh/h) | | 1371 | - | 498 | - | - | |
| HCM Lane V/C Ratio |) | 0.003 | - | 0.112 | - | - | |

| | | ••••= | | | | |
|-----------------------|-----|--------|---|---|--|--|
| HCM Control Delay (s) | 7.6 | 0 13.1 | - | - | | |
| HCM Lane LOS | А | A B | - | - | | |
| HCM 95th %tile Q(veh) | 0 | - 0.4 | - | - | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |

| Intersection | | | | | | |
|------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 1.3 | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y | | | र्स | • | 1 |
| Traffic Vol, veh/h | 42 | 17 | 6 | 327 | 162 | 14 |
| Future Vol, veh/h | 42 | 17 | 6 | 327 | 162 | 14 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | 50 |
| Veh in Median Storage | ,#0 | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 19 | 7 | 363 | 180 | 16 |
| | | | | | | |

| Major/Minor | Minor2 | | Major1 | Ν | /lajor2 | | |
|----------------------|--------|-------|--------|-------|---------|-----|--|
| Conflicting Flow All | 557 | 180 | 196 | 0 | - | 0 | |
| Stage 1 | 180 | - | - | - | - | - | |
| Stage 2 | 377 | - | - | - | - | - | |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | |
| Follow-up Hdwy | | 3.318 | | - | - | - | |
| Pot Cap-1 Maneuver | 491 | 863 | 1377 | - | - | - | |
| Stage 1 | 851 | - | - | - | - | - | |
| Stage 2 | 694 | - | - | - | - | - | |
| Platoon blocked, % | | | | - | - | - | |
| Mov Cap-1 Maneuver | | 863 | 1377 | - | - | - | |
| Mov Cap-2 Maneuver | | - | - | - | - | - | |
| Stage 1 | 846 | - | - | - | - | - | |
| Stage 2 | 694 | - | - | - | - | - | |
| | | | | | | | |
| Approach | EB | | NB | | SB | | |
| HCM Control Delay, s | 5 12.3 | | 0.1 | | 0 | | |
| HCM LOS | В | | | | | | |
| | | | | | | | |
| Minor Lane/Major Mv | mt | NBL | NBTE | EBLn1 | SBT | SBR | |
| Capacity (veh/h) | | 1377 | _ | 558 | - | _ | |

| Capacity (veh/h) | 1377 | - 558 | - | - | |
|-----------------------|-------|---------|---|---|--|
| HCM Lane V/C Ratio | 0.005 | - 0.117 | - | - | |
| HCM Control Delay (s) | 7.6 | 0 12.3 | - | - | |
| HCM Lane LOS | А | A B | - | - | |
| HCM 95th %tile Q(veh) | 0 | - 0.4 | - | - | |

Zebulon South TIA 6: NC-96 (Arendell Avenue) & Perry Curtis Road

| Intersection | | | | | | |
|------------------------|--------------|------|------|------|------|------|
| Int Delay, s/veh | 3.1 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | f, | | | ŧ |
| Traffic Vol, veh/h | 21 | 96 | 237 | 6 | 43 | 136 |
| Future Vol, veh/h | 21 | 96 | 237 | 6 | 43 | 136 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Vah in Madian Starage | . # ∩ | | 0 | | | 0 |

| Storage Length | 0 | - | - | - | - | - | |
|-----------------------|------|-----|-----|----|----|-----|--|
| Veh in Median Storage | e,#0 | - | 0 | - | - | 0 | |
| Grade, % | 0 | - | 0 | - | - | 0 | |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 23 | 107 | 263 | 7 | 48 | 151 | |

| Major/Minor | Minor1 | Ν | /lajor1 | Ν | /lajor2 | |
|----------------------|--------|-------|---------|------|---------|-----|
| Conflicting Flow All | 514 | 267 | 0 | 0 | 270 | 0 |
| Stage 1 | 267 | - | - | - | - | - |
| Stage 2 | 247 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | | 772 | - | - | 1293 | - |
| Stage 1 | 778 | - | - | - | - | - |
| Stage 2 | 794 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuve | | 772 | - | - | 1293 | - |
| Mov Cap-2 Maneuve | | - | - | - | - | - |
| Stage 1 | 778 | - | - | - | - | - |
| Stage 2 | 761 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | s 11.3 | | 0 | | 1.9 | |
| HCM LOS | В | | | | | |
| | | | | | | |
| Minor Lono/Major Mu | mt | NBT | NBRWI | Din1 | SBL | SBT |
| Minor Lane/Major Mv | mt | IND I | INDRVV | | | |
| Capacity (veh/h) | | - | - , | 703 | 1293 | - |
| HCM Lane V/C Ratio | -) | - | - (| | 0.037 | - |
| HCM Control Delay (| S) | - | - | 11.3 | 7.9 | 0 |
| HCM Lane LOS | | - | - | В | Α | Α |

0.7

-

-

0.1

-

HCM 95th %tile Q(veh)

Grade, %

Peak Hour Factor

0

90

0

90

-

90

-

90

| Intersection | | | | | | |
|------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 1.9 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | 1. | | ٦ | • |
| Traffic Vol, veh/h | 8 | 52 | 214 | 4 | 16 | 92 |
| Future Vol, veh/h | 8 | 52 | 214 | 4 | 16 | 92 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 50 | - |
| Veh in Median Storage | e,#0 | - | 0 | - | - | 0 |

-

90

0

90

| Feak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
|----------------------|--------|-------|---------|------|--------|-----|
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 58 | 238 | 4 | 18 | 102 |
| | | | | | | |
| Major/Minor | Minor1 | Ν | /lajor1 | Ν | Major2 | |
| | | | | | | 0 |
| Conflicting Flow All | 378 | 240 | 0 | 0 | 242 | 0 |
| Stage 1 | 240 | - | - | - | - | - |
| Stage 2 | 138 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 624 | 799 | - | - | 1324 | - |
| Stage 1 | 800 | - | - | - | - | - |
| Stage 2 | 889 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | 615 | 799 | - | - | 1324 | - |
| Mov Cap-2 Maneuver | | - | - | - | - | - |
| Stage 1 | 800 | - | - | - | - | - |
| Stage 2 | 877 | - | - | - | - | - |
| | | | | | | |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | 5 10.1 | | 0 | | 1.1 | |
| HCM LOS | В | | | | | |
| | | | | | | |
| Miner Leve/Meier Mu | | NDT | | DI | | ОРТ |
| Minor Lane/Major Mv | mt | NBT | NBRW | REUI | SBL | SBT |

| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBL | SBT | |
|-----------------------|-----|----------|-------|-----|--|
| Capacity (veh/h) | - | - 768 | 1324 | - | |
| HCM Lane V/C Ratio | - | - 0.087 | 0.013 | - | |
| HCM Control Delay (s) | - | - 10.1 | 7.8 | - | |
| HCM Lane LOS | - | - B | А | - | |
| HCM 95th %tile Q(veh) | - | - 0.3 | 0 | - | |

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/05/2023

| | ٠ | + | 1 | 4 | Ļ | * | 1 | 1 | 1 | 4 | ţ | ~ |
|----------------------------|------------|------------|-------|-------|------------|-------|-------|-------------|--------|-------------|-------------|--------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | f, | | 7 | ef 👔 | | | \$ | | | \$ | |
| Traffic Volume (vph) | 10 | 724 | 150 | 73 | 592 | 4 | 113 | 10 | 79 | 7 | 19 | 29 |
| Future Volume (vph) | 10 | 724 | 150 | 73 | 592 | 4 | 113 | 10 | 79 | 7 | 19 | 29 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.974 | | | 0.999 | | | 0.947 | | | 0.929 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.973 | | | 0.993 | |
| Satd. Flow (prot) | 1778 | 1823 | 0 | 1770 | 1861 | 0 | 0 | 1691 | 0 | 0 | 1710 | 0 |
| Flt Permitted | 0.413 | | | 0.950 | | | | 0.812 | | | 0.953 | |
| Satd. Flow (perm) | 773 | 1823 | 0 | 1770 | 1861 | 0 | 0 | 1411 | 0 | 0 | 1641 | 0 |
| Right Turn on Red | | | No | | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 11 | 804 | 167 | 81 | 658 | 4 | 126 | 11 | 88 | 8 | 21 | 32 |
| Shared Lane Traffic (%) | | | | • | | · | | | | | | |
| Lane Group Flow (vph) | 11 | 971 | 0 | 81 | 662 | 0 | 0 | 225 | 0 | 0 | 61 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | Lon | 12 | rugin | Lon | 12 | rugin | 2011 | 0 | rugitt | Lon | 0 | rugint |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | Yes | | | 10 | | | 10 | |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 | 0.00 | 9 | 15 | | 9 | 15 | | 9 | 15 | | 9 |
| Turn Type | Perm | NA | Ŭ | Prot | NA | Ŭ | Perm | NA | Ŭ | Perm | NA | Ű |
| Protected Phases | | 2 | | 1 | 6 | | | 8 | | | 4 | |
| Permitted Phases | 2 | | | | | | 8 | Ū | | 4 | | |
| Detector Phase | 2 | 2 | | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | _ | - | | | Ŭ | | Ŭ | Ŭ | | | | |
| Minimum Initial (s) | 10.0 | 10.0 | | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 17.0 | 17.0 | | 14.0 | 17.0 | | 14.0 | 14.0 | | 14.0 | 14.0 | |
| Total Split (s) | 52.0 | 52.0 | | 14.0 | 66.0 | | 24.0 | 24.0 | | 24.0 | 24.0 | |
| Total Split (%) | 57.8% | 57.8% | | 15.6% | 73.3% | | 26.7% | 26.7% | | 26.7% | 26.7% | |
| Maximum Green (s) | 45.0 | 45.0 | | 7.0 | 59.0 | | 17.0 | 17.0 | | 17.0 | 17.0 | |
| Yellow Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | | -2.0 | -2.0 | | 2.0 | -2.0 | | 2.0 | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | |
| Lead/Lag | Lag | Lag | | Lead | 5.0 | | | 5.0 | | | 5.0 | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | S.U Min | 3.0 Min | | None | 3.0 Min | | None | Z.0 None | | Z.0 None | Z.0 None | |
| | 48.3 | 48.3 | | 9.1 | 59.1 | | NOTE | 17.5 | | NOLIG | 17.5 | |
| Act Effct Green (s) | | | | | | | | 0.20 | | | | |
| Actuated g/C Ratio | 0.56 | 0.56 | | 0.11 | 0.68 | | | | | | 0.20 | |
| v/c Ratio | 0.03 | 0.96 | | 0.44 | 0.52 | | | 0.79 | | | 0.18 | |
| Control Delay | 10.8 | 41.4 | | 46.3 | 8.8 | | | 54.8 | | | 30.9 | |

2026 Build + IMP PM Peak Hour Timmons Group

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/05/2023

| | ۶ | → | 7 | 4 | + | * | 1 | t | 1 | 4 | ŧ | 4 |
|--|-------------|------------|-------------|-----------|------------|------------|-----|------|-----|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 10.8 | 41.4 | | 46.3 | 8.8 | | | 54.8 | | | 30.9 | |
| LOS | В | D | | D | А | | | D | | | С | |
| Approach Delay | | 41.0 | | | 12.9 | | | 54.8 | | | 30.9 | |
| Approach LOS | | D | | | В | | | D | | | С | |
| Queue Length 50th (ft) | 3 | ~557 | | 44 | 164 | | | 121 | | | 29 | |
| Queue Length 95th (ft) | 11 | #825 | | 90 | 244 | | | #232 | | | 63 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 430 | 1016 | | 185 | 1318 | | | 311 | | | 362 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.03 | 0.96 | | 0.44 | 0.50 | | | 0.72 | | | 0.17 | |
| Intersection Summary | | | | | | | | | | | | |
| 71 | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 86. | .6 | | | | | | | | | | | |
| Natural Cycle: 90 | | | | | | | | | | | | |
| Control Type: Actuated-Une | coordinated | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.96 | | | | | | | | | | | | |
| Intersection Signal Delay: 3 | | | | | tersection | | | | | | | |
| Intersection Capacity Utiliza | ation 83.8% | 6 | | IC | U Level | of Service | еE | | | | | |
| Analysis Period (min) 15 | • | | | | | | | | | | | |
| Volume exceeds capac | | | ically infi | nite. | | | | | | | | |
| Queue shown is maximi | | | | | | | | | | | | |
| # 95th percentile volume | | | ueue ma | y be long | jer. | | | | | | | |
| Queue shown is maxim | um atter tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 1: S \ | Wakefield S | Street & N | IC-97 (G | annon Av | venue) | | | | | | | |

Splits and Phases: 1: S Wakefield Street & NC-97 (Gannon Avenue)

| √ Ø1 | | Ø4 |
|----------------|------|------|
| 14 s | 52 s | 24 s |
| ← Ø6 | | Ø8 |
| 66 s | | 24 s |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/05/2023

| Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL Lane Configurations 1 | SBT SBR 282 50 282 50 1900 1900 2% 0 1.00 0 1.00 1.00 0.977 1802 1802 0 |
|---|--|
| Lane Configurations Image: Configuration of the control | 1 282 50 282 50 1900 1900 1900 1900 2% 0 0 0 0 0 1.00 1.00 0.977 1802 0 1802 0 0 0 0 |
| Traffic Volume (vph) 66 363 127 282 512 57 113 357 176 51 Future Volume (vph) 66 363 127 282 512 57 113 357 176 51 Ideal Flow (vphpl) 1900 <td>282 50 282 50 1900 1900 2% 0 1.00 1.00 0.977 1802 0</td> | 282 50 282 50 1900 1900 2% 0 1.00 1.00 0.977 1802 0 |
| Future Volume (vph) 66 363 127 282 512 57 113 357 176 51 Ideal Flow (vphpl) 1900 100 100 | 282 50 1900 1900 2% 0 1.00 1.00 0.977 1802 0 |
| Ideal Flow (vphpl) 1900 <td>1900 1900 2% 0 0 1.00 1.00 0.977 1802 0 1802 0</td> | 1900 1900 2% 0 0 1.00 1.00 0.977 1802 0 1802 0 |
| Grade (%) 2% -2% -2% Storage Length (ft) 200 100 350 0 125 0 250 Storage Lanes 1 1 1 0 1 0 1 Taper Length (ft) 25 25 25 25 25 25 Lane Util. Factor 1.00 | 2% 0 0 1.00 0.977 1802 0 1802 0 |
| Storage Length (ft) 200 100 350 0 125 0 250 Storage Lanes 1 1 1 0 1 0 1 Taper Length (ft) 25 25 25 25 25 25 Lane Util. Factor 1.00 1. | 0 0 1.00 1.00 0.977 1802 0 1802 0 |
| Storage Lanes 1 1 1 0 1 0 1 Taper Length (ft) 25 <td>0 1.00 1.00 0.977 1802 0 1802 0</td> | 0 1.00 1.00 0.977 1802 0 1802 0 |
| Taper Length (ft) 25 25 25 25 Lane Util. Factor 1.00 0.950< | 1.00 1.00 0.977 1802 0 1802 0 |
| Lane Util. Factor 1.00 <td>0.977 1802 0 1802 0</td> | 0.977 1802 0 1802 0 |
| Frt 0.850 0.985 0.950 Fit Protected 0.950 0.950 0.950 0.950 Satd. Flow (prot) 1752 1844 1567 1787 1853 0 1787 1787 0 1752 Flt Permitted 0.425 0.950 0.392 0.146 Satd. Flow (perm) 784 1844 1567 1787 1853 0 737 1787 0 269 Right Turn on Red No No No No No No | 0.977 1802 0 1802 0 |
| Fit Protected 0.950 0.950 0.950 0.950 Satd. Flow (prot) 1752 1844 1567 1787 1853 0 1787 1787 0 1752 Fit Permitted 0.425 0.950 0.392 0.146 Satd. Flow (perm) 784 1844 1567 1787 1853 0 737 1787 0 269 Right Turn on Red No No No No No No | 1802 0 1802 0 |
| Satd. Flow (prot) 1752 1844 1567 1787 1853 0 1787 1787 0 1752 Flt Permitted 0.425 0.950 0.392 0.146 Satd. Flow (perm) 784 1844 1567 1787 1853 0 737 1787 0 269 Right Turn on Red No No No No No No | 1802 0 |
| Flt Permitted 0.425 0.950 0.392 0.146 Satd. Flow (perm) 784 1844 1567 1787 1853 0 737 1787 0 269 Right Turn on Red No No No No No | 1802 0 |
| Satd. Flow (perm) 784 1844 1567 1787 1853 0 737 1787 0 269 Right Turn on Red No No No No | |
| Right Turn on Red No No No | |
| • | No |
| | |
| Link Speed (mph) 35 35 20 | 35 |
| Link Distance (ft) 1453 677 1822 | 478 |
| Travel Time (s) 28.3 13.2 62.1 | 9.3 |
| Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.9 | 0.90 0.90 |
| Adj. Flow (vph) 73 403 141 313 569 63 126 397 196 57 | 313 56 |
| Shared Lane Traffic (%) | |
| Lane Group Flow (vph) 73 403 141 313 632 0 126 593 0 57 | 369 0 |
| Enter Blocked Intersection No | No No |
| Lane Alignment Left Left Right Left Right Left Right Left Right Left | Left Right |
| Median Width(ft) 12 12 12 | 12 |
| Link Offset(ft) 0 0 0 | 0 |
| Crosswalk Width(ft) 16 16 16 | 16 |
| Two way Left Turn Lane Yes Yes | Yes |
| Headway Factor 1.01 1.01 1.01 0.99 0.99 0.99 0.99 0.99 | 1.01 1.01 |
| Turning Speed (mph) 15 9 15 9 15 9 15 | 9 |
| Turn Type Perm NA Perm Prot NA Perm NA Perm | NA |
| Protected Phases 2 1 6 8 | 4 |
| Permitted Phases 2 2 8 4 | |
| Detector Phase 2 2 2 1 6 8 8 4 | 4 |
| Switch Phase | |
| Minimum Initial (s) 10.0 10.0 10.0 7.0 10.0 7.0 7.0 7.0 7.0 | 7.0 |
| Minimum Split (s) 24.0 24.0 24.0 14.0 17.0 24.0 24.0 14.0 | 14.0 |
| Total Split (s) 27.0 27.0 27.0 23.0 50.0 40.0 40.0 40.0 | 40.0 |
| Total Split (%) 30.0% 30.0% 30.0% 25.6% 55.6% 44.4% 44.4% 44.4% | 14.4% |
| Maximum Green (s) 20.0 20.0 20.0 16.0 43.0 33.0 33.0 33.0 | 33.0 |
| Yellow Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 | 5.0 |
| All-Red Time (s) 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 | 2.0 |
| Lost Time Adjust (s) -2.0 -2.0 -2.0 -2.0 -2.0 -2.0 -2.0 -2.0 | -2.0 |
| Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 | 5.0 |
| Lead/Lag Lag Lag Lead | |
| Lead-Lag Optimize? Yes Yes Yes Yes | |
| Vehicle Extension (s) 3.0 3.0 3.0 2.0 3.0 2.0 | 2.0 |
| Recall Mode Min Min Min None Min None None None None | None |
| Walk Time (s) 7.0 7.0 7.0 7.0 7.0 | |
| Flash Dont Walk (s) 10.0 10.0 10.0 10.0 | |
| Pedestrian Calls (#/hr) 0 0 0 0 0 0 | |
| Act Effct Green (s) 21.3 21.3 21.3 17.5 43.9 32.1 32.1 | 32.1 |

2026 Build + IMP PM Peak Hour Timmons Group

Zebulon South TIA

| 2: NC-96 (A | Arendell Avenue |) & NC-97 (| (Gannon Avenue) |) |
|-------------|-----------------|-------------|-----------------|---|
|-------------|-----------------|-------------|-----------------|---|

12/05/2023

| | ٠ | → | 7 | 1 | + | * | 1 | t | 1 | 1 | ţ | 4 |
|-------------------------------|------------|------------|----------|-----------|------------|-------------|------|------|-----|------|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBF |
| Actuated g/C Ratio | 0.25 | 0.25 | 0.25 | 0.20 | 0.51 | | 0.37 | 0.37 | | 0.37 | 0.37 | |
| v/c Ratio | 0.38 | 0.88 | 0.36 | 0.86 | 0.67 | | 0.46 | 0.89 | | 0.57 | 0.55 | |
| Control Delay | 35.0 | 54.9 | 31.1 | 58.3 | 20.7 | | 27.1 | 43.0 | | 47.6 | 25.0 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 35.0 | 54.9 | 31.1 | 58.3 | 20.7 | | 27.1 | 43.0 | | 47.6 | 25.0 | |
| LOS | D | D | С | Е | С | | С | D | | D | С | |
| Approach Delay | | 47.1 | | | 33.2 | | | 40.2 | | | 28.0 | |
| Approach LOS | | D | | | С | | | D | | | С | |
| Queue Length 50th (ft) | 35 | 222 | 67 | 174 | 260 | | 52 | 303 | | 25 | 158 | |
| Queue Length 95th (ft) | 77 | #389 | 121 | #321 | 385 | | 106 | #494 | | #83 | 243 | |
| Internal Link Dist (ft) | | 1373 | | | 597 | | | 1742 | | | 398 | |
| Turn Bay Length (ft) | 200 | | 100 | 350 | | | 125 | | | 250 | | |
| Base Capacity (vph) | 201 | 474 | 403 | 376 | 975 | | 301 | 731 | | 109 | 737 | |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Reduced v/c Ratio | 0.36 | 0.85 | 0.35 | 0.83 | 0.65 | | 0.42 | 0.81 | | 0.52 | 0.50 | |
| Intersection Summary | | | | | | | | | | | | |
| | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 86.1 | 1 | | | | | | | | | | | |
| Natural Cycle: 80 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | oordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.89 | | | | | | | | | | | | |
| Intersection Signal Delay: 3 | 7.4 | | | In | tersection | n LOS: D | | | | | | |
| Intersection Capacity Utiliza | tion 90.8% | 0 | | IC | U Level | of Service | eΕ | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume e | | | ueue ma | y be long | jer. | | | | | | | |
| Queue shown is maximu | m after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 2: NC- | -96 (Areno | dell Aveni | ie) & NC | -97 (Gan | non Aver | nue) | | | | | | |
| 1 | | | | | | | | | | | | |
| ♥ Ø1 | - | Ø2 | | | | + Ø 40 s | 4 | | | | | |

-¶ø8

Ø6

| 4.5 | | | | | | | | | | | | |
|--------|--|--|---|---|---|---|---|--|---|---|---|--|
| EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| | 4 | | | 4 | | | 4 | | | 4 | | |
| 51 | 5 | 82 | 4 | 4 | 7 | 41 | 317 | 4 | 9 | 521 | 47 | |
| 51 | 5 | 82 | 4 | 4 | 7 | 41 | 317 | 4 | 9 | 521 | 47 | |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | |
| - | - | None | - | - | None | - | - | None | - | - | None | |
| - | - | - | - | - | - | - | - | - | - | - | - | |
| e, # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | |
| 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| 57 | 6 | 91 | 4 | 4 | 8 | 46 | 352 | 4 | 10 | 579 | 52 | |
| | EBL 51 51 Stop - - - - 90 2 | EBL EBT 51 5 51 5 0 0 Stop Stop - - - - - 0 - 0 - 0 9, # - 0 90 90 2 2 | EBL EBT EBR 51 5 82 51 5 82 0 0 0 Stop Stop Stop Stop Stop Stop - - None - - - - 0 - - 0 - - 0 - - 0 - 90 90 90 2 2 2 | EBL EBT EBR WBL 51 5 82 4 51 5 82 4 51 5 82 4 0 0 0 0 Stop Stop Stop Stop 5 7 0 0 0 5 7 None - 5 7 0 - - 5 0 0 0 0 - 5 7 0 - - - - 5 90 90 90 90 90 90 90 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 | EBL EBT EBR WBL WBT Image: State of the state o | EBL EBR WBL WBT WBR Image: State of the state o | EBL EBR WBL WBT WBR NBL \clubsuit \clubsuit \clubsuit \clubsuit \clubsuit \clubsuit 51 55 82 4 4 7 41 51 5 82 4 4 7 41 51 5 82 4 4 7 41 0 0 0 0 0 0 0 Stop Stop Stop Stop Stop Free \cdot None $ \cdot$ \cdot | EBL EBR WBL WBT WBR NBL NBT $ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \$ | EBLEBTEBRWBLWBTWBRNBLNBTNBR \clubsuit 5155824474131745158244741317400000000000StopStopStopStopStopFreeFreeFree-None-None-None-None-None-0None $\phi, \#$ 0-0-00- $\phi, \#$ 0090909090909090909022222222222 | EBLEBTEBRWBLWBTWBRNBLNBTNBRSBL \clubsuit \clubsuit \clubsuit \clubsuit \clubsuit \clubsuit \clubsuit \bullet \bullet \bullet \bullet 51558244741317495158244741317490000000000StopStopStopStopStopFreeFreeFree-None-None-None-None-NoneNoneNone-NoneNoneNone-NoneNone ϕ ,# -00 ϕ ,# -90909090909090902222222222 | EBL EBR WBL WBT WBR NBL NBT NBR SBL SBT | EBLEBRWBLWBTWBRNBLNBTNBRSBLSBTSBR \bullet 5158244741317495214751582447413174952147000000000000StopStopStopStopStopFreeFreeFreeFreeFreeFree-None-NoneNone-None-None-NoneNoneNoneNone-NoneNoneNoneNone-NoneNoneNoneNone-NoneNoneNoneNoneNoneNone <td< td=""></td<> |

| Major/Minor | Minor2 | | 1 | Minor1 | | | Major1 | | Ν | /lajor2 | | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|---|---------|---|---|--|--|
| | | 1072 | | | 1007 | | | 0 | | | 0 | 0 | | |
| Conflicting Flow All | 1077 | 1073 | 605 | 1120 | 1097 | 354 | 631 | 0 | 0 | 356 | 0 | 0 | | |
| Stage 1 | 625 | 625 | - | 446 | 446 | - | - | - | - | - | - | - | | |
| Stage 2 | 452 | 448 | - | 674 | 651 | - | - | - | - | - | - | - | | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - | 2.218 | - | - | | |
| Pot Cap-1 Maneuver | 197 | 220 | 498 | 184 | 213 | 690 | 951 | - | - | 1203 | - | - | | |
| Stage 1 | 473 | 477 | - | 591 | 574 | - | - | - | - | - | - | - | | |
| Stage 2 | 587 | 573 | - | 444 | 465 | - | - | - | - | - | - | - | | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | | |
| Mov Cap-1 Maneuver | r 181 | 204 | 498 | 139 | 198 | 690 | 951 | - | - | 1203 | - | - | | |
| Mov Cap-2 Maneuver | r 181 | 204 | - | 139 | 198 | - | - | - | - | - | - | - | | |
| Stage 1 | 445 | 471 | - | 556 | 540 | - | - | - | - | - | - | - | | |
| Stage 2 | 541 | 539 | - | 354 | 459 | - | - | - | - | - | - | - | | |
| | | | | | | | | | | | | | | |
| Approach | EB | | | WB | | | NB | | | SB | | | | |
| | | | | | | | | | | | | | | |

| HCM Control Delay, s | 30 | 20.2 | 1 | 0.1 | |
|----------------------|----|------|---|-----|--|
| HCM LOS | D | С | | | |
| | | | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1V | VBLn1 | SBL | SBT | SBR | |
|-----------------------|-------|-----|-----|--------|-------|-------|-----|-----|--|
| Capacity (veh/h) | 951 | - | - | 293 | 254 | 1203 | - | - | |
| HCM Lane V/C Ratio | 0.048 | - | - | 0.523 | 0.066 | 0.008 | - | - | |
| HCM Control Delay (s) | 9 | 0 | - | 30 | 20.2 | 8 | 0 | - | |
| HCM Lane LOS | Α | А | - | D | С | Α | A | - | |
| HCM 95th %tile Q(veh) | 0.2 | - | - | 2.8 | 0.2 | 0 | - | - | |

| Intersection | | | | | | |
|------------------------|----------|-------|--------|-------|---------|------|
| Int Delay, s/veh | 0.7 | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| | | EDK | INDL | | | |
| Lane Configurations | Y | F | 0 | 4 | 101 | |
| Traffic Vol, veh/h | 27 | 5 | 9 | 258 | 461 | 46 |
| Future Vol, veh/h | 27 | 5 | 9 | 258 | 461 | 46 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | _ 0 | _ 0 | _ 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | 50 |
| Veh in Median Storage | e,#0 | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 30 | 6 | 10 | 287 | 512 | 51 |
| | | • | | | • | ••• |
| | | | | | | |
| | Vinor2 | | Major1 | Ν | /lajor2 | |
| Conflicting Flow All | 819 | 512 | 563 | 0 | - | 0 |
| Stage 1 | 512 | - | - | - | - | - |
| Stage 2 | 307 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 345 | | 1008 | - | - | - |
| Stage 1 | 602 | - | - | - | - | - |
| Stage 2 | 746 | _ | _ | _ | _ | - |
| Platoon blocked, % | 740 | _ | - | _ | - | - |
| | 244 | FGO | 1000 | | | |
| Mov Cap-1 Maneuver | | 562 | 1008 | - | - | - |
| Mov Cap-2 Maneuver | | - | - | - | - | - |
| Stage 1 | 595 | - | - | - | - | - |
| Stage 2 | 746 | - | - | - | - | - |
| | | | | | | |
| Approach | EB | | NB | | SB | |
| HCM Control Delay, s | | | 0.3 | | 0 | |
| HCM LOS | C | | 0.3 | | 0 | |
| | U | | | | | |
| | | | | | | |
| Minor Lane/Major Mvr | nt | NBL | NBTI | EBLn1 | SBT | SBR |
| Capacity (veh/h) | | 1008 | - | 363 | - | _ |
| HCM Lane V/C Ratio | | 0.01 | - | 0.098 | - | - |
| HCM Control Delay (s |) | 8.6 | 0 | 16 | - | - |
| HCM Lane LOS | / | A | A | C | - | - |
| | , | 7 | л | 0 | - | - |

HCM 95th %tile Q(veh)

0

0.3

-

_

-

| Intersection | | | | | | | |
|------------------------|--------|------|------|------|----------|------|----|
| Int Delay, s/veh | 0.9 | | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR | 2 |
| Lane Configurations | Y | | | र्स | † | 1 | ٢. |
| Traffic Vol, veh/h | 27 | 11 | 18 | 240 | 419 | 47 | 7 |
| Future Vol, veh/h | 27 | 11 | 18 | 240 | 419 | 47 | 7 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |) |
| Sign Control | Stop | Stop | Free | Free | Free | Free | Э |
| RT Channelized | - | None | - | None | - | None | Э |
| Storage Length | 0 | - | - | - | - | 50 |) |
| Veh in Median Storage | e, # 0 | - | - | 0 | 0 | - | - |
| Grade, % | 0 | - | - | 0 | 0 | - | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |) |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 30 | 12 | 20 | 267 | 466 | 52 | 2 |

| Major/Minor | Minor2 | | Major1 | Ν | lajor2 | | | | | |
|-----------------------|--------|-------|--------|-------|--------|-----|--|--|--|--|
| Conflicting Flow All | 773 | 466 | 518 | 0 | - | 0 | | | | |
| Stage 1 | 466 | - | - | - | - | - | | | | |
| Stage 2 | 307 | - | - | - | - | - | | | | |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - | | | | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | | | | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | | | | |
| Follow-up Hdwy | | 3.318 | | - | - | - | | | | |
| Pot Cap-1 Maneuver | | 597 | 1048 | - | - | - | | | | |
| Stage 1 | 632 | - | - | - | - | - | | | | |
| Stage 2 | 746 | - | - | - | - | - | | | | |
| Platoon blocked, % | | | | - | - | - | | | | |
| Mov Cap-1 Maneuve | | 597 | 1048 | - | - | - | | | | |
| Mov Cap-2 Maneuve | | - | - | - | - | - | | | | |
| Stage 1 | 618 | - | - | - | - | - | | | | |
| Stage 2 | 746 | - | - | - | - | - | | | | |
| | | | | | | | | | | |
| Approach | EB | | NB | | SB | | | | | |
| HCM Control Delay, s | s 14.9 | | 0.6 | | 0 | | | | | |
| HCM LOS | В | | | | - | | | | | |
| | | | | | | | | | | |
| Minor Lane/Major Mv | rmt | NBL | NBTE | EBLn1 | SBT | SBR | | | | |
| Capacity (veh/h) | | 1048 | - | 406 | - | - | | | | |
| HCM Lane V/C Ratio | | 0.019 | - | 0.104 | - | - | | | | |
| LION Or start Dalay (| | 0 5 | | 44.0 | | | | | | |

| HCM Lane V/C Ratio | 0.019 | - 0.104 | - | - | |
|-----------------------|-------|---------|---|---|--|
| HCM Control Delay (s) | 8.5 | 0 14.9 | - | - | |
| HCM Lane LOS | А | A B | - | - | |
| HCM 95th %tile Q(veh) | 0.1 | - 0.3 | - | - | |
| | | | | | |

Zebulon South TIA 6: NC-96 (Arendell Avenue) & Perry Curtis Road

| 12/05/2023 |
|------------|
|------------|

| Intersection | | | | | | |
|------------------------|--------|------|------|------|------|------|
| Int Delay, s/veh | 2.9 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | Þ | | | र्स |
| Traffic Vol, veh/h | 11 | 72 | 186 | 15 | 138 | 292 |
| Future Vol, veh/h | 11 | 72 | 186 | 15 | 138 | 292 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 12 | 80 | 207 | 17 | 153 | 324 |
| | | | | | | |

| Major/Minor | Minor1 | Ν | /lajor1 | | Major2 | | |
|----------------------|--------|------|---------|-------|--------|-----|--|
| Conflicting Flow All | 846 | 216 | 0 | 0 | 224 | 0 | |
| Stage 1 | 216 | - | - | - | - | - | |
| Stage 2 | 630 | - | - | - | - | - | |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | | - | | 2.218 | - | |
| Pot Cap-1 Maneuve | | 824 | - | - | 1345 | - | |
| Stage 1 | 820 | - | - | - | - | - | |
| Stage 2 | 531 | - | - | - | - | - | |
| Platoon blocked, % | | | - | - | | - | |
| Mov Cap-1 Maneuve | | 824 | - | - | 1345 | - | |
| Mov Cap-2 Maneuve | | - | - | - | - | - | |
| Stage 1 | 820 | - | - | - | - | - | |
| Stage 2 | 457 | - | - | - | - | - | |
| | | | | | | | |
| Approach | WB | | NB | | SB | | |
| HCM Control Delay, | s 11.3 | | 0 | | 2.6 | | |
| HCM LOS | В | | | | | | |
| | | | | | | | |
| Minor Lane/Major M | vmt | NBT | NBRW | /BLn1 | SBL | SBT | |
| Capacity (veh/h) | | - | - | 660 | 1345 | - | |
| HCM Lane V/C Ratio | C | - | - | 0.14 | 0.114 | - | |
| HCM Control Delay | (s) | - | - | 11.3 | 8 | 0 | |

А

0.4

А

-

-

-

-

-

В

0.5

HCM Lane LOS

HCM 95th %tile Q(veh)

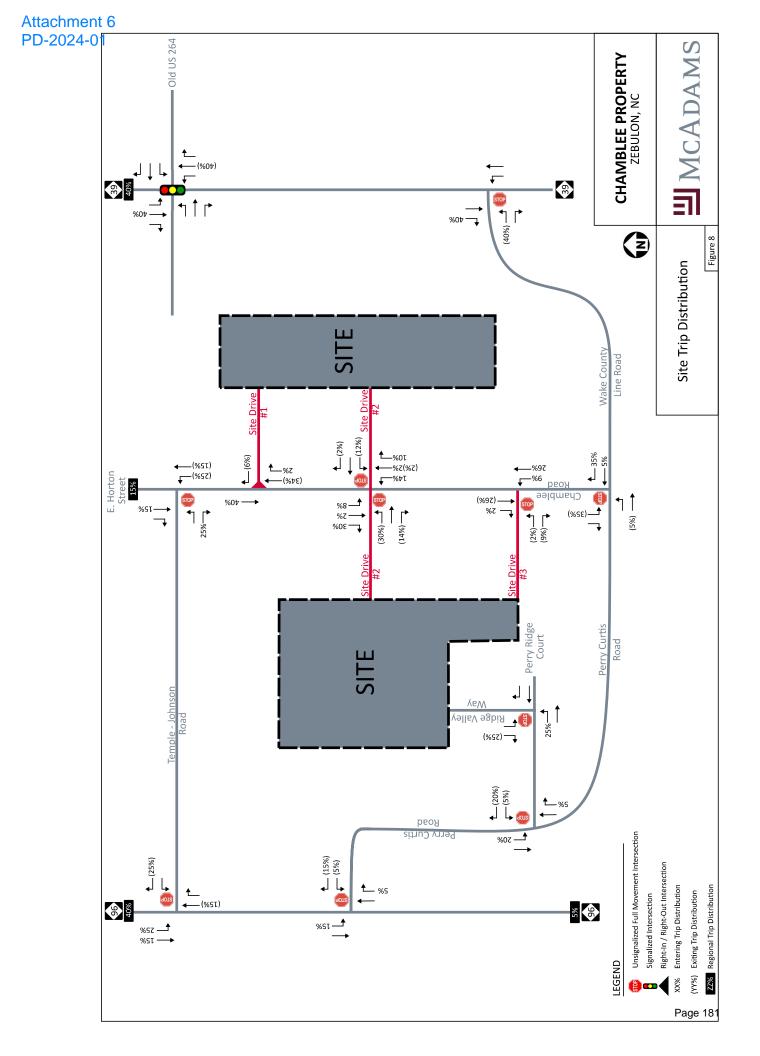
| rsection | | |
|--------------|-----|--|
| Delav. s/veh | 1.8 | |

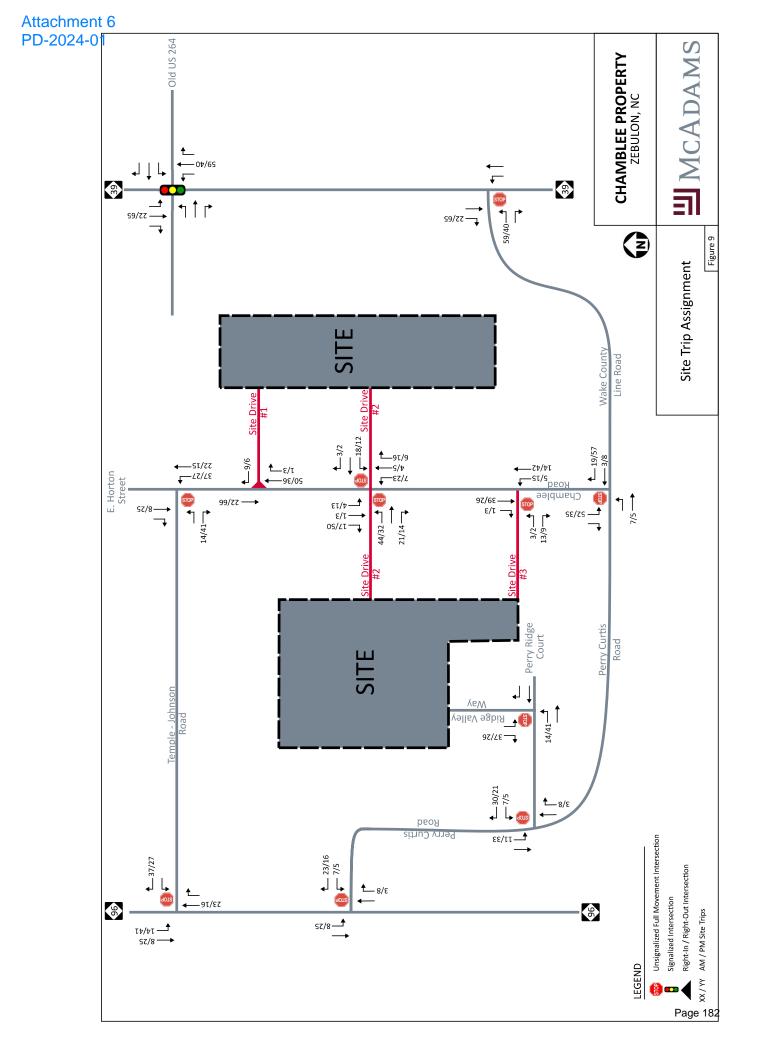
| Intersection | | | | | | |
|------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 1.8 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | 4 | | 5 | 1 |
| Traffic Vol, veh/h | 5 | 34 | 168 | 9 | 55 | 187 |
| Future Vol, veh/h | 5 | 34 | 168 | 9 | 55 | 187 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 50 | - |
| Veh in Median Storage | e,#0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 6 | 38 | 187 | 10 | 61 | 208 |

| Major/Minor | Minor1 | Ν | /lajor1 | Ν | /lajor2 | |
|----------------------|--------|-------|---------|------|---------|-----|
| Conflicting Flow All | 522 | 192 | 0 | 0 | 197 | 0 |
| Stage 1 | 192 | - | - | - | - | - |
| Stage 2 | 330 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 515 | 850 | - | - | 1376 | - |
| Stage 1 | 841 | - | - | - | - | - |
| Stage 2 | 728 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuve | er 492 | 850 | - | - | 1376 | - |
| Mov Cap-2 Maneuve | | - | - | - | - | - |
| Stage 1 | 841 | - | - | - | - | - |
| Stage 2 | 696 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, | s 9.9 | | 0 | | 1.8 | |
| HCM LOS | А | | | | | |
| | | | | | | |
| Minor Lane/Major M | /mt | NBT | NBRW | BLn1 | SBL | SBT |
| Capacity (veh/h) | | - | - | 777 | 1376 | - |
| HCM Lane V/C Ratio |) | - | - (| 056 | | - |

| HCM Lane V/C Ratio | - | - 0.056 0.044 | - |
|-----------------------|---|---------------|---|
| HCM Control Delay (s) | - | - 9.9 7.7 | - |
| HCM Lane LOS | - | - A A | - |
| HCM 95th %tile Q(veh) | - | - 0.2 0.1 | - |

Appendix B – Chamblee Lake Planned Development





To: Adam Culpepper, Town of ZebulonFrom: Jeff Hochanadel, PE, PTOE (Timmons Group)RE: Zebulon South TIA Response to CommentsDate: January 26, 2024

Timmons Group prepared the Zebulon South Traffic Impact Analysis (TIA) – sealed / submitted June 22nd, 2022, and submitted an updated TIA sealed / submitted on January 3rd, 2024. On January 24th, 2024, WSP issued comments (on the Town's behalf). Timmons Group (TG) reviewed WSP's comments (**in bold below**) and prepared the responses below.

Site Plan and Site Access:

1. Based on the updated site plan, the number of units has changed for the site since the TIA was completed. Please add a note in the body of the TIA report discussing this change and confirming that the analysis is still valid because it is more conservative than the current site plan.

TG Response: This was noted on pages 1-1 and 4-1

2. Please confirm site access locations in the Build Synchro files matches the site plan. If Site Access 2 is within 165 feet of Perry Curtis Road, this access will need to be rightin/right-out only. This is based on the 2003 NCDOT's Policy on Street and Driveway Access.

<u>TG Response</u>: Noted. The proposed centerline to centerline measurement between Site Access 2 and Perry Curtis Road is approximately 200-feet. Potential turning movement restrictions will be determined with the Town / NCDOT at the next stage of development when the Driveway permit is requested.

3. For tables 3-1, 3-2, and 5-1 in the TIA and tables 1 and 2 in the supplemental, please provide a footnote to describe the meaning of the "#" symbol in the queue lengths.

TG Response: The subject tables were updated accordingly.

4. For tables 3-1, 3-2, 5-1, and 5-2 in the TIA and tables 1 and 2 in the supplemental, please designate which intersections are unsignalized/signalized to aid in the differentiation of queues which are in feet and queues which are number of cars.

<u>TG Response</u>: The subject tables were updated accordingly.

5. For tables 3-1, 3-2, 5-1, and 5-2 in the TIA and tables 1 and 2 in the supplemental, please add units for queues.

<u>TG Response</u>: Units were clarified in the table headers and footnotes. To match Synchro reporting, 95th percentile queues were reported in feet (for signalized intersections) and 95th percentile queues were reported in car lengths (for unsignalized intersections)

6. Include NCDOT comments from July 2022 referenced in section 6 in the appendix of the TIA if available.

TG Response: This was added as Appendix F.

7. Please add a complete list of recommended improvements to the supplemental memo for clarity, even though the recommendations do not change from the TIA.

<u>TG Response:</u> The memo was updated accordingly.

- 8. The following comment responses were provided by Timmons Group based on the initial submittal review. Please add these explanations in the body of the TIA report to provide a full picture of the analysis methodology:
 - Include discussion on why count data was not balanced between intersections and why Perry Curtis Road volumes were used for site access 1 and 2. TG Response: Traffic volumes were not balanced to the presence of commercial site driveways and various side streets. To provide the most accurate analyses, corridor volumes were not balanced. Site Access 1 and 2 volumes were balanced with Perry Curtis due to the driveways' proximities.

TG Response: Additional explanation provided on page 2-2.

• Please provide justification for the 3% growth rate used for background volume development. TG Response: The 3% growth rate is based on published AADTs.

<u>TG Response</u>: Justification was provided on page 3-2.

• In the Build scenario turn lane analysis, it's mentioned that both S Wakefield Street and NC 96 will have 2026 AADTs higher than 4,000 vpd. Please clarify if this is based on the existing AADT value and an assumption of growth or if this is based on the existing AADT including an assumed growth rate. TG Response: NC-96's AADT currently exceeds 4,000 VPD. Per future projections, this value is not projected to decrease. S Wakefield Street AADT projections are based on recent AADT counts (grown at 3% annually to 2026) and 30% of daily site trips on S Wakefield Street north of Site Access 3.

<u>TG Response</u>: Additional description was provided in Section 5.1 for the three site access intersections (pages 5-1 and 5-2)

 Site access roads are listed as needing 100-feet of IPS. Please define IPS as internal protected stem in the text and reference the standards that guide this recommendation. TG Response: IPS was defined as "internal protected stem" in the updated TIA. IPS requirements are defined in the NCDOT's Driveway Manual.

TG Response: Additional description provided on page 6-1

9. The alignment of the S Wakefield Street/Morphius Bridge and Pully Gordon Road intersection is not ideal for safe operations, but no improvements are required currently.

<u>TG Response</u>: Noted. This intersection was not included in the previously negotiated TIA scope.

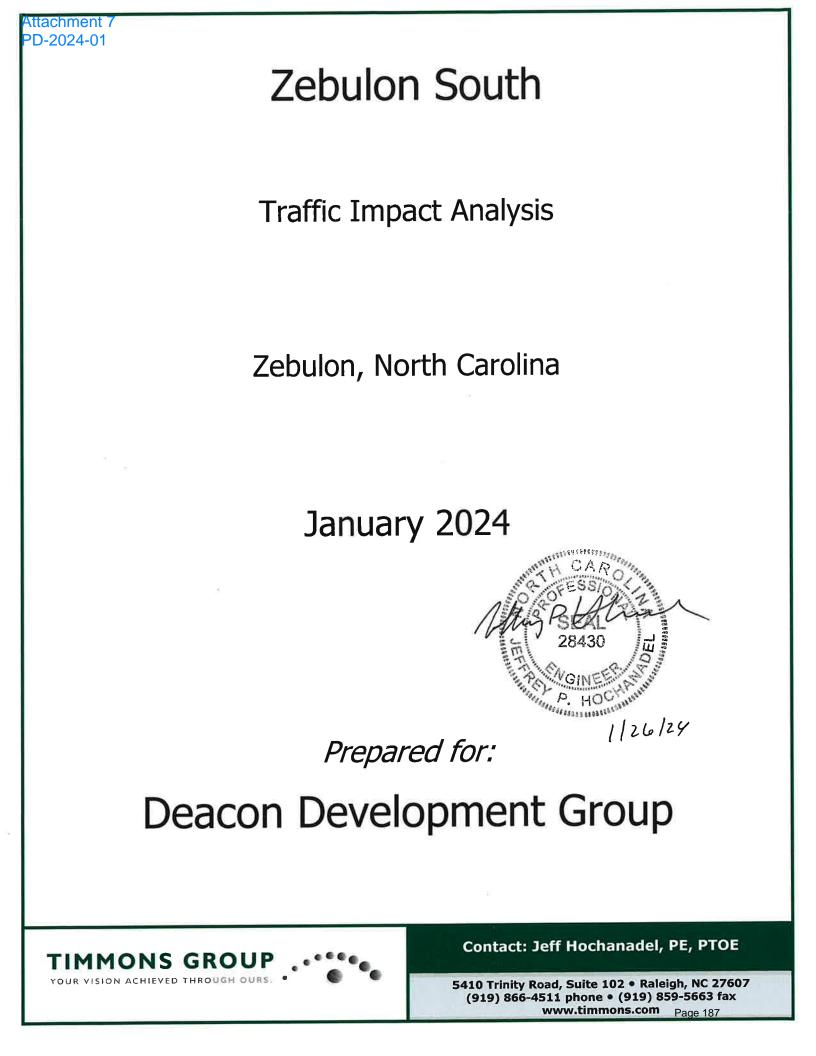


TABLE OF CONTENTS

| T/ | ABLE OF CO | NTENTSI |
|----|--|--|
| IJ | ST OF TAB | LESII |
| IJ | IST OF FIGU | JRESII |
| A | PPENDICES | з II |
| | | CTION1-1 |
| 2 | EXISTING | INFORMATION2-1 |
| | 2.1 | STUDY LIMITS |
| | 2.2 | EXISTING ROADWAYS |
| | 2.3 | EXISTING INTERSECTIONS2-2 |
| | 2.4 | TRAFFIC VOLUMES |
| | 2.5 | CAPACITY ANALYSIS |
| | | |
| 3 | EXISTING | AND BACKGROUND CONDITIONS AND ANALYSIS |
| 3 | EXISTING 3.1 | AND BACKGROUND CONDITIONS AND ANALYSIS |
| 3 | | |
| 3 | 3.1 | 2022 EXISTING ANALYSES |
| | 3.1 3.2 3.3 | 2022 EXISTING ANALYSES |
| | 3.1 3.2 3.3 | 2022 EXISTING ANALYSES |
| | 3.1 3.2 3.3 SITE TRIP | 2022 EXISTING ANALYSES |
| 4 | 3.1 3.2 3.3 SITE TRIP 4.1 4.2 | 2022 EXISTING ANALYSES |
| 4 | 3.1 3.2 3.3 SITE TRIP 4.1 4.2 | 2022 EXISTING ANALYSES |

LIST OF TABLES

| TABLE 2-1: LEVEL OF SERVICE DEFINITIONS 2-1 | 3 |
|---|---|
| TABLE 2-2: SIGNALIZED AND UNSIGNALIZED INTERSECTION LEVEL OF SERVICE CRITERIA | 4 |
| TABLE 3-1: INTERSECTION LEVEL OF SERVICE AND DELAY SUMMARY | |
| 2022 EXISTING TRAFFIC VOLUMES | 2 |
| TABLE 3-2: INTERSECTION LEVEL OF SERVICE AND DELAY SUMMARY | |
| 2026 BACKGROUND TRAFFIC VOLUMES | 4 |
| TABLE 4-1: TRIP GENERATION SUMMARY | 1 |
| TABLE 5-1: INTERSECTION LEVEL OF SERVICE, AND DELAY SUMMARY | |
| 2026 Build Traffic Volumes | 3 |
| TABLE 5-2: INTERSECTION LEVEL OF SERVICE, AND DELAY SUMMARY | |
| 2026 BUILD + IMPROVEMENTS TRAFFIC VOLUMES | 4 |

LIST OF FIGURES

FIGURE 1-1: SITE LOCATION MAP

- FIGURE 2-1: PRELIMINARY SITE LAYOUT
- FIGURE 2-2: 2022 EXISTING LANE CONFIGURATION
- FIGURE 2-3: 2022 EXISTING TRAFFIC VOLUMES
- FIGURE 3-1: 2026 BACKGROUND TRAFFIC VOLUMES
- FIGURE 4-1: TRIP DISTRIBUTION PERCENTAGES
- FIGURE 4-2: TRIP DISTRIBUTION VOLUMES
- FIGURE 5-1: 2026 BUILD TRAFFIC VOLUMES
- FIGURE 6-1: 2026 PROPOSED LANE CONFIGURATION

APPENDICES

- Appendix A Scoping Information
- Appendix B Traffic Counts
- Appendix C Traffic Signal Plans
- Appendix D Synchro Output
- Appendix E NCDOT Nomographs
- Appendix F NCDOT Requirements

1 INTRODUCTION

This report presents the Zebulon South traffic impact analysis (TIA) findings. The proposed development will be located between NC-96 and South Wakefield Street south of Barbee Street (see **Figure 1-1**). The proposed development was analyzed to consist of 225 single-family residential units and 125 multi-family residential units and will be constructed by 2026. The current site plan shows 186 single-family residential units and 134 multi-family residential units. This change results in a more conservative analysis.

Analyses were completed for the following scenarios:

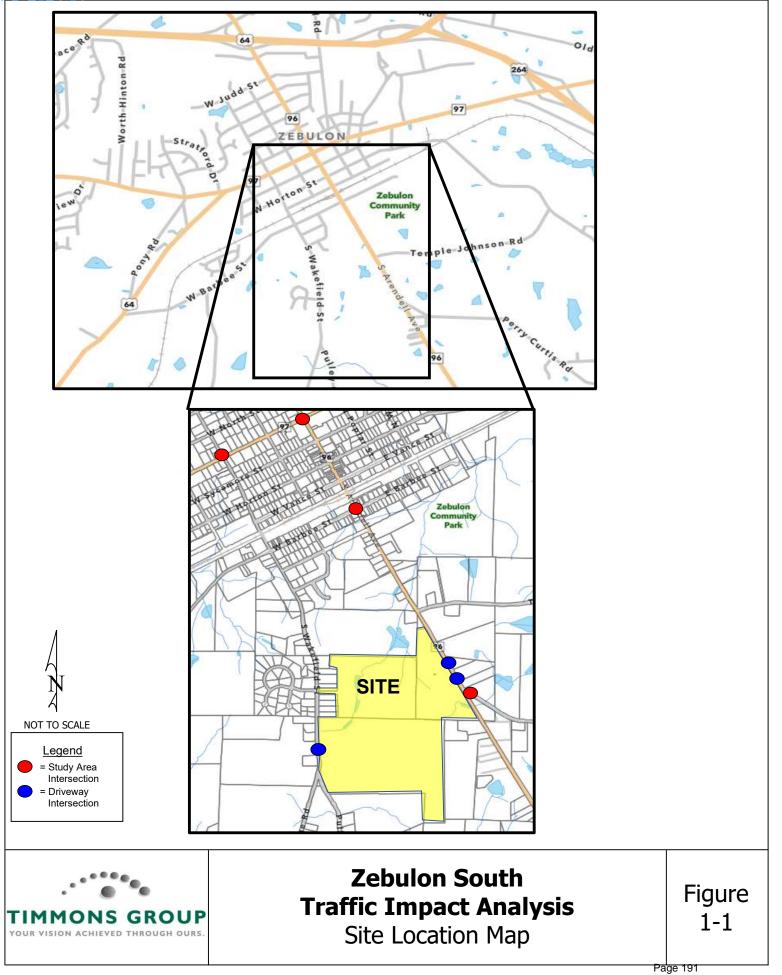
- 2022 Existing traffic volumes;
- 2026 Background traffic volumes; and
- 2026 Build traffic volumes (Background + site trips).

The purpose of this TIA is to verify that the existing geometry provided within the study area is sufficient to accommodate the projected traffic volumes, and to determine what, if any, proposed site access connection improvements are necessary.

The following steps were taken to determine the potential traffic impacts associated with this project:

- 1. <u>Data Collection</u> AM (7:00 9:00) and PM (4:00 6:00) peak hour turning movement counts were collected in April 2022 at the following intersections:
 - NC-97 (Gannon Ave) / SR-2349 (South Wakefield Street);
 - NC-97 (Gannon Ave) / NC-96 (Arendell Ave);
 - NC-96 (Arendell Ave) / SR-2348 (West Barbee Street); and
 - NC-96 (Arendell Ave) / SR-2347 (Perry Curtis Road).
- <u>Trip Generation/Future Traffic</u> Traffic generated by the proposed development was estimated using the 10th Edition of the Institute of Transportation Engineers' <u>Trip Generation Manual</u>. Trip generation was calculated following the NCDOT standards and practices for trip generation. Projected traffic volumes were calculated using a 3% ambient growth rate. Per the scoping document, there are currently no approved developments within the project study area (see **Appendix A**).
- <u>Trip Distribution and Projections</u> The site-generated trip distribution was based on existing area traffic and Engineering judgement. It was assumed, for purposes of analysis, that projected trips for the Zebulon South development would follow similar patterns as existing traffic.
- 4. <u>Traffic Capacity Analysis</u> Level of service analyses were performed using Synchro Version 11.1 for the following intersections:
 - NC-97 (Gannon Ave) / SR-2349 (South Wakefield Street) signalized;
 - NC-97 (Gannon Ave) / NC-96 (Arendell Ave) signalized;
 - NC-96 (Arendell Ave) / SR-2348 (West Barbee Street) unsignalized;
 - NC-96 (Arendell Ave) / Site Access 1 unsignalized;
 - NC-96 (Arendell Ave) / Site Access 2 unsignalized;
 - NC-96 (Arendell Ave) / SR-2347 (Perry Curtis Road) unsignalized; and
 - SR-2349 (South Wakefield Street) / Site Access 3 unsignalized.
- 5. <u>Review of Proposed Improvements</u> Roadway improvements proposed to accommodate projected site-generated traffic were evaluated.

Attachment 7 PD-2024-01



2 EXISTING INFORMATION

The proposed development will be located NC-96 and South Wakefield Street south of West Barbee Street (see **Figure 1-1**).

2.1 STUDY LIMITS

Access to the proposed site will be provided via three (3) full movement connections: Two (2) to NC-96, and one (1) to South Wakefield Street. The preliminary site layout includes these proposed development site entrances (see **Figure 2-1**). All figures are located at the end of their respective chapter.

The study limits include the following seven (7) intersections:

- NC-97 (Gannon Ave) / SR-2349 (South Wakefield Street) signalized;
- NC-97 (Gannon Ave) / NC-96 (Arendell Ave) signalized;
- NC-96 (Arendell Ave) / SR-2348 (West Barbee Street) unsignalized;
- NC-96 (Arendell Ave) / Site Access 1 unsignalized;
- NC-96 (Arendell Ave) / Site Access 2 unsignalized;
- NC-96 (Arendell Ave) / SR-2347 (Perry Curtis Road) unsignalized; and
- SR-2349 (South Wakefield Street) / Site Access 3 unsignalized.

2.2 EXISTING ROADWAYS

NC-97 (W Gannon Ave) is an undivided facility with a varying two to three-lane cross section, running approximately east-west in the study area. The facility is classified by NCDOT as a minor arterial. Within the study area, NC-97 has a posted 35-mph speed limit and provides connection to downtown Zebulon. This facility primarily serves residential and commercial land uses within the study area. Per 2021 NCDOT Average Annual Daily Traffic (AADT) maps, NC-97 carries 14,500 vehicles per day (VPD) west of NC-96.

NC-96 (Arendell Ave) is a two-lane undivided facility, that runs approximately north-south in the study area. The facility is classified by NCDOT as a minor arterial. Within the study area, NC-96 has a posted 20-mph speed limit north of West Barbee St, 35-mph speed limit south of West Barbee St, and 45-mph speed limit south of the town limits. This facility primarily serves residential and commercial land uses within the study area. Per 2021 NCDOT AADT maps, NC-96 carries 6,700 VPD south of NC-97.

SR-2349 (South Wakefield Street) is a two-lane undivided facility, that runs approximately northsouth in the study area. The facility is classified by NCDOT as a local road. Within the study area, NC-96 has a posted 25-mph speed limit north of West Horton St, a posted 35-mph speed limit south of West Horton Street and north of Primrose Place, and a 45 mph speed limit south of Primrose Place. This facility provides a connection to Zebulon. Per 2015 NCDOT AADT maps, South Wakefield Street carries 3,800 VPD south of NC-97.

SR-2348 (West Barbee Street) is a two-lane undivided facility, that runs approximately east-west in the study area. The facility is classified by NCDOT as a local road. Within the study area, West Barbee Street has a posted 25-mph speed limit and serves primarily residential and commercial land uses. Per 2015 NCDOT AADT maps, the facility carries 1,800 VPD east of South Wakefield Street.

SR-2347 (Perry Curtis Road) is a two-lane undivided facility that runs approximately east-west in the project study area, providing access to NC-96. The facility is classified by NCDOT as a local road. Within the study area, Perry Curtis Road primarily serves residential land uses and has a posted 55-mph speed limit. Per 2015 NCDOT AADT maps, the facility carries 1300 VPD east of NC-96.

2.3 EXISTING INTERSECTIONS

Using available aerial imagery, Timmons Group compiled the existing geometry for each study area intersection. The existing intersection geometry is shown in **Figure 2-2**.

NC-97 / South Wakefield Street is a two-phase signalized intersection. The north and southbound intersection approaches include a single shared left / through / right-turn lane. The east and westbound intersection approaches include of an exclusive left-turn lane and a shared through / right-turn lane.

NC-97 / NC-96 is an eight-phase signalized intersection. The north, south, and westbound intersection approaches include an exclusive left-turn lane and a shared through / right-turn lane. The eastbound approach includes exclusive right-turn, through, and left-turn lanes.

NC-96 / West Barbee Street is an unsignalized intersection with the east and westbound approaches encountering the stopped condition. All approaches include a single shared lane that serves all movements.

NC-96 / Perry Curtis Road is an unsignalized intersection with the westbound approach encountering the stopped condition. The northbound approach includes a shared through / right-turn lane. The southbound approach includes a shared through / left-turn lane. The westbound approach includes a shared left / right-turn lane.

2.4 TRAFFIC VOLUMES

Timmons Group calculated peak hour volumes at the study area intersections using the collected AM (7:00 – 9:00) and PM (4:00 – 6:00) peak period turning movement counts undertaken in April 2022. Collected traffic count data is summarized in **Figure 2-3**. The complete traffic count data is found in **Appendix B**.

Traffic volumes were not balanced to the presence of commercial site driveways and various side streets. To provide the most accurate analyses, corridor volumes were not balanced. Site Access 1 and 2 volumes were balanced with Perry Curtis due to the driveways' proximities.

2.5 CAPACITY ANALYSIS

Using field observations, aerial photography, and traffic count data, traffic operations were analyzed during 2022 (existing) and 2026 (without and with the proposed development site trips).

Capacity analysis allows traffic engineers to determine the impacts of traffic on the surrounding roadway network. The Transportation Research Board's (TRB) *Highway Capacity Manual* (HCM) methodologies govern how the capacity analyses are conducted and how the results are interpreted. There are six letter grades of Levels of Service (LOS) from A to F, with LOS A representing the best operating conditions and LOS F the worst operating conditions. At signalized intersections, an overall intersection LOS E is generally considered unacceptable. At unsignalized intersections, a LOS E is generally considered acceptable only if the side street encounters delay. Nevertheless, side streets typically function at a LOS F during peak traffic periods, because the traffic volumes often do not warrant a traffic signal to assist side street traffic. **Table 2-1** shows in detail how each of these levels of service are interpreted.

| Level of Service | Roadway Segments or Controlled Access Highways | Intersections | |
|---------------------|--|---|------------|
| A | Free flow, low traffic density. | No vehicle waits longer than one signal indication. | |
| В | Delay is not unreasonable, stable traffic flow. | On a rare occasion motorists wait through more than one signal indication. | |
| С | Stable condition, movements somewhat restricted due to higher volumes, but not objectionable for motorists. | Intermittently drivers wait through more than one signal indication, and occasionally backups may develop behind left turning vehicles, traffic flow still stable and acceptable. | |
| D | Movements more restricted, queues and delays may occur during short peaks, but lower demands occur often enough to permit clearing, thus preventing excessive backups. | Delays at intersections may become extensive with some, especially left-turning vehicles waiting two or more signal indications, but enough cycles with lower demand occur to permit periodic clearance, thus preventing excessive backups. | D |
| E | Actual capacity of the roadway invloves delay to all motorists due to congestion. | Very long queues may create lengthly delays, especially for left-turning vehicles. | A COLORADO |
| F | Forced flow with demand volumes greater than capacity resulting in complete congestion. Volumes drop to zero in extreme cases. | Backups from locations downstream restrict or prevent movement of vehicles out of approach creating a storage ares during part or all of an hour. | F |

Table 2-1: Level of Service Definitions

SOURCE: "A Policy on Design of Design of Urban Highways and Arterial Streets" - AASHTO, 1973 based upon material published in "Highway Capacity Manual", National Academy of Sciences, 1965.

For signalized and unsignalized intersections, level of service is defined in terms of **delay**, a measure of driver discomfort, frustration, fuel consumption and lost travel time. Table 2-2 summarizes the delay associated with each LOS category:

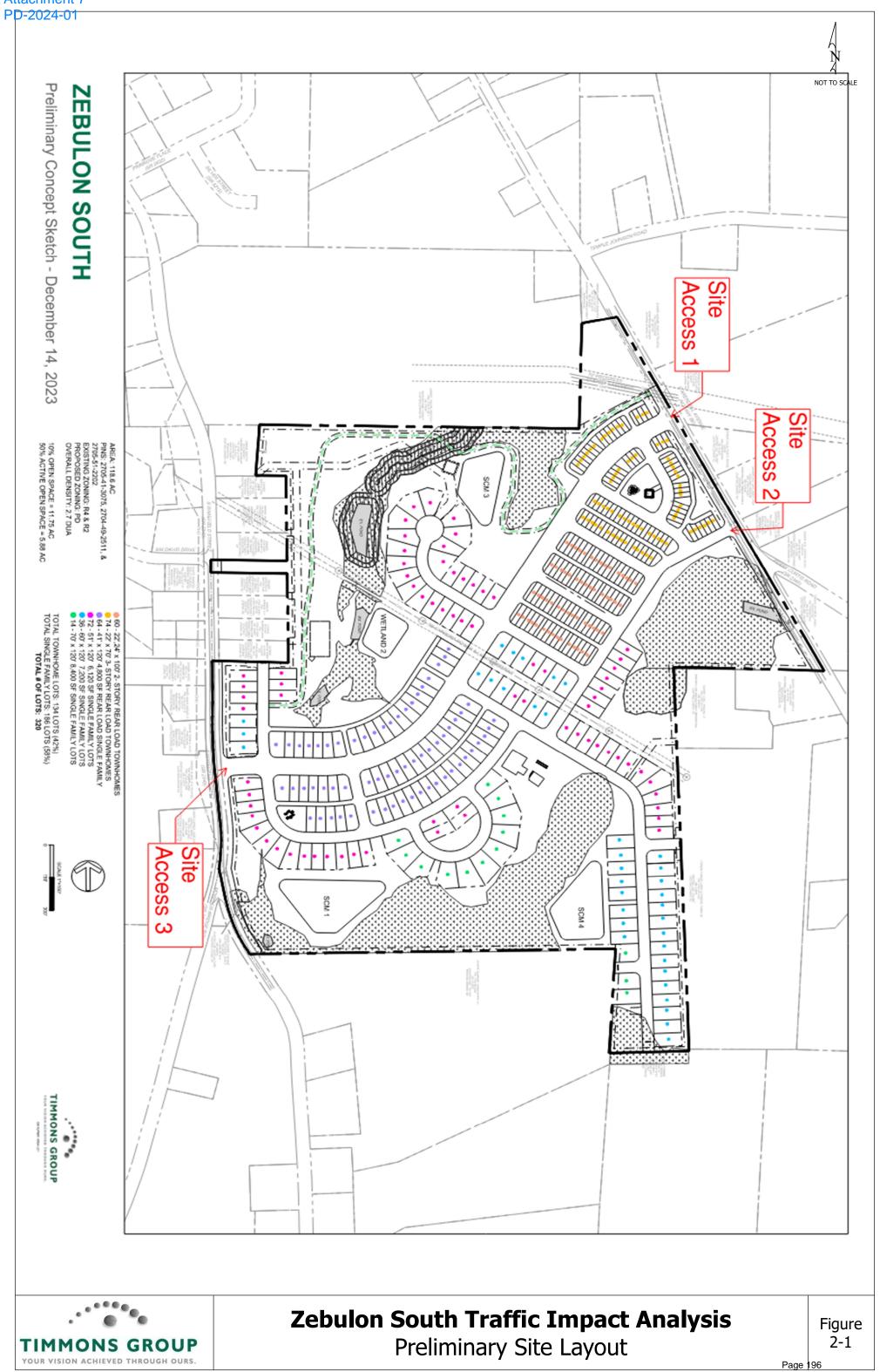
| Signalize | ed Intersections | Unsignalized Intersections | | | |
|---------------------|--|----------------------------|------------------------------------|--|--|
| Level of Service | Control Delay per Vehicle (sec/veh) | Level of Service | Average Control Delay (sec/veh) | | |
| А | ≤ 10 | А | 0 to 10 | | |
| В | > 10 to ≤ 20 | В | > 10 to \leq 15 | | |
| С | > 20 to ≤ 35 | С | > 15 to ≤ 25 | | |
| D | > 35 to ≤ 55 | D | > 25 to \leq 35 | | |
| E | > 55 to ≤ 80 | E | > 35 to \leq 50 | | |
| F | > 80 | F | > 50 | | |

Table 2-2: Signalized and Unsignalized Intersection Level of Service Criteria

Source: Exhibit 16-2 and Exhibit 17-2 from TRB's "Highway Capacity Manual 2000"

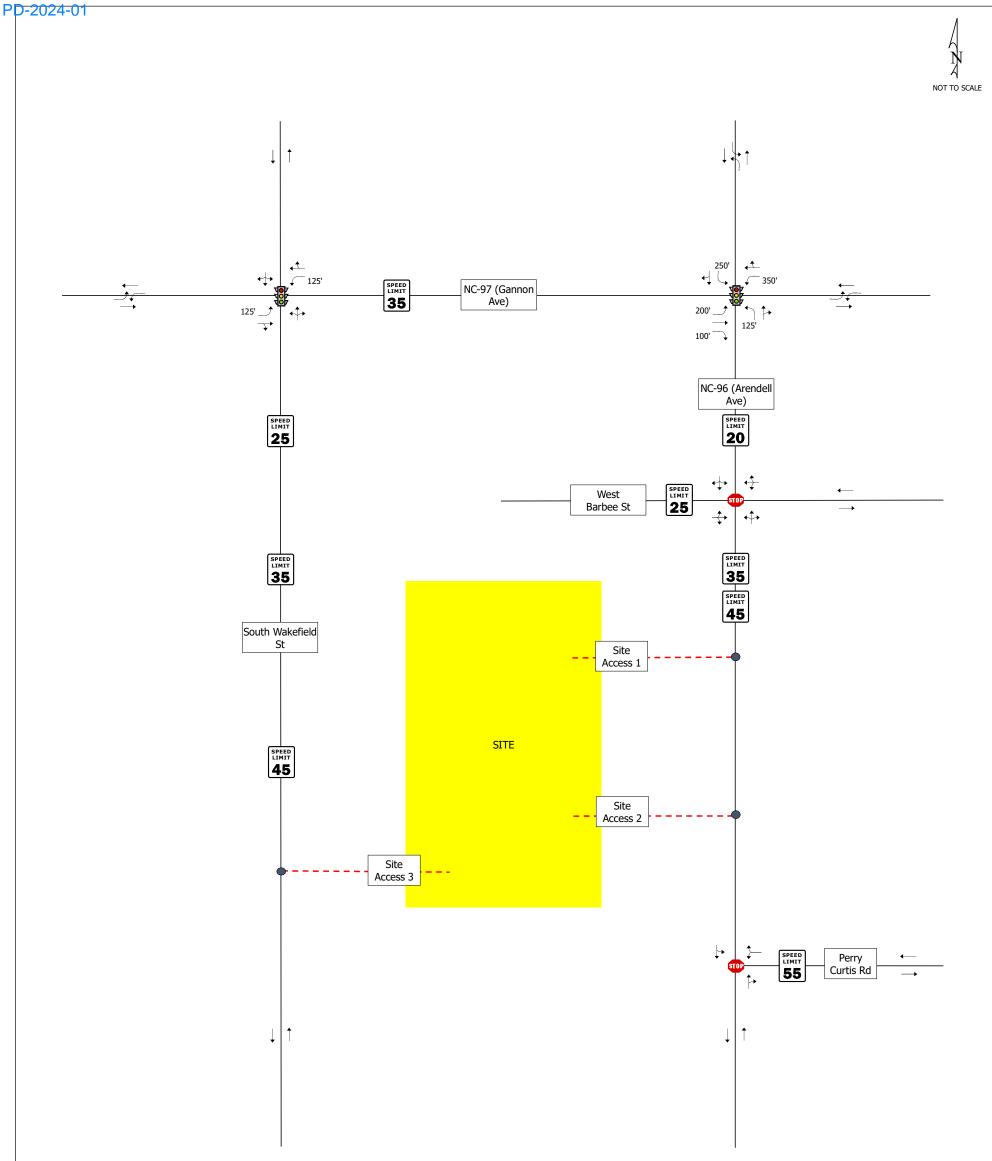
Capacity analyses were performed to assess operational conditions. Study area intersections were analyzed using Synchro Version 11.1 based on Highway Capacity Manual (HCM) methodologies with the following assumptions:

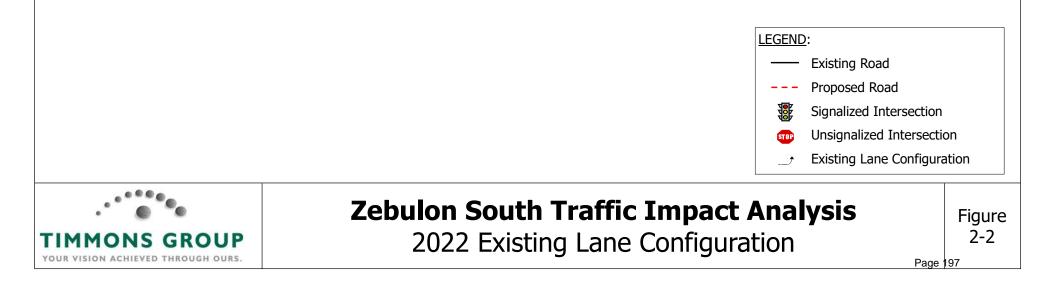
- Existing grades;
- 12-foot lane widths;
- No parking activity, bus stops, or pedestrians;
- Peak hour factor (PHF) of 0.90;
- Heavy vehicle percentages 2%; and
- Minimum turning movement volume of 4 vehicles per hour (VPH) for all allowed movements; and
- Existing traffic signal plan signal data (see **Appendix C**).



Attachment 7





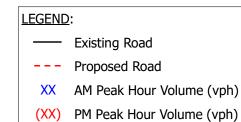


Attachment 7

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Zebulon South Traffic Impact Analysis

2022 Existing Traffic Volumes

Figure 2-3 Page 198

3 EXISTING AND BACKGROUND CONDITIONS AND ANALYSIS

3.1 2022 EXISTING ANALYSES

Table 3-1 summarizes the 2022 Existing intersection LOS and delay based on the geometry shown in **Figure 2-2** and the 2022 Existing traffic volumes shown in **Figure 2-3**. The corresponding Synchro output is included in **Appendix D**.

The signalized intersection of NC-97 / South Wakefield Street is currently operating at an overall LOS B during both 2022 Existing peak hours. All intersection approaches are currently operating at a LOS C or better during both peak hours.

The signalized intersection of NC-97 / NC-96 is currently operating at an overall LOS C during both 2022 Existing peak hours. All intersection approaches are currently operating at a LOS D or better during both peak hours.

All NC-96 / West Barbee Street unsignalized intersection approaches are currently operating at a LOS C or better during the 2022 Existing AM and PM peak hours.

All NC-96 / Perry Curtis Road unsignalized intersection approaches are currently operating at a LOS B or better during the 2022 Existing AM and PM peak hours.

| | | AM PEAK HOU | | JR PM PEAK HOUR | | | | AM PEAK HOUR | PM PEAK HOUR |
|--------------------------------------|-----------------------|---------------------------------|-------|---------------------------------|-------|--------------------|------------------------------|--|--|
| Intersection | Approach / Overall | Delay ¹ (sec/veh) | LOS 1 | Delay ¹ (sec/veh) | LOS 1 | Movement | Turn Lane Storage (ft) | 95th Percentile Queue Length (ft)* | 95th Percentile Queue Length (ft)* |
| 1: S Wakefield Street & NC-97 | | () () | | | | EB Left | 125 | 5 | 6 |
| Gannon Avenue) | Eastbound | 11.6 | В | 10.8 | в | EB Thru/Right | | 170 | 291 |
| Signalized | | | | | | EB Approach | | 22.5 | - <u>-</u> |
| | | | | | | WB Left | 125 | 21 | 24 |
| | Westbound | 11.2 | В | 7.7 | A | WB Thru/Right | | 168 | 170 |
| | | | | | | WB Approach | | 245 | 1944 1 |
| | Northbound | 18.1 | в | 24.6 | с | NB Left/Thru/Right | | 94 | 113 |
| | Northbound | 10.1 | D | 24.0 | 100 | NB Approach | | 77-2 | |
| | Southbound | 11.0 | в | 17.9 | в | SB Left/Thru/Right | | 18 | 44 |
| | Southbound | 11.0 | D | 17.5 | D | SB Approach | | 227 | <u></u> |
| | Overall | 12.5 | В | 11.3 | В | Overall | | 2257 | 1722 |
| 2: NC-96 (Arendell Avenue) & NC-97 | | | | | | EB Left | 200 | 34 | 52 |
| Gannon Avenue)) Signalized | Eastbound | 31.2 | с | 32.6 | с | EB Thru | | #399 | #321 |
| signalizeu | Eastbound | | | | | EB Right | 100 | 47 | 56 |
| | | | | | | EB Approach | | 20 | 1000 |
| | Westbound | 21.4 | с | 40.3 | D | WB Left | 350 | 125 | #175 |
| | | | | | | WB Thru/Right | | 271 | #599 |
| | | | | | | WB Approach | | 22.0 | |
| | Northbound | 27.3 | с | 33.1 | с | NB Left | 125 | 54 | 50 |
| | | | | | | NB Thru/Right | | 233 | 357 |
| | | | | | | NB Approach | | | 10 4 |
| | | | | | | SB Left | 250 | 22 | 35 |
| | Southbound | 31.9 | С | 26.0 | C | SB Thru/Right | | 170 | 184 |
| | | | | | | SB Approach | | 22.5 | 397 |
| | Overall | 27.2 | С | 34.7 | С | Overall | | | - |
| 3: NC-96 (Arendell Avenue) & | Eastbound | 12.7 | в | 15.7 | с | EB Left/Thru/Right | | 0.4 | 1.1 |
| Barbee Street <i>Unsignalized</i> | Eastbound | 12.7 | D | 15.7 | | EB Approach | | | |
| Unsignalizeu | Westbound | 12.3 | в | 13.9 | в | WB Left/Thru/Right | | 0.1 | 0.1 |
| | westbourid | 12.5 | D | 13.9 | D | WB Approach | | 225 | 1944 1 |
| | Northbound | 1.1 | A | 1.1 | A | NB Left/Thru/Right | | 0.1 | 0.1 |
| | Northbound | 1.1 | | 1.1 | | NB Approach | | 370 | |
| | Southbound | 0.2 | A | 0.2 | A | SB Left/Thru/Right | | 0 | 0 |
| | Southbound | 0.2 | A | 0.2 | A | SB Approach | | 225 | 84 |
| 5: NC-96 (Arendell Avenue) & Perry | Westbound | 10.3 | в | 10.0 | в | WB Left/Right | | 0.4 | 0.2 |
| Curtis Road <i>Unsignalized</i> | Westboulid | 10.5 | D | 10.0 | D | WB Approach | | 370 | |
| unsignalized | Northbound | 0.0 | | 0.0 | A | NB Thru/Right | | 0 | 0 |
| | Norunbourid | 0.0 | A | 0.0 | A | NB Approach | | 227 | 344 1 |
| | Southbound | 1.4 | A | 2.2 | A | SB Left/Thru | | 0.1 | 0.2 |
| | Soumbound | 1.4 | A | 2.2 | A | SB Approach | | 375 | |

Table 3-1: Intersection Level of Service and Delay Summary 2022 Existing Traffic Volumes

¹ Overall intersection LOS and delay not reported for TWSC intersections.

* - 95th percentile queues for unsignalized intersections reported in number of vehicles.

- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles

3.2 2026 BACKGROUND TRAFFIC VOLUMES

Figure 3-1 shows the 2026 ambient traffic volumes calculated using a 3% growth rate (based on published AADTs) for four (4) years.

Per the scoping document, there are currently no approved area developments or public improvement projects within the study area (see **Appendix A**).

3.3 2026 BACKGROUND ANALYSIS

Table 3-2 summarizes the intersection LOS and delay based on the geometry shown in **Figure 2-2** and the 2026 Background traffic volumes shown in **Figure 3-1**. The corresponding Synchro output is included in **Appendix D**.

The signalized intersection of NC-97 / South Wakefield Street is projected to operate at an overall LOS B and C during the 2026 Background AM and PM peak hours, respectively. All intersection approaches are projected to operate at a LOS D or better during both peak hours.

The signalized intersection of NC-97 / NC-96 is projected to operate at an overall LOS C during the 2026 Background peak hours. All intersection approaches are projected to operate at a LOS D or better during both peak hours.

All NC-96 / West Barbee Street unsignalized intersection approaches are projected to operate at a LOS C or better during the 2026 Background AM and PM peak hours.

All NC-96 / Perry Curtis Road unsignalized intersection approaches are projected to operate at a LOS B or better during the 2026 Background AM and PM peak hours.

| | _ | AM PEAK HOUR | | PM PEAK HOUR | | | Turn Long | AM PEAK HOUR 95th | PM PEAK HOUR 95th |
|------------------------------------|--------------------------|---------------------------------|-------|---------------------------------|-------|-----------------------------|------------------------------|--|--|
| Intersection | Approach / Overall | Delay ¹ (sec/veh) | LOS 1 | Delay ¹ (sec/veh) | LOS 1 | Movement | Turn Lane Storage (ft) | Percentile Queue Length (ft)* | 95th Percentile Queue Length (ft)* |
| 1: S Wakefield Street & NC-97 | | | | | | EB Left | 125 | 10 | 10 |
| (Gannon Avenue) | Eastbound | 22.3 | С | 25.1 | С | EB Thru/Right | | 368 | #602 |
| Signalized | | | | | | EB Approach | | (1 11) | |
| | | | | | | WB Left | 125 | 63 | 72 |
| | Westbound | 13.4 | В | 10.0 | Α | WB Thru/Right | | 241 | 184 |
| | | | | | | WB Approach | | 5 | |
| | No. diamat | 21.2 | | | D | NB Left/Thru/Right | | 199 | #213 |
| | Northbound | 31.3 | С | 54.1 | D | NB Approach | | 8 <u>80</u> | |
| | 2010/00/00 | 200400 | 1 | | 144 | SB Left/Thru/Right | | 35 | 67 |
| | Southbound | 23.3 | C | 33.8 | С | SB Approach | | 5.00 | |
| | Overall | 20.0 | В | 22.7 | С | Overall | | | |
| 2: NC-96 (Arendell Avenue) & NC-97 | | | - | | | EB Left | 200 | 45 | 72 |
| (Gannon Avenue)) | Eastbound | 31.6 | с | 36.6 | D | EB Thru | 12/2011 | 360 | #309 |
| Signalized | | | | | | EB Right | 100 | 60 | 70 |
| | | | | | | EB Approach | | 222 | |
| | Westbound | 21.6 | с | 27.2 | с | WB Left | 350 | #226 | #276 |
| | | | | | | WB Thru/Right | | 155 | 339 |
| | | 2007053553 | | | | WB Approach | | 35 | |
| | Northbound | 38.6 | D | 37.1 | D | NB Left | 125 | 92 | 74 |
| | | | | | | NB Thru/Right | | #304 | #422 |
| | | | | | | NB Approach | | | |
| | Southbound | - | | 26.4 | с | SB Left | 250 | 36 | 66 |
| | | 30.8 | с | | | SB Thru/Right | | 195 | 189 |
| | | | | | | SB Approach | | | |
| | Overall | 29.8 | с | 31.8 | с | Overall | | 899 23 | |
| 3: NC-96 (Arendell Avenue) & | 111 | | | b s | 0.0 | EB Left/Thru/Right | () () | 0.4 | 1.5 |
| Barbee Street | Eastbound | 13.6 | В | 18.3 | С | EB Approach | | | |
| Unsignalized | | | | | | WB Left/Thru/Right | | 0.1 | 0.1 |
| | Westbound | 13.0 | В | 14.9 | В | WB Approach | | - | |
| | | | | | | NB Left/Thru/Right | | 0.1 | 0.1 |
| | Northbound | 1.1 | A | 1.1 | Α | NB Approach | | 0.1 | 0.1 |
| | | | | | | SB Left/Thru/Right | | 0 | 0 |
| | Southbound | 0.2 | A | 0.2 | Α | SB Approach | | - | |
| 6: NC-96 (Arendell Avenue) & Perry | | - | - | | - | WB Left/Right | | 0.4 | 0.3 |
| Curtis Road | Westbound | 10.7 | В | 10.4 | В | WB Approach | | | |
| Unsignalized | - | | | | | NB Thru/Right | | 0 | 0 |
| | Northbound | 0.0 | A | 0.0 | Α | | | - | |
| | | | - | - | | NB Approach SB Left/Thru | | 0.1 | 0.3 |
| | Southbound | 1.4 | A | 2.2 | Α | | | 0.1 | |
| | 100.00 00 0000000 | 1 | 1 | | - and | SB Approach | | ·/···· | |

Table 3-2: Intersection Level of Service and Delay Summary2026 Background Traffic Volumes

¹ Overall intersection LOS and delay not reported for TWSC intersections.

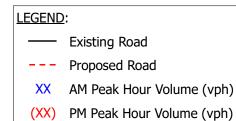
* - 95th percentile queues for unsignalized intersections reported in number of vehicles.

- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles

Attachment 7

PD-2024-01

A N A NOT TO SCALE 26 (55) ↓ 19 (21) 248 (300) ↓ 1 241 (429) ↓ 19 (57)
 ↓ 349 (494)
 ↓ 221 (245) (1) (558) (55) 19 (29) 5 (19) ↓ $\begin{array}{c} \bullet & 3 \\ \bullet & 517 \\ \hline & 46 \end{array}$ 2 (7) 627 589 (677) (796) NC-97 (Gannon Ave) 86 176 138 (79) (306) (154) 529 609 6 482 41 46 446 70 (10) (669) (113) ((66) (352) (72)
 Image: 10 state
 Image: 10 \rightarrow $\overrightarrow{}$ (792) (557) NC-96 (Arendell Ave) 20 145 1 (47) (371) (9) (7) (3) (0) 2 1 0 69 (86) € 3 (10) West 48 270 0 (36) (225) (2) <mark>2</mark> (16) Barbee St (51) → (5) → (73) → 53 34 1 18 (129) 146 (389) South Wakefield Ļ St Site Access 1 Î 301 (215) SITE 146 (389) ↓ Site Access 2 Î <mark>92</mark> (187) <mark>301</mark> (215) Site Access 3 Î 214 (168) 119 27 (281) (108) ↓ ↓ 84 (53) ↓ 70 14 (47) (6) Perry Curtis Rd 30 (115) Ť (231 3 (168) (7) ↑ <mark>214</mark> (168) 92 (187) ↓ 133 (287) ↓ ↑ 234 (175)





Zebulon South Traffic Impact Analysis 2026 Background Traffic Volumes

Figure 3-1

4 SITE TRIP GENERATION AND DISTRIBUTION

Proposed development site trips were estimated based on the proposed land uses supplied by the developer and subsequently distributed onto the surrounding roadway network.

4.1 TRIP GENERATION

The site-generated trips shown in **Table 4-1** are based on trip generation information provided in the 10th Edition of the Institute of Transportation Engineers' (ITE's) *Trip Generation Manual* and the anticipated development size. The trip generation was calculated using the proposed number of residential units as the independent variable and the provided equation (per NCDOT standards). The proposed development was analyzed to consist of 225 single-family residential units and 125 multi-family residential units and will be constructed by 2026. The current site plan shows 186 single-family residential units and 134 multi-family residential units. This change results in a more conservative analysis.

| ITE Land Use Code | Independent | dent ADT | | M Peak Ho | our | PM Peak Hour | | |
|--|-------------|----------|-----|-----------|-------|--------------|-----|-------|
| | Variable | ADT | In | Out | Total | In | Out | Total |
| 210- Single Family Detached Housing | 225 | 2193 | 41 | 124 | 165 | 139 | 82 | 221 |
| 220- Multifamily Housing (Low-Rise) | 125 | 904 | 14 | 45 | 59 | 45 | 27 | 72 |
| Total | 3097 | 55 | 169 | 224 | 184 | 109 | 293 | |

Table 4-1: Trip Generation Summary

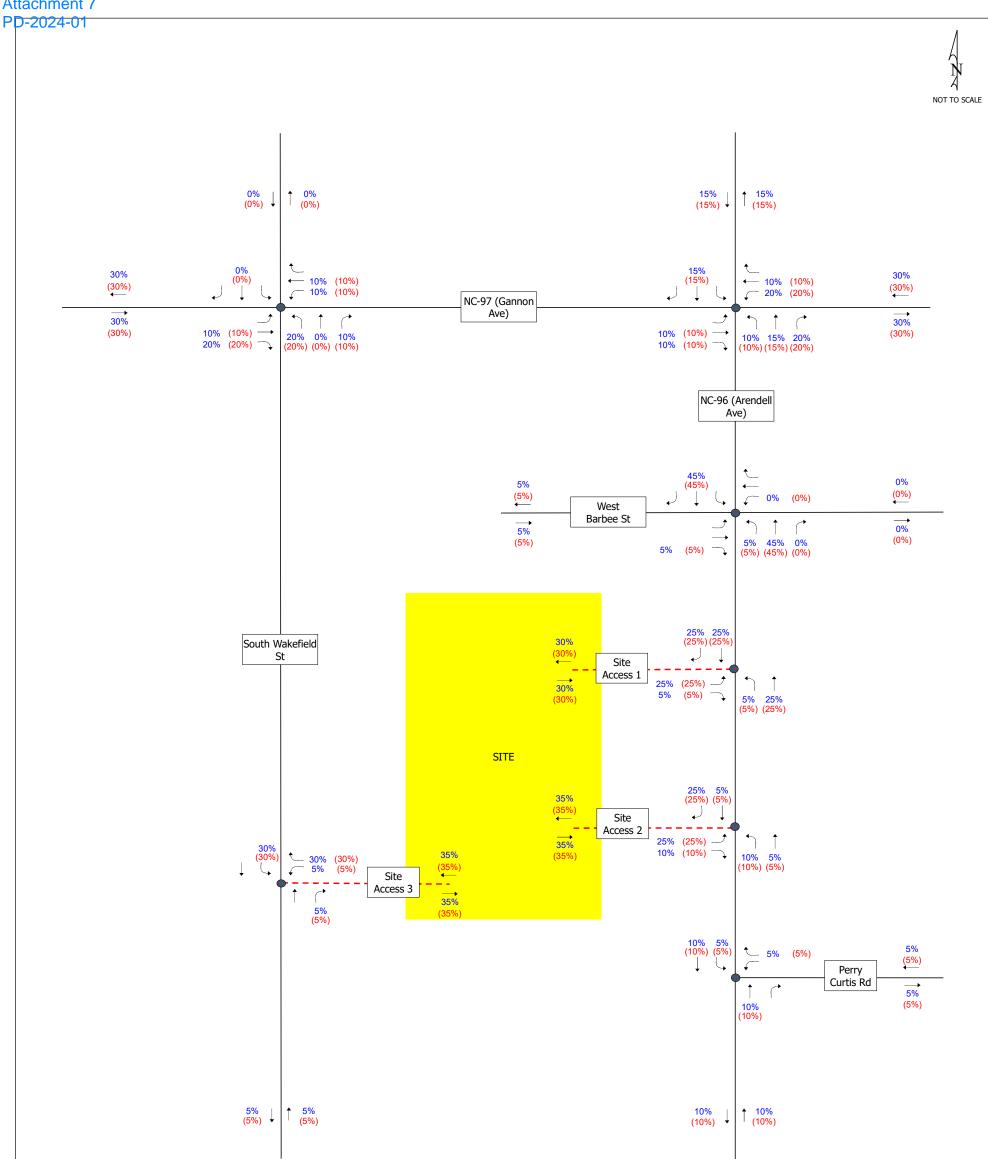
SOURCE: Institute of Transportation Engineers' *Trip Generation Manual* 10th Edition (2017)

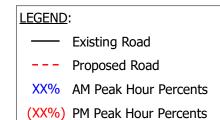
AM peak hour trips generated totaled 55 incoming and 169 outgoing where PM peak hour trips totaled 184 incoming and 109 outgoing. Average daily traffic (ADT) volumes generated by the development totaled 3,097 VPD. No reduction in trips were included due to internal capture or pass-by trips.

4.2 TRIP DISTRIBUTION

The directional traffic patterns, or trip distribution, of the site-generated traffic was determined using the existing traffic characteristics and engineering judgement. It was assumed, for purposes of this study, that all site traffic would enter and exit the study area in a similar manner as the existing traffic. Area trip distribution is based on traffic counts performed by Timmons Group. Total trips into and out of the study area using NC-96, NC-97, South Wakefield Street, and Perry Curtis Road form the basis for the percentage distribution. The percentages were routed, via shortest path, to and from the proposed development. The distribution percentages were then applied to the generated trips to predict routes and project traffic volumes for the 2026 Build scenario. Trip distribution percentages are shown in **Figure 4-1** and trip distribution volumes are shown in **Figure 4-2**.



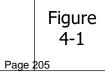




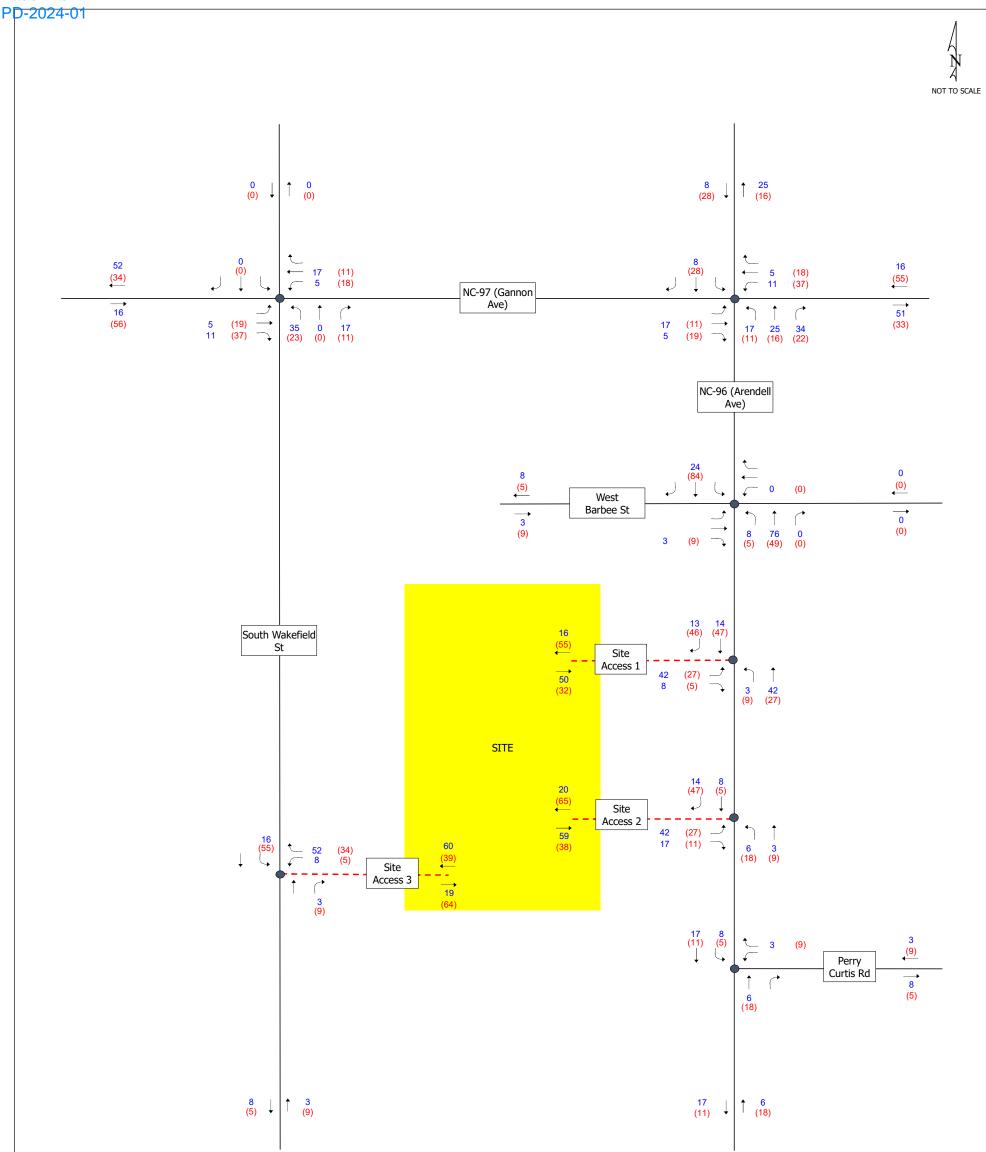


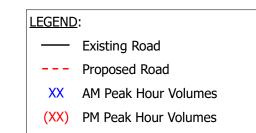
Zebulon South Traffic Impact Analysis

Trip Distribution Percentages



Attachment 7







Zebulon South Traffic Impact Analysis Trip Distribution Volumes

Figure 4-2

5 2026 BUILD CONDITION AND ANALYSIS

To complete the 2026 Build analyses (including proposed development), the estimated site trips were added to the 2026 Background traffic volumes. The projected total volumes, along with the existing intersection geometry, were used to complete the capacity analyses. The 2026 Background traffic volumes (**Figure 3-1**) were added to the projected site trips (**Figure 4-2**) to generate the 2026 Build traffic volumes (background + site) shown in **Figure 5-1**.

To summarize, the 2026 Build traffic volumes shown in **Figure 5-1** contain the following:

- Existing 2022 traffic volumes grown by a 3% per year ambient growth rate for 4 years; and
- Site trips generated by the subject development.

5.1 2026 BUILD ANALYSIS

Table 5-1 summarizes the intersection LOS and delay based on the geometry shown in **Figure 2-2** and the 2026 Build traffic volumes shown in **Figure 5-1**. The corresponding Synchro output is included in **Appendix D**.

The signalized intersection of NC-97 / South Wakefield Street is projected to operate at an overall LOS C during both 2026 Build peak hours. All intersection approaches are projected to operate at a LOS D or better during both peak hours. No improvements are recommended at this intersection due to the proposed development's construction.

The signalized intersection of NC-97 / NC-96 is projected to operate at an overall LOS C and D during the 2026 Build AM and PM peak hours, respectively. All intersection approaches are projected to operate at a LOS D or better during both peak hours. No improvements are recommended at this intersection due to the proposed development's construction.

All NC-96 / West Barbee Street unsignalized intersection approaches are projected to operate at a LOS C or better during the 2026 Background AM and PM peak hours. No improvements are recommended at this intersection due to the proposed development's construction.

All NC-96 / Site Access 1 unsignalized intersection approaches are projected to operate at a LOS C or better during the 2026 Background AM and PM peak hours. Per the NCDOT Policy on Street and Driveway Access to North Carolina Highways Manual:

"Generally left and right turn lanes and tapers shall be considered when:

• In accordance with G.S. 136-18(29), the average daily traffic meets or exceeds 4,000 vehicles per day on any secondary route (the average daily traffic should include both the existing traffic plus traffic generated by the proposed development)"

The 2026 AADT along NC-96 currently exceeds 4,000 VPD and is not projected to decrease. Because of this, turn lanes were considered at Site Access 1. Per the NCDOT Nomograph (see **Appendix E**) and projected 2026 peak hour volumes, a 50-foot southbound right-turn lane (with appropriate taper) is recommended. As shown in **Table 5-2**, all intersection movements are projected to operate acceptably following the turn-lane's construction. No additional improvements are recommended at this intersection due to the proposed development's construction.

All NC-96 / Site Access 2 unsignalized intersection approaches are projected to operate at a LOS B or better during the 2026 Background AM and PM peak hours. The 2026 AADT along NC-96 currently exceeds 4,000 VPD and is not projected to decrease.Because of this, turn lanes were considered at Site Access 2. Per the NCDOT Nomograph (see **Appendix E**) and projected 2026 peak hour volumes, a 50-foot southbound right-turn lane (with appropriate taper) is recommended. As shown in **Table 5-2**, all intersection movements are projected to operate acceptably following the turn-lane's construction. No additional improvements are recommended at this intersection due to the proposed development's construction.

All NC-96 / Perry Curtis Road unsignalized intersection approaches are projected to operate at a LOS B or better during the 2026 Background AM and PM peak hours. No improvements are recommended at this intersection due to the proposed development's construction.

All South Wakefield Street/ Site Access 3 unsignalized intersection approaches are projected to operate at a LOS B or better during the 2026 Background AM and PM peak hours. The 2026 AADT along South Wakefield Street is projected to exceed 4,000 VPD (based on recent AADT counts grown at 3% annually to 2026 and 30% of daily site trips on S Wakefield Street north of Site Access 3). Because of this, turn lanes were considered at Site Access 3. Per the NCDOT Nomograph (see **Appendix E**) and projected 2026 peak hour volumes, a 50-foot southbound left-turn lane (with appropriate taper) is recommended. As shown in **Table 5-2**, all intersection movements are projected to operate acceptably following the turn-lane's construction. No additional improvements are recommended at this intersection due to the proposed development's construction.

| | | AM PEAK HOUR | | PM PEAK HOUR | | | Turn | AM PEAK HOUR | PM PEAK HOUR |
|---|-----------------------|---------------------------------|-------|---------------------------------|---------|--|-------------------------|--|--|
| Intersection | Approach / Overall | Delay ¹ (sec/veh) | LOS 1 | Delay ¹ (sec/veh) | LOS 1 | Movement | Lane Storage (ft) | 95th Percentile Queue Length (ft)* | 95th Percentile Queue Length (ft)* |
| 1: S Wakefield Street & NC-97 | | | | | | EB Left | 125 | 10 | 11 |
| (Gannon Avenue) | Eastbound | 25.4 | C | 37.5 | D | EB Thru/Right | | 403 | #774 |
| Signalized | | 1000 C 500 | | 0.0000.07 | | EB Approach | | | |
| | | | | | | WB Left | 125 | 68 | 90 |
| | Westbound | 15.7 | в | 12.9 | в | WB Thru/Right | | 272 | 229 |
| | | | - | | - T | WB Approach | | 100 | |
| | 1 | C.2004 | 200 | 2016.04 | CVC . | NB Left/Thru/Right | | #264 | #234 |
| | Northbound | 33.9 | C | 53.0 | D | NB Approach | - | | |
| | | | | | | SB Left/Thru/Right | | 34 | 63 |
| | Southbound | 22.5 | C | 30.8 | С | SB Approach | | | |
| | Overall | 22.9 | с | 30.0 | с | Overall | | | |
| NC AC (Accordent Accordent Ac | Overall | 22.9 | 0 | 30.0 | C | | 200 | | |
| 2: NC-96 (Arendell Avenue) & NC- 97 (Gannon Avenue)) | | | | | | EB Left | 200 | 45 | 75 |
| Signalized | Eastbound | 33.6 | С | 42.2 | D | EB Thru | | 378 | #365 |
| or an and the second | | ABC 805 | | | | EB Right | 100 | 63 | 87 |
| | | | | | | EB Approach | | 1220 | - 1944 - 1944 |
| | 0.000.000 00 | 0.555 | | 265 | | WB Left | 350 | #268 | #309 |
| | Westbound | 25.9 | С | 29.9 | C | WB Thru/Right | | 168 | 356 |
| | | | | | | WB Approach | | (| |
| | | 111 | | | | NB Left | 125 | 106 | 85 |
| | Northbound | 41.6 | D | 42.1 | D | NB Thru/Right | | #370 | #481 |
| | 0.000353636005 | | - 55 | 0.928 | | NB Approach | | | |
| | Southbound | 29.4 | с | 29.1 | с | SB Left | 250 | 36 | #84 |
| | | | | | | SB Thru/Right | | 195 | 212 |
| | | 1.5565/5 | - | | | SB Approach | | | |
| | Overall | 32.6 | C | 35.7 | D | Overall | | | |
| 3: NC-96 (Arendell Avenue) & | Overall | 32.0 | - | 22.7 | | EB Left/Thru/Right | | 0.6 | 2.2 |
| Barbee Street | Eastbound | 15.4 | C | 23.7 | С | and the second sec | | 100.00 | |
| Unsignalized | | | - | | - | EB Approach | - | ** | ** |
| | Westbound | 14.7 | в | 17.7 | с | WB Left/Thru/Right | | 0.1 | 0.2 |
| | | | | | - | WB Approach | | | * |
| | Northbound | 1.1 | A | 1.1 | A | NB Left/Thru/Right | | 0.1 | 0.1 |
| | 1.00.00.000.000.000 | 8258 | 98 | 10105 | - 55 | NB Approach | | | |
| | Southbound | 0.2 | A | 0.1 | A | SB Left/Thru/Right | | 0 | 0 |
| | | | | | | SB Approach | | | |
| 4: NC-96 (Arendell Avenue) & Site | Eastbound | 12.9 | в | 15.7 | с | EB Left/Right | | 0.4 | 0.3 |
| Access 1 Unsignalized | | | | 9000 | 100 | EB Approach | | | |
| onsynaaco | Northbound | 0.1 | A | 0.3 | A | NB Left/Thru | | 0 | 0 |
| | The area and | × | 12 | | 10 | NB Approach | | | ** |
| | | | | | | SB Thru/Right | | 0 | 0 |
| | Southbound | 0.0 | A | 0.0 | Α | SB Approach | | | |
| 5: NC-96 (Arendell Avenue) & Site | (La restricted) | 12.5 | | 10000 | 1 | EB Left/Right | 1 | 0.4 | 0.3 |
| Access 2 | Eastbound | 12.1 | В | 14.7 | в | EB Approach | | | ** |
| Unsignalized | | | | | | NB Left/Thru | | 0 | 0.1 |
| | Northbound | 0.1 | A | 0.6 | A | NB Approach | | | |
| | 14. 10409 - 80 | 0.93 | 0.2 | | | SB Thru/Right | | 0 | 0 |
| | Southbound | 0.0 | A | 0.0 | A | SB Approach | - | | |
| 6: NC-96 (Arendell Avenue) & Perry | | | | - | | WB Left/Right | | 0.5 | 0.3 |
| Curtis Road | Westbound | 10.8 | в | 10.5 | В | And the College of the Second Second | | 123223 | 0.5.35 |
| Unsignalized | | | | | | WB Approach | | | |
| 1. 2424 THE CAR. | Northbound | 0.0 | A | 0.0 | A | NB Thru/Right | | 0 | 0 |
| | | | | | | NB Approach | | | |
| | Southbound | 1.6 | A | 2.2 | A | SB Left/Thru | | 0.1 | 0.3 |
| | | | 10.5 | | | SB Approach | | | |
| 7: S Wakefield Street & Site Access | Westbound | 10.1 | в | 9.9 | A | WB Left/Right | | 0.3 | 0.2 |
| 3 Unsignalized | | | 1 | | <u></u> | WB Approach | | 1.22 | |
| unagnalizeu | Northbound | 0.0 | | 0.0 | A | NB Thru/Right | | 0 | 0 |
| | Northbound | 0.0 | A | 0.0 | A | NB Approach | | | ** |
| | Parties | 832 | 13 | 100 | 395 | SB Left/Thru | | 0 | 0.1 |
| | Southbound | 1.1 | A | 1.8 | A | SB Approach | | | |

Table 5-1: Intersection Level of Service and Delay Summary2026 Build Traffic Volumes

¹ Overall intersection LOS and delay not reported for TWSC intersections.

* - 95th percentile queues for unsignalized intersections reported in number of vehicles.

- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles

| | | AM PEAK HOUR | | PM PEAK HOUR | | | | AM PEAK HOUR | PM PEAK HOUR |
|-------------------------------------|-----------------------|---------------------------------|-------|---------------------------------|--|---------------|------------------------------|--|--|
| Intersection | Approach / Overall | Delay ¹ (sec/veh) | LOS 1 | Delay ¹ (sec/veh) | LOS 1 | Movement | Turn Lane Storage (ft) | 95th Percentile Queue Length (ft)* | 95th Percentile Queue Length (ft)* |
| 4: NC-96 (Arendell Avenue) & Site | Eastbound | 12.7 | в | 15.3 | с | EB Left/Right | | 0.4 | 0.3 |
| Access 1 Unsignalized | Lastbound | 12.7 | D. | 15.5 | 100 | EB Approach | | - | |
| Unsignalized | Northbound | 0.1 | A | 0.3 | A | NB Left/Thru | | 0 | 0 |
| | Northbodina | 0.1 | - 1 | 0.5 | - | NB Approach | | 1977 | - |
| | - | Contemporte | | | 0.0 A SB Thru SB Right SB Approach | | 0 | 0 | |
| | Southbound | 0.0 | A | 0.0 | | SB Right | 50 | 0 | 0 |
| | | | | | | SB Approach | | 122 | 2 |
| 5: NC-96 (Arendell Avenue) & Site | Eastbound | 12.0 | В | 14.3 | В | EB Left/Right | | 0.4 | 0.3 |
| Access 2 Unsignalized | | | | | | EB Approach | | 1000 | 0.000 |
| Unsignalized | Northbound | 0.1 | A | 0.6 | A | NB Left/Thru | | 0 | 0.1 |
| | | 0.1 | - | | | NB Approach | | - | |
| | | 0.0 | A | 0.0 | | SB Thru | | 0 | 0 |
| | Southbound | | | | Α | SB Right | 50 | 0 | 0 |
| | | | | | | SB Approach | | 1922 | |
| 7: S Wakefield Street & Site Access | Westbound | 10.1 | В | 9,9 | A | WB Left/Right | | 0.3 | 0.2 |
| 3 <i>Unsignalized</i> | Westbound | 10.1 | | 3.5 | | WB Approach | | 1 | |
| Unsignalized | Northbound | 0.0 | A | 0.0 | A | NB Thru/Right | | 0 | 0 |
| | norabound | 0.0 | - | 0.0 | ~ | NB Approach | | - | |
| | | | | | | SB Left | 50 | 0 | 0.1 |
| | Southbound | 1.1 | A | 1.8 | Α | SB Thru | | 0 | 0 |
| | | | | | | SB Approach | | - | - |

Table 5-2: Intersection Level of Service and Delay Summary2026 Build + Improvement Traffic Volumes

¹ Overall intersection LOS and delay not reported for TWSC intersections.

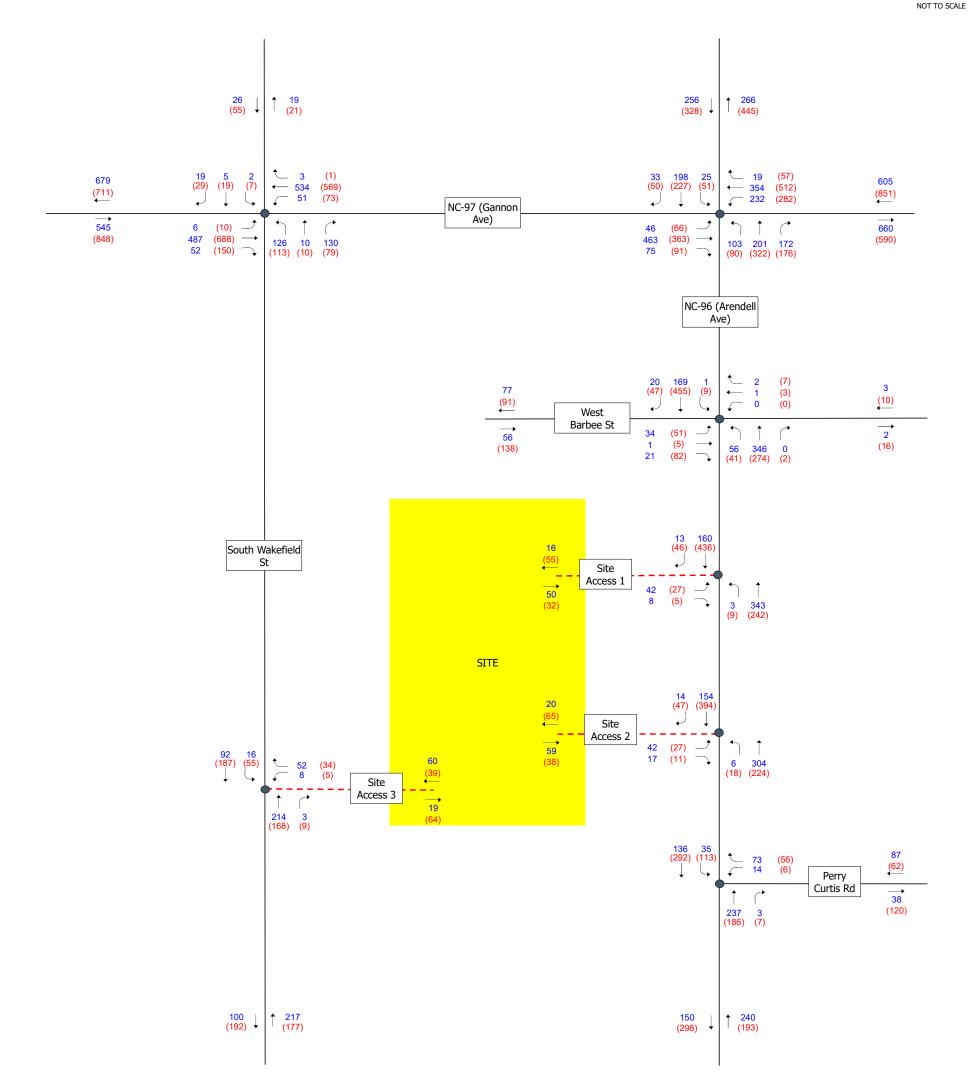
* - 95th percentile queues for unsignalized intersections reported in number of vehicles.

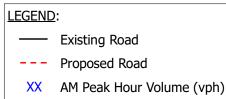
- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles





A N A

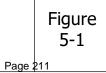




(XX) PM Peak Hour Volume (vph)



Zebulon South Traffic Impact Analysis 2026 Build Traffic Volumes



6 CONCLUSIONS AND RECOMMENDATIONS

Capacity analyses were performed for the following scenarios:

- 2022 Existing traffic volumes
- 2026 Background traffic volumes
- 2026 Build traffic volumes (Background + site trips)

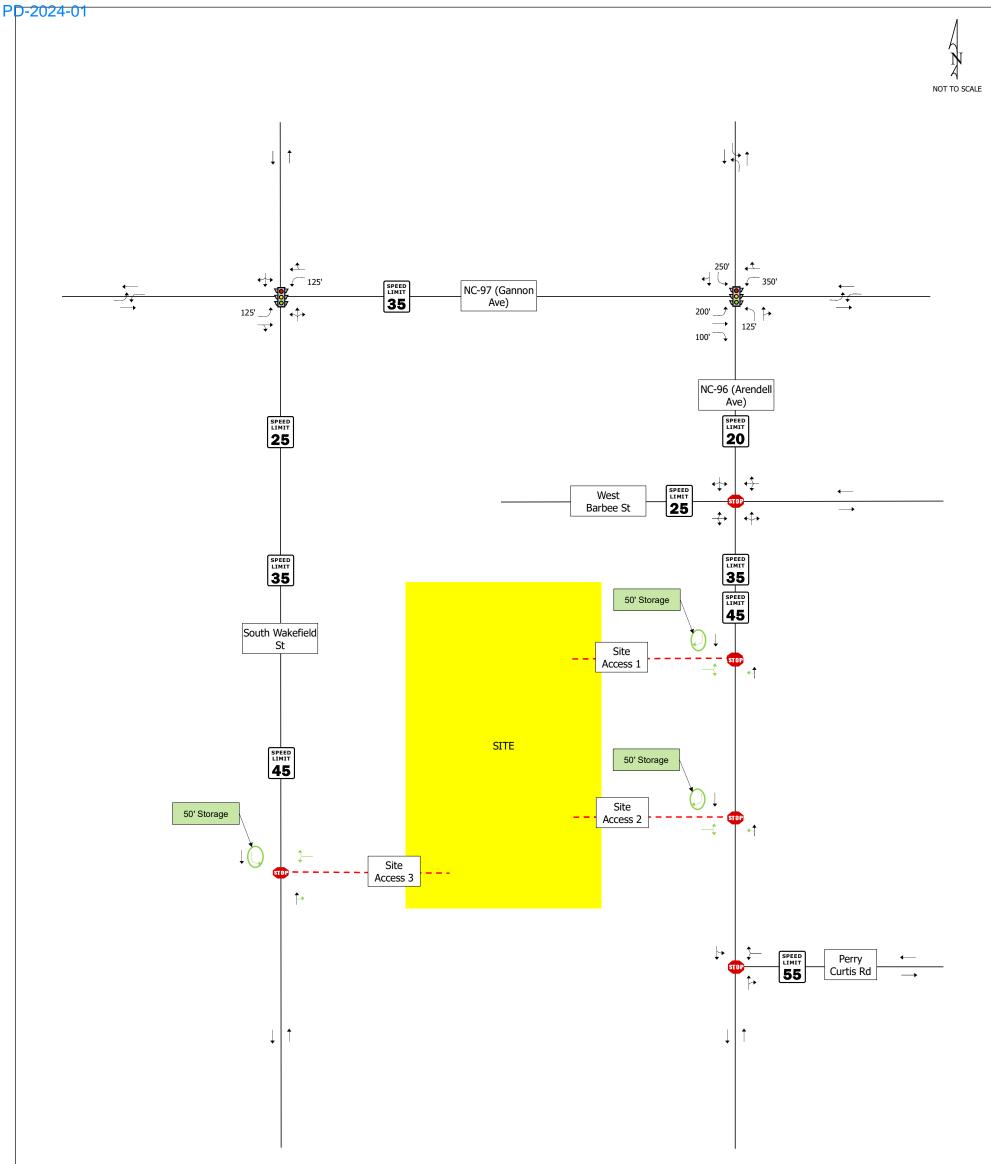
In closing, the following improvements (see **Figure 6-1**) are recommended in conjunction with the construction of the proposed development*:

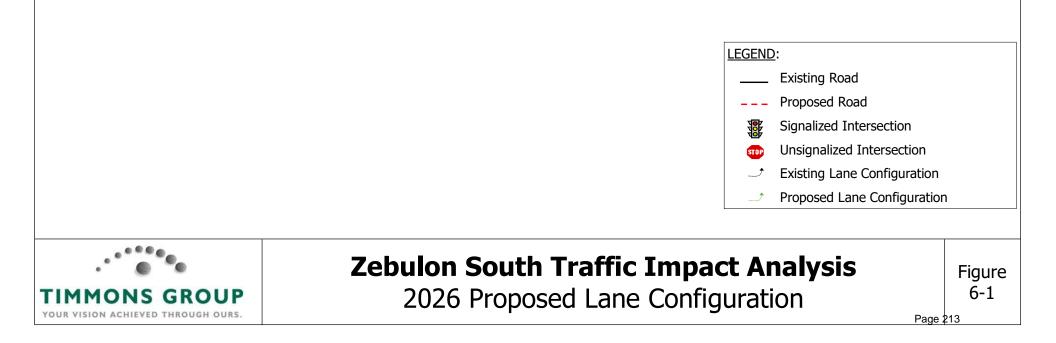
- NC-97 / South Wakefield Street
 - o None
- NC-97 / NC-96
 - o None
- NC-96 / West Barbee Street
 - o None
- NC-96 / Site Access 1
 - 50-foot southbound right-turn lane (with appropriate taper)
 - 100-feet IPS (Internal Protected Stem) along Site Access 1**
- NC-96 / Site Access 2
 - 50-foot southbound right-turn lane (with appropriate taper)
 - 100-feet IPS along Site Access 2**
- NC-96 / Perry Curtis Road
 - o None
- South Wakefield Street / Site Access 3
 - 50-foot southbound left-turn lane (with appropriate taper)
 - 100-feet IPS along Site Access 3**

* NCDOT comments provided in July 2022 (**see Appendix F)** required additional improvements at the intersections of NC-96 / Site Access 1 and NC-96 / Site Access 2. These improvements were not analyzed or included to represent a more conservative scenario.

** IPS requirements are defined in the NCDOT's Driveway Manual.







Appendix A – Scoping Information

Attachment 7 PD-2024-01

| From: | Brennan, Sean P |
|--------------|--|
| To: | Jeff Hochanadel; Meade Bradshaw; Warren, Jeremy L; Ishak, Doumit Y; Bunting, Clarence B; Lineberger, |
| | Nicholas C; Walker, Braden M |
| Cc: | Cliff Lawson; Beth Blackmon |
| Subject: | Re: [External] Zebulon South TIA Scoping |
| Date: | Friday, April 1, 2022 10:20:18 AM |
| Attachments: | 49084-331C-SPLAYO3-Layout1.pdf |

CAUTION: This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.

Jeff,

Isn't there a 3rd site access that connects to S Wakefield St? So you might want to include S Wakefield and W Gannon Ave as well.

I have some concerns with the site access to S Arendell located across from Perry Curtis Rd. The geometry of that intersection makes adding a 4th leg difficult.

Regards,

Sean Brennan, PE Senior Assistant District Engineer Division 5/District 1 Department of Transportation

919-733-3213 office 919-715-5778 fax <u>spbrennan@ncdot.gov</u>

4009 District Drive (Physical Address) Raleigh, NC 27607

1575 Mail Service Center (Mailing Address) Raleigh, NC 27699-1575

cid:image001.png@01D10DA4.5CC88DA0

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?

From: Jeff Hochanadel <Jeff.Hochanadel@timmons.com>

Sent: Thursday, March 31, 2022 4:51 PM

To: Meade Bradshaw <Mbradshaw@townofzebulon.org>; Brennan, Sean P <spbrennan@ncdot.gov>; Warren, Jeremy L <jlwarren@ncdot.gov>; Ishak, Doumit Y <dishak@ncdot.gov>; Bunting, Clarence B <cbunting@ncdot.gov>; Lineberger, Nicholas C <nclineberger@ncdot.gov>; Walker, Braden M <bmwalker1@ncdot.gov> **Cc:** Cliff Lawson <cliff.lawson@timmons.com>; Beth Blackmon <Beth.Blackmon@timmons.com> **Subject:** [External] Zebulon South TIA Scoping

CAUTION: External email. Do not click links or open attachments unless you verify. Send all suspicious email as an attachment to <u>Report Spam.</u>

All,

Timmons Group would like to scope a TIA for the subject residential development in Zebulon, NC. Per the attached conceptual site plan, the proposed development will consist of 148 townhomes and 166 single-family lots to be constructed off NC-96 (Arendell Avenue). I am ok scoping this via email or we could set up a virtual meeting to discuss the subject project.

Our scoping assumptions include the following:

- Study Area Intersections:
 - NC-97 (Gannon Avenue) / NC-96 (Arendell Avenue)
 - W Barbee Street / NC-96 (Arendell Avenue)
 - Site Access 1 / NC-96 (Arendell Avenue)
 - Site Access 2 / NC-96 (Arendell Avenue)
 - Perry Curtis Road / NC-96 (Arendell Avenue)
- Growth Rate:
 - 3%
- Approved Area Developments:
 - <mark>None</mark>
- STIP Projects:
 - None
- Build-Out Years:
 - Phase 1 2024
 - Phase 2 2026

Once we have determined the final project scope, I will be happy to submit the NCDOT TIA Scoping Checklist (as needed).

Please do not hesitate to contact me with any questions.

Thanks! Jeff

Jeff Hochanadel, PE, PTOE

Principal | North Carolina Transportation Group Leader **TIMMONS GROUP** | www.timmons.com 5410 Trinity Rd, Suite 102 | Raleigh, NC 27607 Office: 919.866.4511 | Fax: 919.859.5663 Cell: 919.426.8405 jeff.hochanadel@timmons.com Your Vision Achieved Through Ours **To send me files greater than 20MB click here**

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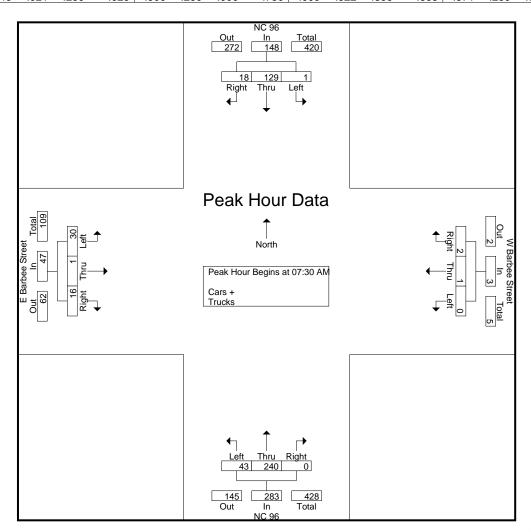
Appendix B – Traffic Counts



| | | | | | | G | roups F | rinted- C | ars + - 1 | Frucks | | | | | | | |
|-----------------|-------|-------|-------|------------|-------|---------|---------|------------|-----------|--------|-------|------------|-------|---------|---------|------------|------------|
| | | NC | 96 | | ١ | N Barbe | e Stre | et | | NC | C 96 | | | E Barbe | e Stree | et | |
| | | South | bound | | | West | bound | | | North | bound | | | East | bound | | |
| Start Time | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Int. Total |
| 07:00 AM | 7 | 15 | 0 | 22 | 1 | 0 | 0 | 1 | 1 | 47 | 7 | 55 | 7 | 1 | 5 | 13 | 91 |
| 07:15 AM | 3 | 18 | 1 | 22 | 1 | 1 | 0 | 2 | 1 | 67 | 11 | 79 | 2 | 0 | 5 | 7 | 110 |
| 07:30 AM | 5 | 27 | 0 | 32 | 0 | 0 | 0 | 0 | 0 | 62 | 12 | 74 | 2 | 1 | 5 | 8 | 114 |
| 07:45 AM | 7 | 33 | 0 | 40 | 1 | 0 | 0 | 1 | 0 | 73 | 9 | 82 | 6 | 0 | 10 | 16 | 139 |
| Total | 22 | 93 | 1 | 116 | 3 | 1 | 0 | 4 | 2 | 249 | 39 | 290 | 17 | 2 | 25 | 44 | 454 |
| | | | | | | | | | | | | | | | | | |
| 08:00 AM | 2 | 34 | 0 | 36 | 1 | 0 | 0 | 1 | 0 | 53 | 12 | 65 | 7 | 0 | 5 | 12 | 114 |
| 08:15 AM | 4 | 35 | 1 | 40 | 0 | 1 | 0 | 1 | 0 | 52 | 10 | 62 | 1 | 0 | 10 | 11 | 114 |
| 08:30 AM | 5 | 26 | 1 | 32 | 1 | 0 | 0 | 1 | 0 | 52 | 7 | 59 | 4 | 0 | 10 | 14 | 106 |
| 08:45 AM | 8 | 26 | 0 | 34 | 0 | 0 | 0 | 0 | 0 | 50 | 14 | 64 | 6 | 1 | 6 | 13 | 111 |
| Total | 19 | 121 | 2 | 142 | 2 | 1 | 0 | 3 | 0 | 207 | 43 | 250 | 18 | 1 | 31 | 50 | 445 |
| | | | | | | | | | | | | | | | | | |
| Grand Total | 41 | 214 | 3 | 258 | 5 | 2 | 0 | 7 | 2 | 456 | 82 | 540 | 35 | 3 | 56 | 94 | 899 |
| Apprch % | 15.9 | 82.9 | 1.2 | | 71.4 | 28.6 | 0 | | 0.4 | 84.4 | 15.2 | | 37.2 | 3.2 | 59.6 | | |
| Total % | 4.6 | 23.8 | 0.3 | 28.7 | 0.6 | 0.2 | 0 | 0.8 | 0.2 | 50.7 | 9.1 | 60.1 | 3.9 | 0.3 | 6.2 | 10.5 | |
| Cars + | 37 | 195 | 3 | 235 | 5 | 2 | 0 | 7 | 2 | 436 | 81 | 519 | 34 | 3 | 53 | 90 | 851 |
| <u>% Cars +</u> | 90.2 | 91.1 | 100 | 91.1 | 100 | 100 | 0 | 100 | 100 | 95.6 | 98.8 | 96.1 | 97.1 | 100 | 94.6 | 95.7 | 94.7 |
| Trucks | 4 | 19 | 0 | 23 | 0 | 0 | 0 | 0 | 0 | 20 | 1 | 21 | 1 | 0 | 3 | 4 | 48 |
| % Trucks | 9.8 | 8.9 | 0 | 8.9 | 0 | 0 | 0 | 0 | 0 | 4.4 | 1.2 | 3.9 | 2.9 | 0 | 5.4 | 4.3 | 5.3 |



| | | NC | ; 96 | | \ | N Barbe | e Stree | et | | NC | 96 | | | E Barbe | e Stree | et | |
|-----------------|------------|----------|--------|------------|---------|----------|---------|------------|-------|-------|-------|------------|-------|---------|---------|------------|------------|
| | | South | bound | | | West | bound | | | North | bound | | | East | bound | | |
| Start Time | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Int. Total |
| Peak Hour Ana | alysis Fro | om 07:0 | 0 AM t | o 08:45 A | M - Pea | k 1 of 1 | | | | | | | | | | | |
| Peak Hour for I | Entire In | tersecti | on Beg | ins at 07: | 30 AM | | | | | | | | | | | | |
| 07:30 AM | 5 | 27 | 0 | 32 | 0 | 0 | 0 | 0 | 0 | 62 | 12 | 74 | 2 | 1 | 5 | 8 | 114 |
| 07:45 AM | 7 | 33 | 0 | 40 | 1 | 0 | 0 | 1 | 0 | 73 | 9 | 82 | 6 | 0 | 10 | 16 | 139 |
| 08:00 AM | 2 | 34 | 0 | 36 | 1 | 0 | 0 | 1 | 0 | 53 | 12 | 65 | 7 | 0 | 5 | 12 | 114 |
| 08:15 AM | 4 | 35 | 1 | 40 | 0 | 1 | 0 | 1 | 0 | 52 | 10 | 62 | 1 | 0 | 10 | 11 | 114 |
| Total Volume | 18 | 129 | 1 | 148 | 2 | 1 | 0 | 3 | 0 | 240 | 43 | 283 | 16 | 1 | 30 | 47 | 481 |
| % App. Total | 12.2 | 87.2 | 0.7 | | 66.7 | 33.3 | 0 | | 0 | 84.8 | 15.2 | | 34 | 2.1 | 63.8 | | |
| PHF | .643 | .921 | .250 | .925 | .500 | .250 | .000 | .750 | .000 | .822 | .896 | .863 | .571 | .250 | .750 | .734 | .865 |

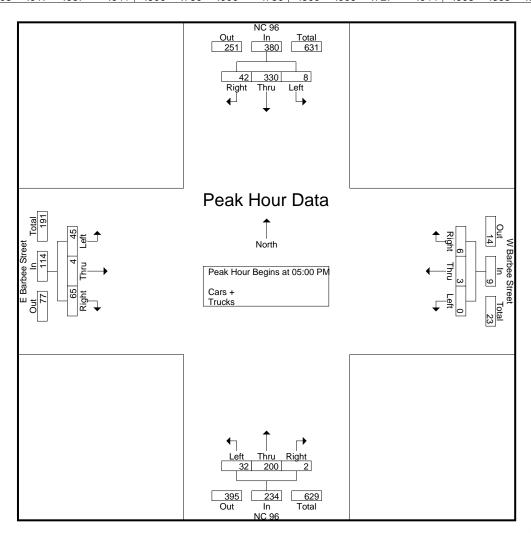




| | | | | | | G | roups F | Printed- C | ars + - 1 | Frucks | | | | | | | |
|-------------|-------|-------|-------|------------|--------|---------|---------|------------|-----------|--------|-------|------------|-------|---------|---------|------------|------------|
| | | NC | 96 | | ۱ ۱ | N Barbe | e Stre | et | | NC | 96 | | | E Barbe | e Stree | et | |
| | | South | bound | | | West | bound | | | North | bound | | | East | ound | | |
| Start Time | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Int. Total |
| 04:00 PM | 11 | 71 | 1 | 83 | 0 | 1 | 1 | 2 | 0 | 47 | 6 | 53 | 13 | 1 | 13 | 27 | 165 |
| 04:15 PM | 14 | 57 | 2 | 73 | 0 | 0 | 0 | 0 | 1 | 52 | 14 | 67 | 14 | 0 | 16 | 30 | 170 |
| 04:30 PM | 12 | 72 | 0 | 84 | 1 | 1 | 0 | 2 | 0 | 49 | 6 | 55 | 22 | 2 | 12 | 36 | 177 |
| 04:45 PM | 14 | 75 | 0 | 89 | 2 | 2 | 0 | 4 | 0 | 44 | 12 | 56 | 14 | 2 | 12 | 28 | 177 |
| Total | 51 | 275 | 3 | 329 | 3 | 4 | 1 | 8 | 1 | 192 | 38 | 231 | 63 | 5 | 53 | 121 | 689 |
| | | | | | | | | | | | | | | | | | |
| 05:00 PM | 12 | 80 | 3 | 95 | 0 | 1 | 0 | 1 | 0 | 42 | 11 | 53 | 15 | 0 | 7 | 22 | 171 |
| 05:15 PM | 9 | 90 | 1 | 100 | 2 | 1 | 0 | 3 | 0 | 55 | 7 | 62 | 17 | 3 | 14 | 34 | 199 |
| 05:30 PM | 8 | 75 | 1 | 84 | 1 | 1 | 0 | 2 | 1 | 52 | 7 | 60 | 18 | 1 | 11 | 30 | 176 |
| 05:45 PM | 13 | 85 | 3 | 101 | 3 | 0 | 0 | 3 | 1 | 51 | 7 | 59 | 15 | 0 | 13 | 28 | 191 |
| Total | 42 | 330 | 8 | 380 | 6 | 3 | 0 | 9 | 2 | 200 | 32 | 234 | 65 | 4 | 45 | 114 | 737 |
| | | | | | | | | | | | | | | | | | |
| Grand Total | 93 | 605 | 11 | 709 | 9 | 7 | 1 | 17 | 3 | 392 | 70 | 465 | 128 | 9 | 98 | 235 | 1426 |
| Apprch % | 13.1 | 85.3 | 1.6 | | 52.9 | 41.2 | 5.9 | | 0.6 | 84.3 | 15.1 | | 54.5 | 3.8 | 41.7 | | |
| Total % | 6.5 | 42.4 | 0.8 | 49.7 | 0.6 | 0.5 | 0.1 | 1.2 | 0.2 | 27.5 | 4.9 | 32.6 | 9 | 0.6 | 6.9 | 16.5 | |
| Cars + | 87 | 593 | 11 | 691 | 9 | 7 | 1 | 17 | 3 | 379 | 66 | 448 | 127 | 9 | 96 | 232 | 1388 |
| % Cars + | 93.5 | 98 | 100 | 97.5 | 100 | 100 | 100 | 100 | 100 | 96.7 | 94.3 | 96.3 | 99.2 | 100 | 98 | 98.7 | 97.3 |
| Trucks | 6 | 12 | 0 | 18 | 0 | 0 | 0 | 0 | 0 | 13 | 4 | 17 | 1 | 0 | 2 | 3 | 38 |
| % Trucks | 6.5 | 2 | 0 | 2.5 | 0 | 0 | 0 | 0 | 0 | 3.3 | 5.7 | 3.7 | 0.8 | 0 | 2 | 1.3 | 2.7 |



| | | NC | 96 | | ۱ | N Barbe | e Stree | et | | NC | 96 | | | E Barbe | e Stree | et 🔰 | |
|-----------------|-----------|----------|--------|------------|---------|----------|---------|------------|-------|-------|-------|------------|-------|---------|---------|------------|------------|
| | | South | bound | | | West | bound | | | North | bound | | | Eastl | bound | | |
| Start Time | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Int. Total |
| Peak Hour Ana | lysis Fr | om 04:0 | 0 PM t | o 05:45 F | M - Pea | k 1 of 1 | | | | | | | | | | | |
| Peak Hour for I | Entire In | tersecti | on Beg | ins at 05: | 00 PM | | | | | | | | | | | | |
| 05:00 PM | 12 | 80 | 3 | 95 | 0 | 1 | 0 | 1 | 0 | 42 | 11 | 53 | 15 | 0 | 7 | 22 | 171 |
| 05:15 PM | 9 | 90 | 1 | 100 | 2 | 1 | 0 | 3 | 0 | 55 | 7 | 62 | 17 | 3 | 14 | 34 | 199 |
| 05:30 PM | 8 | 75 | 1 | 84 | 1 | 1 | 0 | 2 | 1 | 52 | 7 | 60 | 18 | 1 | 11 | 30 | 176 |
| 05:45 PM | 13 | 85 | 3 | 101 | 3 | 0 | 0 | 3 | 1 | 51 | 7 | 59 | 15 | 0 | 13 | 28 | 191 |
| Total Volume | 42 | 330 | 8 | 380 | 6 | 3 | 0 | 9 | 2 | 200 | 32 | 234 | 65 | 4 | 45 | 114 | 737 |
| % App. Total | 11.1 | 86.8 | 2.1 | | 66.7 | 33.3 | 0 | | 0.9 | 85.5 | 13.7 | | 57 | 3.5 | 39.5 | | |
| PHF | .808. | .917 | .667 | .941 | .500 | .750 | .000 | .750 | .500 | .909 | .727 | .944 | .903 | .333 | .804 | .838 | .926 |

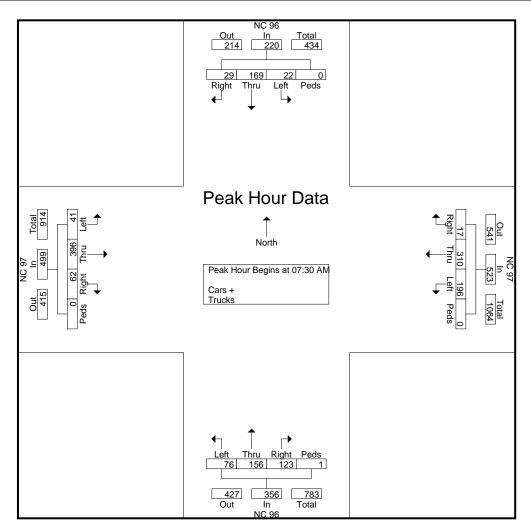




| | | | | | | | | Gro | ups Pr | rinted- (| Cars + | - Truc | ks | | | | | | | | |
|-------------|-------|------|-------|------|------------|-------|------|-------|--------|------------|--------|--------|--------|------|------------|-------|------|--------|------|------------|------------|
| | | | NC 96 | 6 | | | | NC 97 | 7 | | | | NC 96 | 6 | | | | NC 97 | 7 | | |
| | | So | uthbo | und | | | W | estbo | und | | | N | orthbo | und | | | E | astbou | ind | | |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| 07:00 AM | 4 | 32 | 6 | 1 | 43 | 1 | 38 | 28 | 0 | 67 | 22 | 23 | 4 | 0 | 49 | 9 | 59 | 3 | 0 | 71 | 230 |
| 07:15 AM | 9 | 59 | 7 | 0 | 75 | 1 | 56 | 42 | 0 | 99 | 30 | 21 | 8 | 0 | 59 | 16 | 62 | 7 | 0 | 85 | 318 |
| 07:30 AM | 7 | 47 | 9 | 0 | 63 | 6 | 83 | 39 | 0 | 128 | 25 | 36 | 16 | 0 | 77 | 22 | 94 | 7 | 0 | 123 | 391 |
| 07:45 AM | 9 | 42 | 4 | 0 | 55 | 4 | 94 | 52 | 0 | 150 | 46 | 35 | 26 | 0 | 107 | 11 | 111 | 9 | 0 | 131 | 443 |
| Total | 29 | 180 | 26 | 1 | 236 | 12 | 271 | 161 | 0 | 444 | 123 | 115 | 54 | 0 | 292 | 58 | 326 | 26 | 0 | 410 | 1382 |
| 1 | | | | | | | | | | | | | | | | | | | | | I. |
| 08:00 AM | 11 | 37 | 5 | 0 | 53 | 3 | 83 | 56 | 0 | 142 | 19 | 41 | 19 | 1 | 80 | 16 | 109 | 10 | 0 | 135 | 410 |
| 08:15 AM | 2 | 43 | 4 | 0 | 49 | 4 | 50 | 49 | 0 | 103 | 33 | 44 | 15 | 0 | 92 | 13 | 82 | 15 | 0 | 110 | 354 |
| 08:30 AM | 7 | 44 | 5 | 0 | 56 | 8 | 35 | 42 | 0 | 85 | 40 | 41 | 9 | 0 | 90 | 12 | 46 | 9 | 0 | 67 | 298 |
| 08:45 AM | 7 | 46 | 10 | 0 | 63 | 9 | 55 | 44 | 0 | 108 | 39 | 36 | 6 | 0 | 81 | 11 | 57 | 1 | 0 | 69 | 321 |
| Total | 27 | 170 | 24 | 0 | 221 | 24 | 223 | 191 | 0 | 438 | 131 | 162 | 49 | 1 | 343 | 52 | 294 | 35 | 0 | 381 | 1383 |
| i | | | | | | | | | | | | | | | | | | | | | i. |
| Grand Total | 56 | 350 | 50 | 1 | 457 | 36 | 494 | 352 | 0 | 882 | 254 | 277 | 103 | 1 | 635 | 110 | 620 | 61 | 0 | 791 | 2765 |
| Apprch % | 12.3 | 76.6 | 10.9 | 0.2 | | 4.1 | 56 | 39.9 | 0 | | 40 | 43.6 | 16.2 | 0.2 | | 13.9 | 78.4 | 7.7 | 0 | | |
| Total % | 2 | 12.7 | 1.8 | 0 | 16.5 | 1.3 | 17.9 | 12.7 | 0 | 31.9 | 9.2 | 10 | 3.7 | 0 | 23 | 4 | 22.4 | 2.2 | 0 | 28.6 | |
| Cars + | 55 | 336 | 48 | 1 | 440 | 34 | 482 | 341 | 0 | 857 | 244 | 260 | 100 | 1 | 605 | 104 | 599 | 60 | 0 | 763 | 2665 |
| % Cars + | 98.2 | 96 | 96 | 100 | 96.3 | 94.4 | 97.6 | 96.9 | 0 | 97.2 | 96.1 | 93.9 | 97.1 | 100 | 95.3 | 94.5 | 96.6 | 98.4 | 0 | 96.5 | 96.4 |
| Trucks | 1 | 14 | 2 | 0 | 17 | 2 | 12 | 11 | 0 | 25 | 10 | 17 | 3 | 0 | 30 | 6 | 21 | 1 | 0 | 28 | 100 |
| % Trucks | 1.8 | 4 | 4 | 0 | 3.7 | 5.6 | 2.4 | 3.1 | 0 | 2.8 | 3.9 | 6.1 | 2.9 | 0 | 4.7 | 5.5 | 3.4 | 1.6 | 0 | 3.5 | 3.6 |



| | | | NC 96 | 6 | | | | NC 9 | 7 | | | | NC 96 | 3 | | | | NC 9 | 7 | |] |
|---------------|----------|---------|---------|---------|------------|--------|--------|-------|------|------------|-------|------|--------|------|------------|-------|------|--------|------|------------|------------|
| | | Sc | outhbo | und | | | W | estbo | und | | | N | orthbo | und | | | E | astbou | und | | |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Peak Hour A | nalysi | s From | n 07:00 | O AM to | o 08:45 | AM - | Peak 1 | of 1 | | | | | | | | | | | | | |
| Peak Hour for | or Entii | re Inte | rsectio | on Beg | ins at 0 | 7:30 A | M | | | | | | | | | | | | | | |
| 07:30 AM | 7 | 47 | 9 | 0 | 63 | 6 | 83 | 39 | 0 | 128 | 25 | 36 | 16 | 0 | 77 | 22 | 94 | 7 | 0 | 123 | 391 |
| 07:45 AM | 9 | 42 | 4 | 0 | 55 | 4 | 94 | 52 | 0 | 150 | 46 | 35 | 26 | 0 | 107 | 11 | 111 | 9 | 0 | 131 | 443 |
| 08:00 AM | 11 | 37 | 5 | 0 | 53 | 3 | 83 | 56 | 0 | 142 | 19 | 41 | 19 | 1 | 80 | 16 | 109 | 10 | 0 | 135 | 410 |
| 08:15 AM | 2 | 43 | 4 | 0 | 49 | 4 | 50 | 49 | 0 | 103 | 33 | 44 | 15 | 0 | 92 | 13 | 82 | 15 | 0 | 110 | 354 |
| Total Volume | 29 | 169 | 22 | 0 | 220 | 17 | 310 | 196 | 0 | 523 | 123 | 156 | 76 | 1 | 356 | 62 | 396 | 41 | 0 | 499 | 1598 |
| % App. Total | 13.2 | 76.8 | 10 | 0 | | 3.3 | 59.3 | 37.5 | 0 | | 34.6 | 43.8 | 21.3 | 0.3 | | 12.4 | 79.4 | 8.2 | 0 | | |
| PHF | .659 | .899 | .611 | .000 | .873 | .708 | .824 | .875 | .000 | .872 | .668 | .886 | .731 | .250 | .832 | .705 | .892 | .683 | .000 | .924 | .902 |

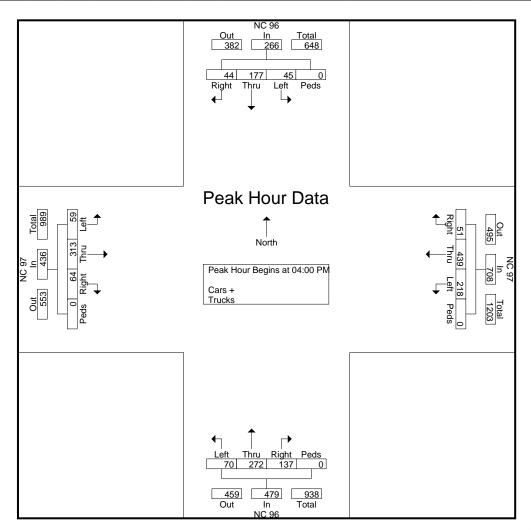




| | | | | | | | | Gro | ups Pr | rinted- C | Cars + | - Truc | ks | | | | | | | | |
|-------------|-------|------|--------|------|------------|-------|------|-------|--------|------------|--------|--------|--------|------|------------|-------|------|--------|------|------------|------------|
| | | | NC 96 | ; | | | | NC 97 | 7 | | | | NC 96 | 3 | | | | NC 97 | 7 | | |
| | | Sc | uthbou | und | | | W | estbo | und | | | No | orthbo | und | | | E | astbou | Ind | | |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| 04:00 PM | 15 | 56 | 17 | 0 | 88 | 10 | 115 | 61 | 0 | 186 | 33 | 67 | 17 | 0 | 117 | 12 | 84 | 17 | 0 | 113 | 504 |
| 04:15 PM | 7 | 38 | 7 | 0 | 52 | 10 | 98 | 40 | 0 | 148 | 34 | 64 | 25 | 0 | 123 | 19 | 93 | 15 | 0 | 127 | 450 |
| 04:30 PM | 11 | 44 | 8 | 0 | 63 | 19 | 117 | 59 | 0 | 195 | 36 | 75 | 14 | 0 | 125 | 16 | 69 | 9 | 0 | 94 | 477 |
| 04:45 PM | 11 | 39 | 13 | 0 | 63 | 12 | 109 | 58 | 0 | 179 | 34 | 66 | 14 | 0 | 114 | 17 | 67 | 18 | 0 | 102 | 458 |
| Total | 44 | 177 | 45 | 0 | 266 | 51 | 439 | 218 | 0 | 708 | 137 | 272 | 70 | 0 | 479 | 64 | 313 | 59 | 0 | 436 | 1889 |
| | | | | | | | | | | | | | | | | | | | | | |
| 05:00 PM | 8 | 54 | 13 | 0 | 75 | 9 | 117 | 64 | 0 | 190 | 37 | 57 | 17 | 0 | 111 | 15 | 70 | 16 | 0 | 101 | 477 |
| 05:15 PM | 12 | 47 | 9 | 0 | 68 | 9 | 111 | 56 | 0 | 176 | 29 | 56 | 13 | 1 | 99 | 21 | 87 | 11 | 0 | 119 | 462 |
| 05:30 PM | 11 | 59 | 9 | 0 | 79 | 16 | 94 | 63 | 0 | 173 | 37 | 61 | 16 | 0 | 114 | 9 | 61 | 15 | 0 | 85 | 451 |
| 05:45 PM | 12 | 48 | 8 | 0 | 68 | 10 | 106 | 48 | 0 | 164 | 52 | 62 | 13 | 0 | 127 | 17 | 60 | 16 | 0 | 93 | 452 |
| Total | 43 | 208 | 39 | 0 | 290 | 44 | 428 | 231 | 0 | 703 | 155 | 236 | 59 | 1 | 451 | 62 | 278 | 58 | 0 | 398 | 1842 |
| | | | | | | | | | | | | | | | | | | | | | |
| Grand Total | 87 | 385 | 84 | 0 | 556 | 95 | 867 | 449 | 0 | 1411 | 292 | 508 | 129 | 1 | 930 | 126 | 591 | 117 | 0 | 834 | 3731 |
| Apprch % | 15.6 | 69.2 | 15.1 | 0 | | 6.7 | 61.4 | 31.8 | 0 | | 31.4 | 54.6 | 13.9 | 0.1 | | 15.1 | 70.9 | 14 | 0 | | |
| Total % | 2.3 | 10.3 | 2.3 | 0 | 14.9 | 2.5 | 23.2 | 12 | 0 | 37.8 | 7.8 | 13.6 | 3.5 | 0 | 24.9 | 3.4 | 15.8 | 3.1 | 0 | 22.4 | |
| Cars + | 86 | 384 | 82 | 0 | 552 | 94 | 858 | 442 | 0 | 1394 | 284 | 497 | 124 | 1 | 906 | 126 | 581 | 117 | 0 | 824 | 3676 |
| % Cars + | 98.9 | 99.7 | 97.6 | 0 | 99.3 | 98.9 | 99 | 98.4 | 0 | 98.8 | 97.3 | 97.8 | 96.1 | 100 | 97.4 | 100 | 98.3 | 100 | 0 | 98.8 | 98.5 |
| Trucks | 1 | 1 | 2 | 0 | 4 | 1 | 9 | 7 | 0 | 17 | 8 | 11 | 5 | 0 | 24 | 0 | 10 | 0 | 0 | 10 | 55 |
| % Trucks | 1.1 | 0.3 | 2.4 | 0 | 0.7 | 1.1 | 1 | 1.6 | 0 | 1.2 | 2.7 | 2.2 | 3.9 | 0 | 2.6 | 0 | 1.7 | 0 | 0 | 1.2 | 1.5 |



| | | | NC 96 | 3 | | | | NC 97 | 7 | | | | NC 96 | 3 | | | | NC 97 | 7 | | |
|---------------|---------|---------|---------|--------|------------|--------|--------|--------|------|------------|-------|------|--------|------|------------|-------|------|--------|------|------------|------------|
| | | Sc | outhbo | und | | | W | estbou | und | | | No | orthbo | und | | | E | astbou | Ind | | |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Peak Hour A | nalysi | s From | n 04:00 | D PM t | o 05:45 | PM - F | Peak 1 | of 1 | | | | | | | | | | | | | |
| Peak Hour for | or Enti | re Inte | rsectio | n Beg | ins at 0 | 4:00 P | M | | | | | | | | | | | | | | |
| 04:00 PM | 15 | 56 | 17 | 0 | 88 | 10 | 115 | 61 | 0 | 186 | 33 | 67 | 17 | 0 | 117 | 12 | 84 | 17 | 0 | 113 | 504 |
| 04:15 PM | 7 | 38 | 7 | 0 | 52 | 10 | 98 | 40 | 0 | 148 | 34 | 64 | 25 | 0 | 123 | 19 | 93 | 15 | 0 | 127 | 450 |
| 04:30 PM | 11 | 44 | 8 | 0 | 63 | 19 | 117 | 59 | 0 | 195 | 36 | 75 | 14 | 0 | 125 | 16 | 69 | 9 | 0 | 94 | 477 |
| 04:45 PM | 11 | 39 | 13 | 0 | 63 | 12 | 109 | 58 | 0 | 179 | 34 | 66 | 14 | 0 | 114 | 17 | 67 | 18 | 0 | 102 | 458 |
| Total Volume | 44 | 177 | 45 | 0 | 266 | 51 | 439 | 218 | 0 | 708 | 137 | 272 | 70 | 0 | 479 | 64 | 313 | 59 | 0 | 436 | 1889 |
| % App. Total | 16.5 | 66.5 | 16.9 | 0 | | 7.2 | 62 | 30.8 | 0 | | 28.6 | 56.8 | 14.6 | 0 | | 14.7 | 71.8 | 13.5 | 0 | | |
| PHF | .733 | .790 | .662 | .000 | .756 | .671 | .938 | .893 | .000 | .908 | .951 | .907 | .700 | .000 | .958 | .842 | .841 | .819 | .000 | .858 | .937 |

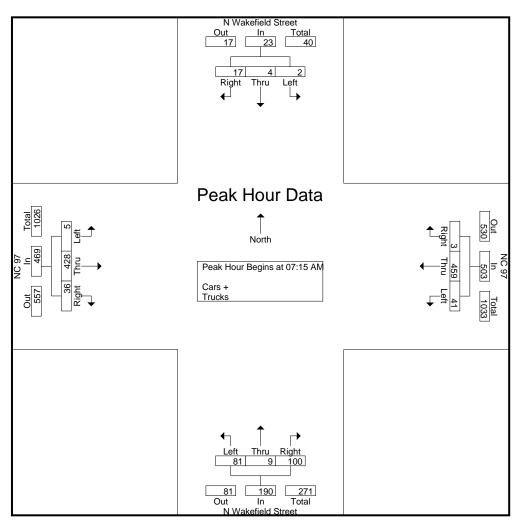




| | | | | | | G | roups F | Printed- C | ars + - | Trucks | | | | | | | |
|-------------|-------|--------|---------|------------|-------|------|---------|------------|---------|--------|-----------|------------|-------|------|-------|------------|------------|
| | N | Wakefi | eld Str | eet | | NC | ; 97 | | N | Wakef | ield Stre | eet | | NC | 97 | | |
| | | South | bound | | | West | bound | | | North | bound | | | East | bound | | |
| Start Time | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Int. Total |
| 07:00 AM | 2 | 1 | 0 | 3 | 0 | 85 | 8 | 93 | 30 | 1 | 16 | 47 | 6 | 59 | 2 | 67 | 210 |
| 07:15 AM | 1 | 1 | 1 | 3 | 0 | 92 | 12 | 104 | 22 | 3 | 15 | 40 | 5 | 89 | 0 | 94 | 241 |
| 07:30 AM | 3 | 0 | 0 | 3 | 1 | 104 | 8 | 113 | 25 | 3 | 26 | 54 | 14 | 127 | 0 | 141 | 311 |
| 07:45 AM | 8 | 3 | 1 | 12 | 1 | 125 | 10 | 136 | 26 | 1 | 24 | 51 | 6 | 119 | 3 | 128 | 327 |
| Total | 14 | 5 | 2 | 21 | 2 | 406 | 38 | 446 | 103 | 8 | 81 | 192 | 31 | 394 | 5 | 430 | 1089 |
| | | | | | | | | | | | | | | | | | |
| 08:00 AM | 5 | 0 | 0 | 5 | 1 | 138 | 11 | 150 | 27 | 2 | 16 | 45 | 11 | 93 | 2 | 106 | 306 |
| 08:15 AM | 2 | 4 | 1 | 7 | 0 | 125 | 8 | 133 | 12 | 2 | 14 | 28 | 12 | 59 | 0 | 71 | 239 |
| 08:30 AM | 1 | 1 | 0 | 2 | 0 | 115 | 6 | 121 | 22 | 1 | 16 | 39 | 15 | 78 | 1 | 94 | 256 |
| 08:45 AM | 3 | 6 | 0 | 9 | 0 | 111 | 5 | 116 | 17 | 0 | 21 | 38 | 11 | 73 | 3 | 87 | 250 |
| Total | 11 | 11 | 1 | 23 | 1 | 489 | 30 | 520 | 78 | 5 | 67 | 150 | 49 | 303 | 6 | 358 | 1051 |
| | | | | | | | | | | | | | | | | | |
| Grand Total | 25 | 16 | 3 | 44 | 3 | 895 | 68 | 966 | 181 | 13 | 148 | 342 | 80 | 697 | 11 | 788 | 2140 |
| Apprch % | 56.8 | 36.4 | 6.8 | | 0.3 | 92.7 | 7 | | 52.9 | 3.8 | 43.3 | | 10.2 | 88.5 | 1.4 | | |
| Total % | 1.2 | 0.7 | 0.1 | 2.1 | 0.1 | 41.8 | 3.2 | 45.1 | 8.5 | 0.6 | 6.9 | 16 | 3.7 | 32.6 | 0.5 | 36.8 | |
| Cars + | 24 | 16 | 3 | 43 | 3 | 873 | 68 | 944 | 177 | 13 | 144 | 334 | 78 | 665 | 11 | 754 | 2075 |
| % Cars + | 96 | 100 | 100 | 97.7 | 100 | 97.5 | 100 | 97.7 | 97.8 | 100 | 97.3 | 97.7 | 97.5 | 95.4 | 100 | 95.7 | 97 |
| Trucks | 1 | 0 | 0 | 1 | 0 | 22 | 0 | 22 | 4 | 0 | 4 | 8 | 2 | 32 | 0 | 34 | 65 |
| % Trucks | 4 | 0 | 0 | 2.3 | 0 | 2.5 | 0 | 2.3 | 2.2 | 0 | 2.7 | 2.3 | 2.5 | 4.6 | 0 | 4.3 | 3 |



| | N | Wakefi | eld Stre | eet | | NC | 97 | | N | Wakefi | eld Stre | et | | NC | 2 97 | | |
|---------------|------------|------------|----------|------------|---------|-----------|-------|------------|-------|--------|----------|------------|-------|------|-------|------------|------------|
| | | South | bound | | | West | bound | | | North | bound | | | East | bound | | |
| Start Time | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Int. Total |
| Peak Hour Ana | alysis Fro | om 07:0 | 0 AM t | o 08:45 A | M - Pea | ak 1 of 1 | | | | | | | | | | | |
| Peak Hour for | Entire In | tersection | on Beg | ins at 07: | 15 AM | | | | | | | | | | | | |
| 07:15 AM | 1 | 1 | 1 | 3 | 0 | 92 | 12 | 104 | 22 | 3 | 15 | 40 | 5 | 89 | 0 | 94 | 241 |
| 07:30 AM | 3 | 0 | 0 | 3 | 1 | 104 | 8 | 113 | 25 | 3 | 26 | 54 | 14 | 127 | 0 | 141 | 311 |
| 07:45 AM | 8 | 3 | 1 | 12 | 1 | 125 | 10 | 136 | 26 | 1 | 24 | 51 | 6 | 119 | 3 | 128 | 327 |
| 08:00 AM | 5 | 0 | 0 | 5 | 1 | 138 | 11 | 150 | 27 | 2 | 16 | 45 | 11 | 93 | 2 | 106 | 306 |
| Total Volume | 17 | 4 | 2 | 23 | 3 | 459 | 41 | 503 | 100 | 9 | 81 | 190 | 36 | 428 | 5 | 469 | 1185 |
| % App. Total | 73.9 | 17.4 | 8.7 | | 0.6 | 91.3 | 8.2 | | 52.6 | 4.7 | 42.6 | | 7.7 | 91.3 | 1.1 | | |
| PHF | .531 | .333 | .500 | .479 | .750 | .832 | .854 | .838 | .926 | .750 | .779 | .880 | .643 | .843 | .417 | .832 | .906 |

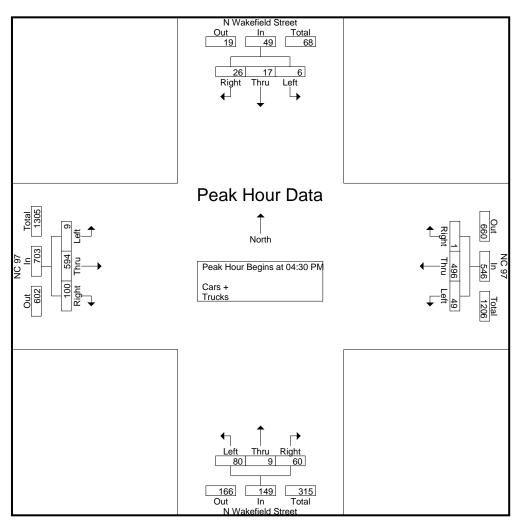




| | | | | | | G | roups F | Printed- C | ars + - | Trucks | | | | | | | |
|-------------|-------|--------|----------|------------|-------|------|---------|------------|---------|--------|-----------|------------|-------|------|-------|------------|------------|
| | N | Wakefi | ield Str | eet | | NC | 97 | | N | Wakef | ield Stre | eet | | NC | C 97 | | |
| | | South | bound | | | West | bound | | | North | bound | | | East | bound | | |
| Start Time | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Int. Total |
| 04:00 PM | 9 | 10 | 1 | 20 | 0 | 132 | 10 | 142 | 22 | 2 | 20 | 44 | 22 | 127 | 2 | 151 | 357 |
| 04:15 PM | 3 | 3 | 0 | 6 | 2 | 88 | 8 | 98 | 16 | 2 | 18 | 36 | 34 | 158 | 2 | 194 | 334 |
| 04:30 PM | 7 | 4 | 2 | 13 | 0 | 118 | 12 | 130 | 12 | 1 | 20 | 33 | 27 | 158 | 6 | 191 | 367 |
| 04:45 PM | 6 | 4 | 0 | 10 | 0 | 135 | 14 | 149 | 13 | 2 | 26 | 41 | 21 | 157 | 2 | 180 | 380 |
| Total | 25 | 21 | 3 | 49 | 2 | 473 | 44 | 519 | 63 | 7 | 84 | 154 | 104 | 600 | 12 | 716 | 1438 |
| | | | | | | | | | | | | | | | | | |
| 05:00 PM | 8 | 5 | 3 | 16 | 1 | 113 | 13 | 127 | 13 | 4 | 16 | 33 | 18 | 151 | 0 | 169 | 345 |
| 05:15 PM | 5 | 4 | 1 | 10 | 0 | 130 | 10 | 140 | 22 | 2 | 18 | 42 | 34 | 128 | 1 | 163 | 355 |
| 05:30 PM | 11 | 1 | 3 | 15 | 0 | 113 | 10 | 123 | 16 | 1 | 16 | 33 | 24 | 136 | 0 | 160 | 331 |
| 05:45 PM | 5 | 2 | 1 | 8 | 0 | 116 | 13 | 129 | 18 | 2 | 15 | 35 | 22 | 128 | 2 | 152 | 324 |
| Total | 29 | 12 | 8 | 49 | 1 | 472 | 46 | 519 | 69 | 9 | 65 | 143 | 98 | 543 | 3 | 644 | 1355 |
| | | | | | | | | | | | | | | | | | |
| Grand Total | 54 | 33 | 11 | 98 | 3 | 945 | 90 | 1038 | 132 | 16 | 149 | 297 | 202 | 1143 | 15 | 1360 | 2793 |
| Apprch % | 55.1 | 33.7 | 11.2 | | 0.3 | 91 | 8.7 | | 44.4 | 5.4 | 50.2 | | 14.9 | 84 | 1.1 | | |
| Total % | 1.9 | 1.2 | 0.4 | 3.5 | 0.1 | 33.8 | 3.2 | 37.2 | 4.7 | 0.6 | 5.3 | 10.6 | 7.2 | 40.9 | 0.5 | 48.7 | |
| Cars + | 53 | 33 | 11 | 97 | 3 | 914 | 88 | 1005 | 127 | 16 | 144 | 287 | 197 | 1120 | 15 | 1332 | 2721 |
| % Cars + | 98.1 | 100 | 100 | 99 | 100 | 96.7 | 97.8 | 96.8 | 96.2 | 100 | 96.6 | 96.6 | 97.5 | 98 | 100 | 97.9 | 97.4 |
| Trucks | 1 | 0 | 0 | 1 | 0 | 31 | 2 | 33 | 5 | 0 | 5 | 10 | 5 | 23 | 0 | 28 | 72 |
| % Trucks | 1.9 | 0 | 0 | 1 | 0 | 3.3 | 2.2 | 3.2 | 3.8 | 0 | 3.4 | 3.4 | 2.5 | 2 | 0 | 2.1 | 2.6 |



| | N | Wakefi | eld Stre | eet | | NC | 97 | | N | Wakef | ield Stre | et | | NC | 97 | | |
|---------------|------------|----------|----------|------------|----------|-----------|-------|------------|-------|-------|-----------|------------|-------|------|-------|------------|------------|
| | | South | bound | | | West | bound | | | North | bound | | | East | bound | | |
| Start Time | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Right | Thru | Left | App. Total | Int. Total |
| Peak Hour Ana | ilysis Fro | om 04:0 | 0 PM t | o 05:45 F | PM - Pea | ak 1 of 1 | | | | | | | | | | | |
| Peak Hour for | Entire In | tersecti | on Beg | ins at 04: | 30 PM | | | | | | | | | | | | |
| 04:30 PM | 7 | 4 | 2 | 13 | 0 | 118 | 12 | 130 | 12 | 1 | 20 | 33 | 27 | 158 | 6 | 191 | 367 |
| 04:45 PM | 6 | 4 | 0 | 10 | 0 | 135 | 14 | 149 | 13 | 2 | 26 | 41 | 21 | 157 | 2 | 180 | 380 |
| 05:00 PM | 8 | 5 | 3 | 16 | 1 | 113 | 13 | 127 | 13 | 4 | 16 | 33 | 18 | 151 | 0 | 169 | 345 |
| 05:15 PM | 5 | 4 | 1 | 10 | 0 | 130 | 10 | 140 | 22 | 2 | 18 | 42 | 34 | 128 | 1 | 163 | 355 |
| Total Volume | 26 | 17 | 6 | 49 | 1 | 496 | 49 | 546 | 60 | 9 | 80 | 149 | 100 | 594 | 9 | 703 | 1447 |
| % App. Total | 53.1 | 34.7 | 12.2 | | 0.2 | 90.8 | 9 | | 40.3 | 6 | 53.7 | | 14.2 | 84.5 | 1.3 | | |
| PHF | .813 | .850 | .500 | .766 | .250 | .919 | .875 | .916 | .682 | .563 | .769 | .887 | .735 | .940 | .375 | .920 | .952 |

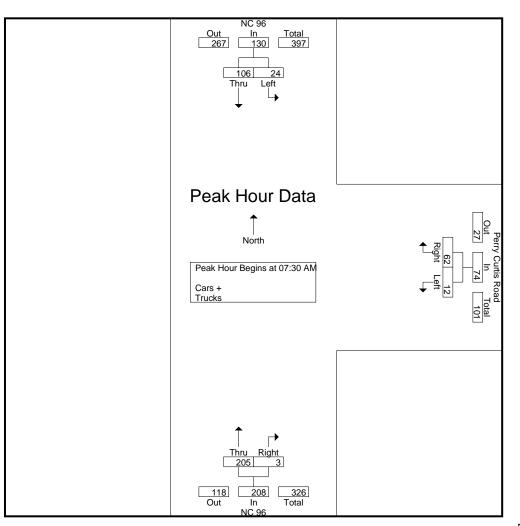




| | | | Gi | oups Printed | Cars + - | Trucks | | | | |
|-------------|------|-----------|------------|--------------|-------------|------------|-------|-----------|------------|------------|
| | | NC 96 | | | y Curtis Ro | | | NC 96 | | |
| | S | outhbound | | V | Vestbound | | N | orthbound | k | |
| Start Time | Thru | Left | App. Total | Right | Left | App. Total | Right | Thru | App. Total | Int. Total |
| 07:00 AM | 17 | 3 | 20 | 17 | 3 | 20 | 2 | 42 | 44 | 84 |
| 07:15 AM | 20 | 2 | 22 | 13 | 0 | 13 | 0 | 64 | 64 | 99 |
| 07:30 AM | 20 | 4 | 24 | 9 | 3 | 12 | 0 | 61 | 61 | 97 |
| 07:45 AM | 31 | 5 | 36 | 19 | 3 | 22 | 1 | 57 | 58 | 116 |
| Total | 88 | 14 | 102 | 58 | 9 | 67 | 3 | 224 | 227 | 396 |
| 08:00 AM | 27 | 6 | 33 | 13 | 2 | 15 | 1 | 49 | 50 | 98 |
| 08:15 AM | 28 | 9 | 37 | 21 | 4 | 25 | 1 | 38 | 39 | 101 |
| 08:30 AM | 17 | 8 | 25 | 8 | 2 | 10 | 2 | 47 | 49 | 84 |
| 08:45 AM | 24 | 2 | 26 | 19 | 3 | 22 | 1 | 43 | 44 | 92 |
| Total | 96 | 25 | 121 | 61 | 11 | 72 | 5 | 177 | 182 | 375 |
| Grand Total | 184 | 39 | 223 | 119 | 20 | 139 | 8 | 401 | 409 | 771 |
| Apprch % | 82.5 | 17.5 | - | 85.6 | 14.4 | | 2 | 98 | | |
| Total % | 23.9 | 5.1 | 28.9 | 15.4 | 2.6 | 18 | 1 | 52 | 53 | |
| Cars + | 166 | 38 | 204 | 117 | 19 | 136 | 5 | 378 | 383 | 723 |
| % Cars + | 90.2 | 97.4 | 91.5 | 98.3 | 95 | 97.8 | 62.5 | 94.3 | 93.6 | 93.8 |
| Trucks | 18 | 1 | 19 | 2 | 1 | 3 | 3 | 23 | 26 | 48 |
| % Trucks | 9.8 | 2.6 | 8.5 | 1.7 | 5 | 2.2 | 37.5 | 5.7 | 6.4 | 6.2 |



| | | NC 96 | | Perry Curtis Road | | | | NC 96 | | |
|--------------------------|---------------|--------------|-----------------|-------------------|-----------|------------|-------|-------|------------|------------|
| | S | Southbound | | | Westbound | | | | | |
| Start Time | Thru | Left | App. Total | Right | Left | App. Total | Right | Thru | App. Total | Int. Total |
| Peak Hour Analysis Fro | m 07:00 AM t | o 08:45 AN | 1 - Peak 1 of 1 | | | | - | | | |
| Peak Hour for Entire Int | ersection Beg | ins at 07:30 | 0 AM | | | | | | | |
| 07:30 AM | 20 | 4 | 24 | 9 | 3 | 12 | 0 | 61 | 61 | 97 |
| 07:45 AM | 31 | 5 | 36 | 19 | 3 | 22 | 1 | 57 | 58 | 116 |
| 08:00 AM | 27 | 6 | 33 | 13 | 2 | 15 | 1 | 49 | 50 | 98 |
| 08:15 AM | 28 | 9 | 37 | 21 | 4 | 25 | 1 | 38 | 39 | 101 |
| Total Volume | 106 | 24 | 130 | 62 | 12 | 74 | 3 | 205 | 208 | 412 |
| % App. Total | 81.5 | 18.5 | | 83.8 | 16.2 | | 1.4 | 98.6 | | |
| PHF | .855 | .667 | .878 | .738 | .750 | .740 | .750 | .840 | .852 | .888 |

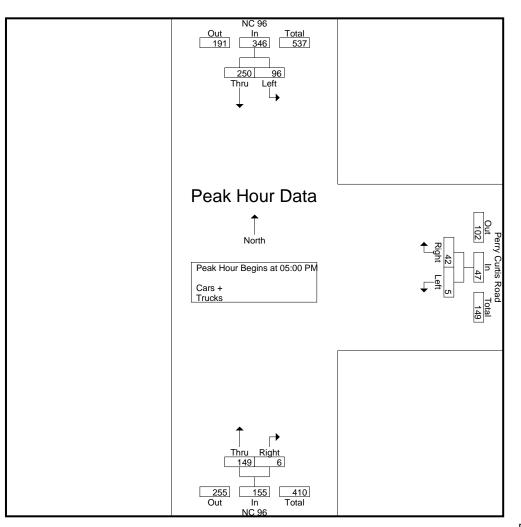




| | | | Gi | roups Printed | | | | | | |
|-------------|------|-----------|------------|---------------|-------------|------------|----------|------------|------------|------------|
| | | NC 96 | | | y Curtis Ro | | | NC 96 | | |
| | S | outhbounc | | V | Vestbound | | <u> </u> | lorthbound | d | |
| Start Time | Thru | Left | App. Total | Right | Left | App. Total | Right | Thru | App. Total | Int. Total |
| 04:00 PM | 56 | 20 | 76 | 12 | 4 | 16 | 2 | 35 | 37 | 129 |
| 04:15 PM | 50 | 12 | 62 | 21 | 2 | 23 | 1 | 37 | 38 | 123 |
| 04:30 PM | 52 | 28 | 80 | 14 | 1 | 15 | 2 | 41 | 43 | 138 |
| 04:45 PM | 63 | 24 | 87 | 17 | 1 | 18 | 0 | 32 | 32 | 137 |
| Total | 221 | 84 | 305 | 64 | 8 | 72 | 5 | 145 | 150 | 527 |
| 05:00 PM | 49 | 24 | 73 | 13 | 2 | 15 | 1 | 31 | 32 | 120 |
| 05:15 PM | 72 | 23 | 95 | 10 | 2 | 12 | 3 | 40 | 43 | 150 |
| 05:30 PM | 64 | 19 | 83 | 8 | 0 | 8 | 1 | 36 | 37 | 128 |
| 05:45 PM | 65 | 30 | 95 | 11 | 1 | 12 | 1 | 42 | 43 | 150 |
| Total | 250 | 96 | 346 | 42 | 5 | 47 | 6 | 149 | 155 | 548 |
| Grand Total | 471 | 180 | 651 | 106 | 13 | 119 | 11 | 294 | 305 | 1075 |
| Apprch % | 72.4 | 27.6 | | 89.1 | 10.9 | | 3.6 | 96.4 | | |
| Total % | 43.8 | 16.7 | 60.6 | 9.9 | 1.2 | 11.1 | 1 | 27.3 | 28.4 | |
| Cars + | 461 | 178 | 639 | 104 | 11 | 115 | 11 | 280 | 291 | 1045 |
| % Cars + | 97.9 | 98.9 | 98.2 | 98.1 | 84.6 | 96.6 | 100 | 95.2 | 95.4 | 97.2 |
| Trucks | 10 | 2 | 12 | 2 | 2 | 4 | 0 | 14 | 14 | 30 |
| % Trucks | 2.1 | 1.1 | 1.8 | 1.9 | 15.4 | 3.4 | 0 | 4.8 | 4.6 | 2.8 |

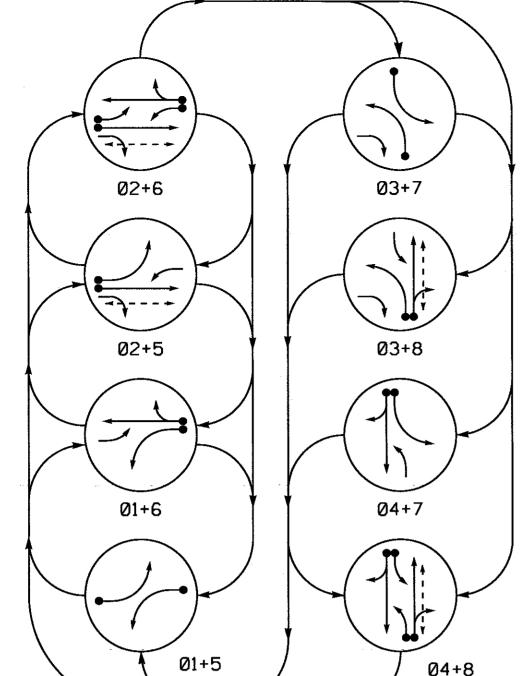


| | | NC 96 | | Perr | y Curtis R | bad | | NC 96 | | |
|---------------------------|---------------|-------------|-----------------|-------|------------|------------|-------|-------|------------|------------|
| | S | outhbound | | V | | 1 | | | | |
| Start Time | Thru | Left | App. Total | Right | Left | App. Total | Right | Thru | App. Total | Int. Total |
| Peak Hour Analysis Fro | m 04:00 PM t | o 05:45 PN | 1 - Peak 1 of 1 | | | | | | | |
| Peak Hour for Entire Inte | ersection Beg | ins at 05:0 | 0 PM | | | | | | | |
| 05:00 PM | 49 | 24 | 73 | 13 | 2 | 15 | 1 | 31 | 32 | 120 |
| 05:15 PM | 72 | 23 | 95 | 10 | 2 | 12 | 3 | 40 | 43 | 150 |
| 05:30 PM | 64 | 19 | 83 | 8 | 0 | 8 | 1 | 36 | 37 | 128 |
| 05:45 PM | 65 | 30 | 95 | 11 | 1 | 12 | 1 | 42 | 43 | 150 |
| Total Volume | 250 | 96 | 346 | 42 | 5 | 47 | 6 | 149 | 155 | 548 |
| % App. Total | 72.3 | 27.7 | | 89.4 | 10.6 | | 3.9 | 96.1 | | |
| PHF | .868 | .800 | .911 | .808 | .625 | .783 | .500 | .887 | .901 | .913 |



Appendix C – Signal Timing Plans

PHASING DIAGRAM

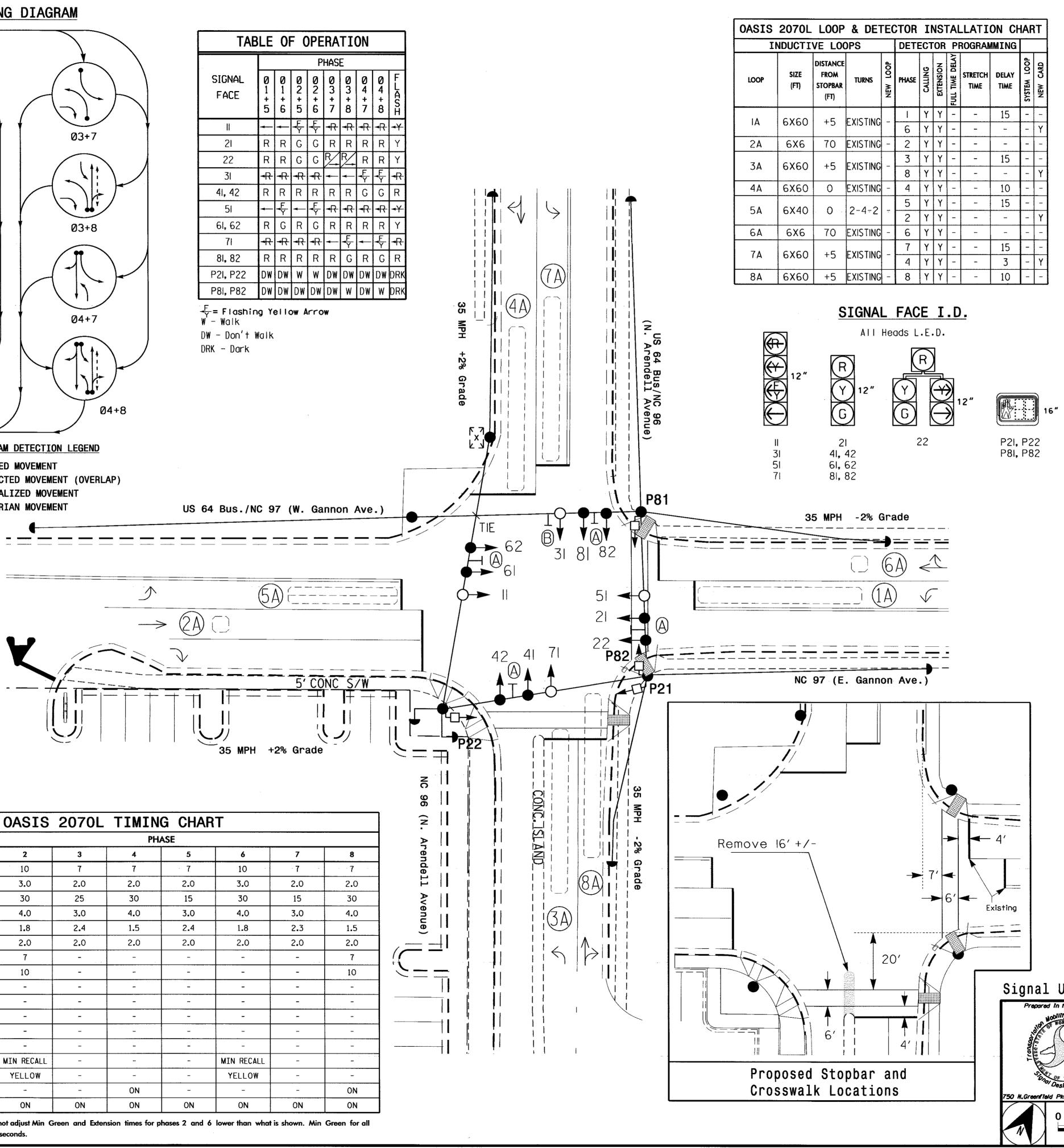


| TABLE OF OPERATI | | | | | | | | | | |
|------------------|------------------|------------------|------|---------------|----------------|----------------|--|--|--|--|
| | PHASE | | | | | | | | | |
| signal Face | 0 1 + 5 | Ø 1 + 6 | Ø2+5 | Ø2+6 | Ø3+7 | Ø3+8 | | | | |
| | | + | F | F | ₽ | - ₽ | | | | |
| 21 | R | R | G | G | R | R | | | | |
| 22 | R | R | G | G | R | Ŕ | | | | |
| 31 | ⊀R | ╉ | ╉ | ╉ | + | * | | | | |
| 41, 42 | R | R | R | R | R | R | | | | |
| 51 | + | F | ł | F | - ₽ | - R | | | | |
| 61, 62 | R | G | R | G | R | R | | | | |
| 71 | ∢R | ⊀R | ₹R | ⊀R | + | Ŧ | | | | |
| 81, 82 | R | R | R | R | R | G | | | | |
| P2I, P22 | DW | DW | W | W | DW | DW | | | | |
| P8I, P82 | DW | DW | D₩ | DW | DW | W | | | | |

PHASING DIAGRAM DETECTION LEGEND

DETECTED MOVEMENT UNDETECTED MOVEMENT (OVERLAP) UNSIGNALIZED MOVEMENT **---**<---> PEDESTRIAN MOVEMENT

| | 5A (|
|----------------------|------|
| | |
| \rightarrow $(2A)$ | |



| | | OASIS | 2070L | TIMIN | G CHAR | T | | | | | | |
|-------------------------|-------|------------|-------|-------|--------|------------|-------|-----|--|--|--|--|
| | PHASE | | | | | | | | | | | |
| FEATURE | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Min Green 1 * | 7 | 10 | 7 | 7 | • 7 | 10 | · 7 · | 7 | | | | |
| Extension 1 * | 2.0 | 3.0 | 2.0 | 2.0 | 2.0 | 3.0 | 2.0 | 2.0 | | | | |
| Max Green 1 * | 15 | 30 | 25 | 30 | 15 | 30 | 15 | 30 | | | | |
| Yellow Clearance | 3.0 | 4.0 | 3.0 | 4.0 | 3.0 | 4.0 | 3.0 | 4.0 | | | | |
| Red Clearance | 2.8 | 1.8 | 2.4 | 1.5 | 2.4 | 1.8 | 2.3 | 1.5 | | | | |
| Red Revert | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | | | | |
| Walk 1 * | _ | 7 | - | | - | - | - | 7 | | | | |
| Don't Walk 1 | _ | 10 | - | - | | - | - | 10 | | | | |
| Seconds Per Actuation * | _ | - | - | - | - | - | - | - | | | | |
| Max Variable Initial * | _ | - | - | | | - | - | - | | | | |
| Time Before Reduction * | _ | - | - | - | | - | - | - | | | | |
| Time To Reduce * | - | - | - | _ | - | - | - | - | | | | |
| Minimum Gap | _ | - | - | | - | - | - | - | | | | |
| Recall Mode | - | MIN RECALL | - | - | - | MIN RECALL | - | - | | | | |
| Vehicle Call Memory | _ | YELLOW | _ | _ | - | YELLOW | - | - | | | | |
| Dual Entry | _ | - | _ | ON | - | - | - | ON | | | | |
| Simultaneous Gap | ON | ON | ON | ON | ON | ON | ON | ON | | | | |

* These values may be field adjusted. Do not adjust Min Green and Extension times for phases 2 and 6 lower than what is shown. Min Green for all other phases should not be lower than 4 seconds.

| PROJECT REFERENCE NO. | SHEET | NO |
|-----------------------|-------|----|
| 05-0156 | Sig. | 1 |

8 Phase Fully Actuated (Isolated)

NOTES

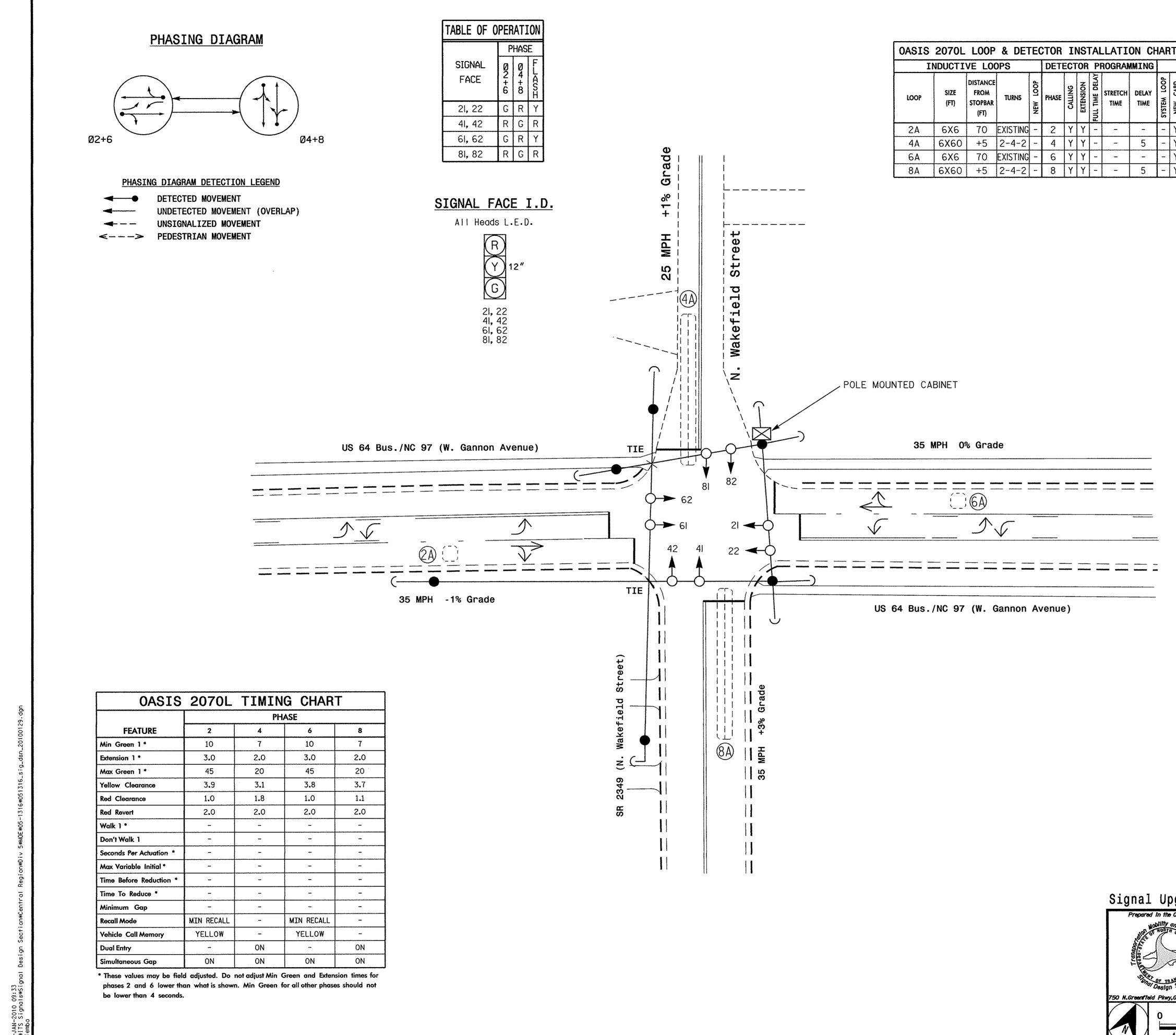
- 1. Refer to "Roadway Standard Drawings NCDOT" dated July 2006 and "Standard Specifications for Roads and Structures" dated July 2006.
- 2. Do not program signal for late night flashing operation unless otherwise directed by the Engineer.
- Phase 1 and/or phase 5 may be lagged. 3.
- Phase 3 and/or phase 7 may be lagged. 4.
- Set all detector units to presence mode. 5. 6. In the event of loop replacement, refer
- to the current ITS and Signals Design Manual and submit a Plan of Record to the Signal Design Section.
- 7. Omit "WALK" and flashing "DON'T WALK" with no pedestrian calls.
- 8. Program pedestrian heads to countdown the flashing "Don't Walk" time only.
- 9. Pavement markings are existing unless otherwise shown.

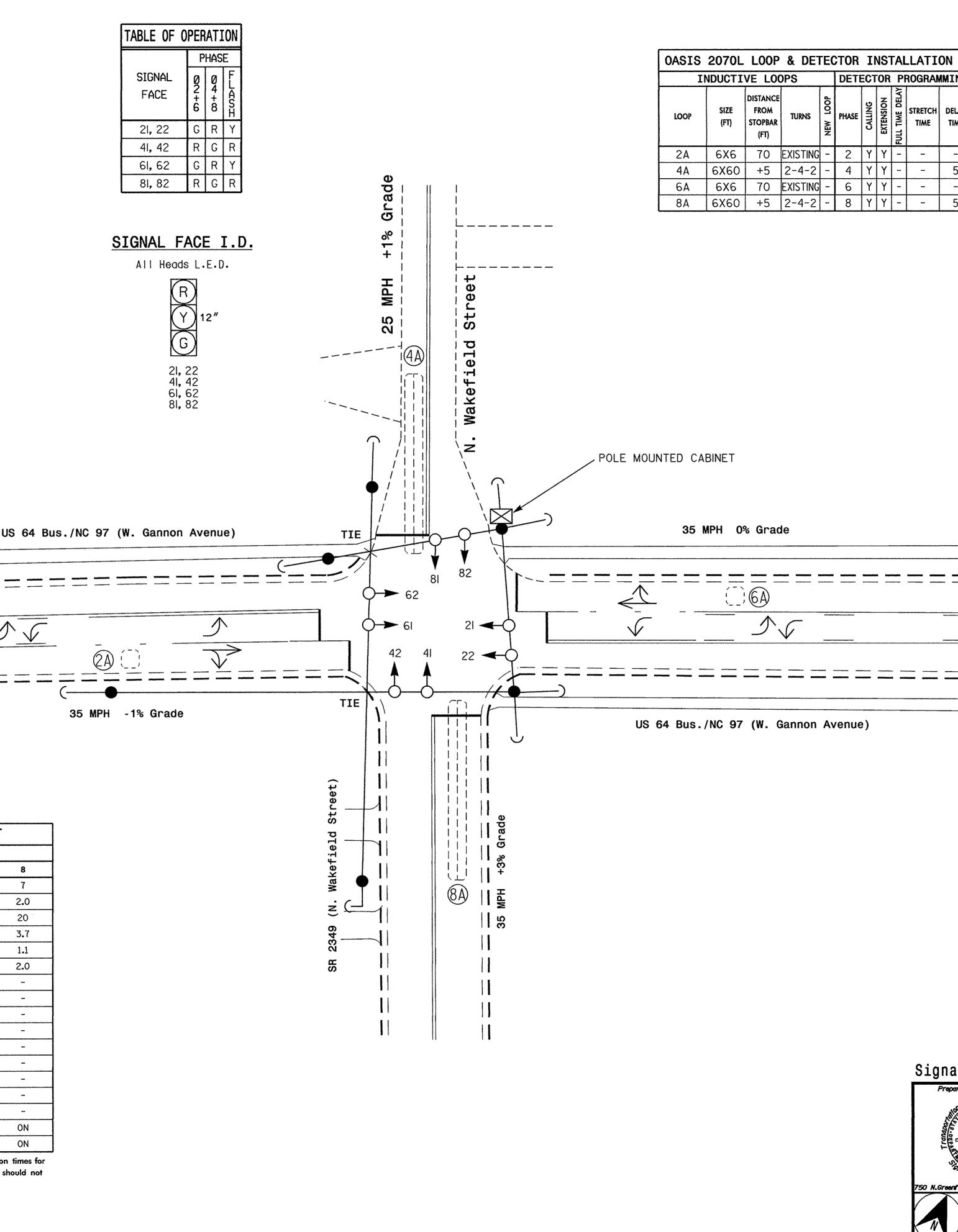
LEGEND

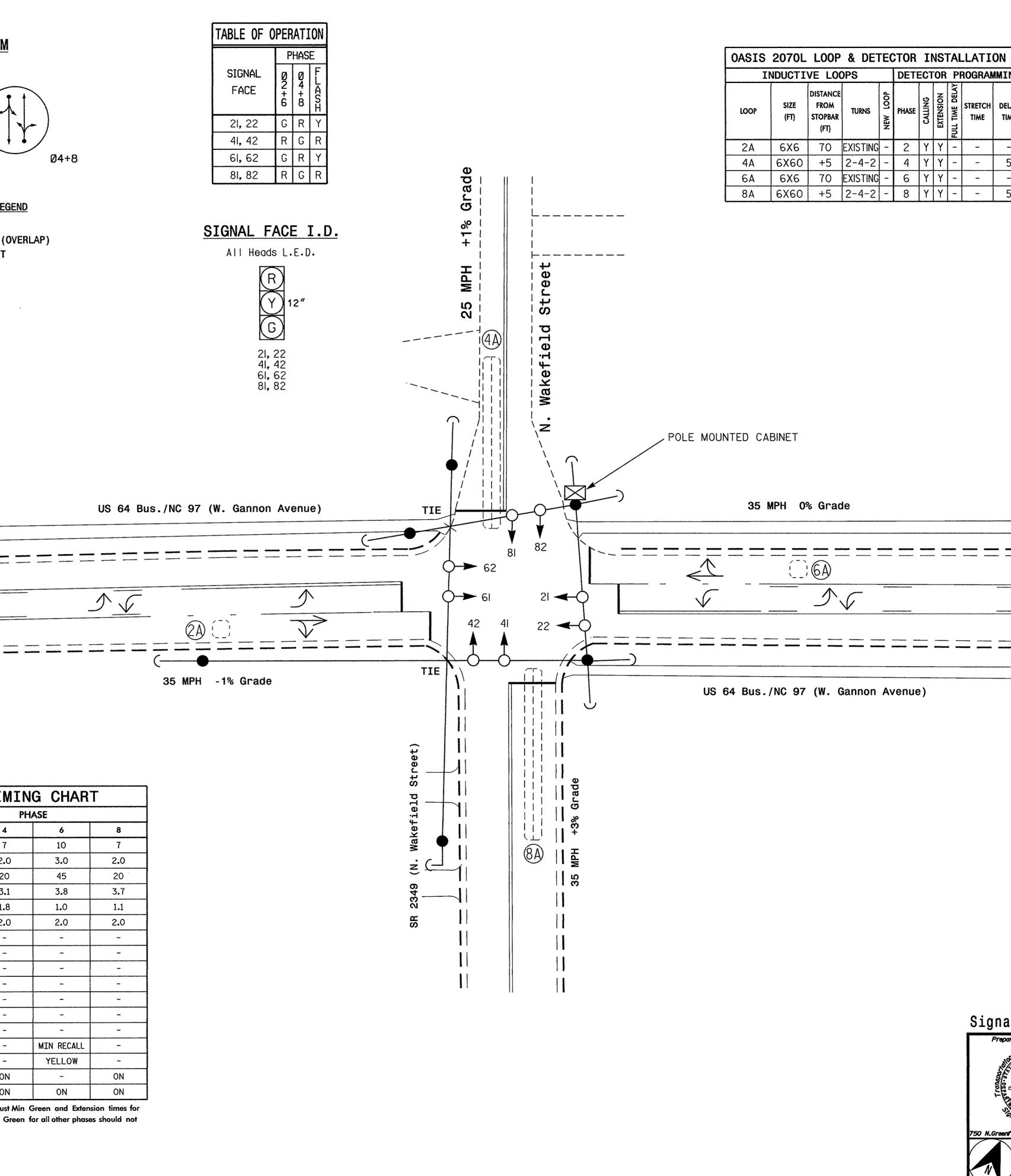
| | · | |
|---------------------|---|---------------|
| PROPOSED | | EXISTING |
| \frown | Traffic Signal Head | •>- |
| ●→ | Modified Signal Head | N/A |
| 1 | Sign | |
| i ↓ | Pedestrian Signal Head With Push Button & Sign | v v a∰n |
| \sim | Signal Pole with Guy | • |
| <u> </u> | Signal Pole with Sidewalk Guy | • |
| | Inductive Loop Detector | \square |
| \bowtie | Controller & Cabinet | |
| | Junction Box | |
| | - 2-in Underground Conduit | |
| N/A | Right of Way | |
| \rightarrow | Directional Arrow | \rightarrow |
| $\langle A \rangle$ | Street Name Sign (D3-1) | (A) |
| B | "U-TURN YIELD TO RIGHT TURN" Sign (R10-16) | B |
| | | |

Signal Upgrade

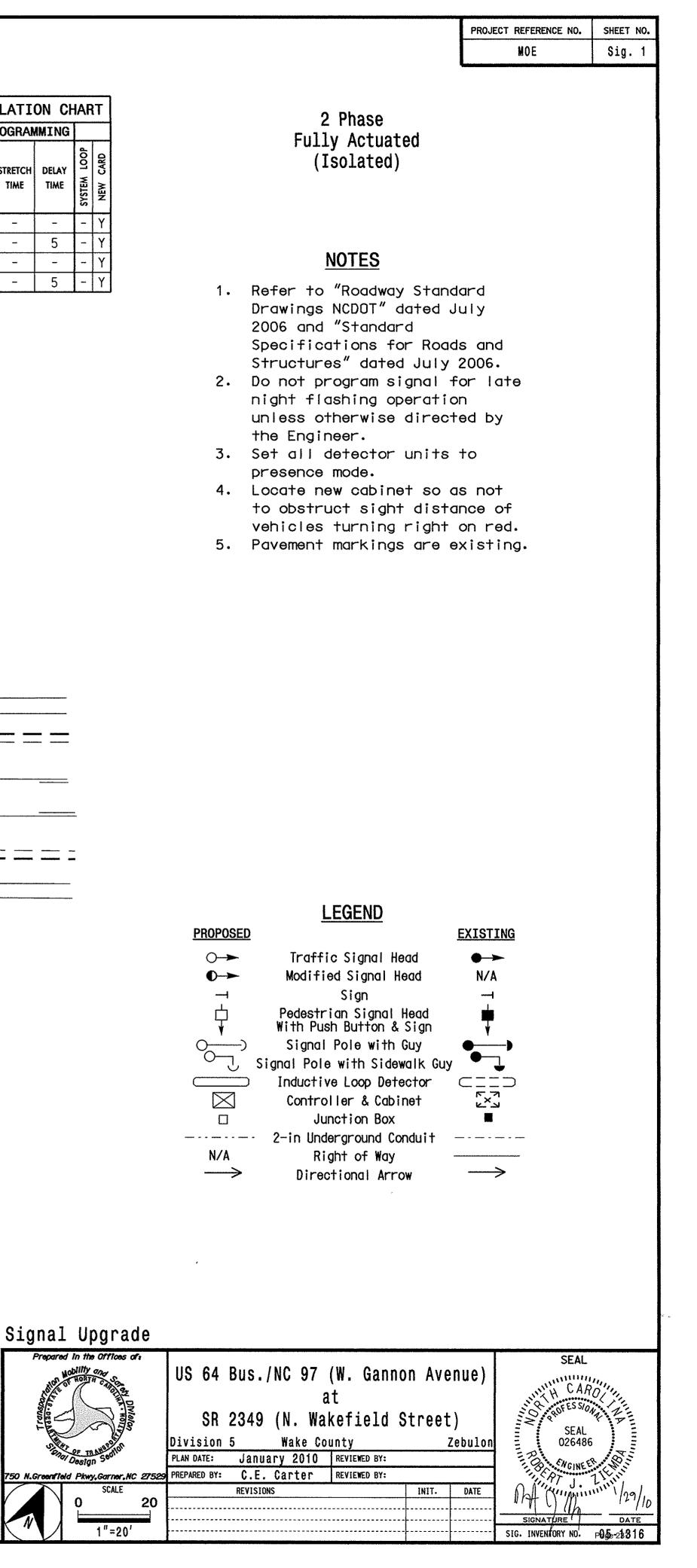
| nur opgrade | | |
|--|--|---|
| Prepared in the Offices of: Nobility on- Nobility on- N | US 64 Bus./NC 97 (Gannon Ave at US 64 Bus./NC 96 (N. Arendell A Division 5 Wake County 2 PLAN DATE: November 2011 Reviewed By: | WITH CARO |
| "Design " | PLAN DATE. NUVEHIDET ZUTT NEVIENED DI. | |
| reenfield Pkwy,Garner,NC 27529 | PREPARED BY: Sterling REVIEWED BY: | THE PART IN THE T |
| SCALE 0 20 1"=20' | REVISIONS INIT. | DATE 1 4 0 11/12/12 SIGNATURE DATE SIG. INVENTORY NO. 05-0156 |







| OASIS | 2070L | TIMIN | G CHAR | Т |
|-------------------------|------------|-------|------------|-----|
| | | PH | ASE | |
| FEATURE | 2 | 4 | 6 | 8 |
| Min Green 1 * | 10 | 7 | 10 | 7 |
| Extension 1 * | 3.0 | 2.0 | 3.0 | 2.0 |
| Max Green 1 * | 45 | 20 | 45 | 20 |
| Yellow Clearance | 3.9 | 3.1 | 3.8 | 3.7 |
| Red Clearance | 1.0 | 1.8 | 1.0 | 1.1 |
| Red Revert | 2.0 | 2.0 | 2.0 | 2.0 |
| Walk 1 * | - | | | |
| Don't Walk 1 | - | - | | - |
| Seconds Per Actuation * | - | - | - | _ |
| Max Variable Initial * | | - | - | - |
| Time Before Reduction * | - | | - | - |
| Time To Reduce * | - | - | | - |
| Minimum Gap | | - | | _ |
| Recall Mode | MIN RECALL | - | MIN RECALL | _ |
| Vehicle Call Memory | YELLOW | - | YELLOW | - |
| Dual Entry | | ON | - | ON |
| Simultaneous Gap | ON | ON | ON | ON |



Appendix D – Synchro Output

2022 Existing Traffic Volumes

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/04/2023

| | ٠ | → | 7 | 4 | + | * | 1 | t | 1 | 1 | ţ | ~ |
|----------------------------|-------|----------|-------|-------|-------|---------|-------|-------|---------|-------|-------|--------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ሻ | ef 🗧 | | ሻ | ¢Î 🔒 | | | 4 | | | \$ | |
| Traffic Volume (vph) | 5 | 428 | 36 | 41 | 459 | 4 | 81 | 9 | 100 | 4 | 4 | 17 |
| Future Volume (vph) | 5 | 428 | 36 | 41 | 459 | 4 | 81 | 9 | 100 | 4 | 4 | 17 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.988 | | | 0.999 | | | 0.929 | | | 0.905 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.979 | | | 0.993 | |
| Satd. Flow (prot) | 1778 | 1850 | 0 | 1770 | 1861 | 0 | 0 | 1669 | 0 | 0 | 1666 | 0 |
| Flt Permitted | 0.393 | | | 0.391 | | | | 0.849 | | | 0.942 | - |
| Satd. Flow (perm) | 736 | 1850 | 0 | 728 | 1861 | 0 | 0 | 1447 | 0 | 0 | 1580 | 0 |
| Right Turn on Red | | | No | - | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 6 | 476 | 40 | 46 | 510 | 4 | 90 | 10 | 111 | 4 | 4 | 19 |
| Shared Lane Traffic (%) | Ŭ | • | | | • • • | · | | | | · · | | |
| Lane Group Flow (vph) | 6 | 516 | 0 | 46 | 514 | 0 | 0 | 211 | 0 | 0 | 27 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | | 12 | | | 12 | . ug. u | | 0 | . ugint | | 0 | . ug.u |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | Yes | | | | | | | |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 | | 9 | 15 | | 9 | 15 | | 9 | 15 | | 9 |
| Turn Type | Perm | NA | | Perm | NA | | Perm | NA | | Perm | NA | - |
| Protected Phases | | 2 | | | 6 | | | 8 | | | 4 | |
| Permitted Phases | 2 | | | 6 | - | | 8 | | | 4 | - | |
| Detector Phase | 2 | 2 | | 6 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | | | | | | | | | | | | |
| Minimum Initial (s) | 10.0 | 10.0 | | 10.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 14.9 | 14.9 | | 14.8 | 14.8 | | 11.8 | 11.8 | | 11.9 | 11.9 | |
| Total Split (s) | 45.0 | 45.0 | | 45.0 | 45.0 | | 20.0 | 20.0 | | 20.0 | 20.0 | |
| Total Split (%) | 69.2% | 69.2% | | 69.2% | 69.2% | | 30.8% | 30.8% | | 30.8% | 30.8% | |
| Maximum Green (s) | 40.1 | 40.1 | | 40.2 | 40.2 | | 15.2 | 15.2 | | 15.1 | 15.1 | |
| Yellow Time (s) | 3.9 | 3.9 | | 3.8 | 3.8 | | 3.7 | 3.7 | | 3.1 | 3.1 | |
| All-Red Time (s) | 1.0 | 1.0 | | 1.0 | 1.0 | | 1.1 | 1.1 | | 1.8 | 1.8 | |
| Lost Time Adjust (s) | 0.1 | 0.1 | | 0.2 | 0.2 | | | 0.2 | | | 0.1 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | |
| Lead/Lag | 5.0 | 2.0 | | 5.0 | 2.0 | | | | | | 0.0 | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | | Min | Min | | None | None | | None | None | |
| Act Effct Green (s) | 17.6 | 17.6 | | 17.6 | 17.6 | | | 9.9 | | | 9.9 | |
| Actuated g/C Ratio | 0.46 | 0.46 | | 0.46 | 0.46 | | | 0.26 | | | 0.26 | |
| v/c Ratio | 0.40 | 0.40 | | 0.40 | 0.40 | | | 0.20 | | | 0.20 | |
| Control Delay | 6.6 | 11.6 | | 7.9 | 11.5 | | | 18.1 | | | 11.0 | |
| | 0.0 | 11.0 | | 1.5 | 11.0 | | | 10.1 | | | 11.0 | |

2022 Existing AM Peak Hour Timmons Group

Synchro 11 Report

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/04/2023

| | ٨ | → | 7 | 4 | + | * | 1 | t | 1 | 4 | ţ | ~ |
|------------------------------|--------------|----------|-----|------|-----------|------------|-----|------|-----|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 6.6 | 11.6 | | 7.9 | 11.5 | | | 18.1 | | | 11.0 | |
| LOS | А | В | | А | В | | | В | | | В | |
| Approach Delay | | 11.6 | | | 11.2 | | | 18.1 | | | 11.0 | |
| Approach LOS | | В | | | В | | | В | | | В | |
| Queue Length 50th (ft) | 1 | 65 | | 4 | 65 | | | 31 | | | 3 | |
| Queue Length 95th (ft) | 5 | 170 | | 21 | 168 | | | 94 | | | 18 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 706 | 1775 | | 699 | 1786 | | | 591 | | | 645 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.01 | 0.29 | | 0.07 | 0.29 | | | 0.36 | | | 0.04 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: | Other | | | | | | | | | | | |
| Cycle Length: 65 | | | | | | | | | | | | |
| Actuated Cycle Length: 37 | 7.9 | | | | | | | | | | | |
| Natural Cycle: 40 | | | | | | | | | | | | |
| Control Type: Actuated-U | ncoordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.60 | | | | | | | | | | | | |
| Intersection Signal Delay: | | | | | tersectio | | | | | | | |
| Intersection Capacity Utiliz | zation 60.2% | 6 | | IC | U Level | of Service | вB | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |

Splits and Phases: 1: S Wakefield Street & NC-97 (Gannon Avenue)

| | Ø4 |
|---------|-------------|
| 45 s | 20 s |
| € Ø6 | ™ ø8 |
| 45 s | 20 s |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/04/2023

| , | ٠ | -+ | 7 | 4 | + | * | 1 | 1 | 1 | 1 | ŧ | ~ |
|--|-------------|----------|------------|-------------|---------|-------|-------------|---------|-------|------------|-------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ٦ | † | 1 | ٦ | ţ, | | ٦ | f. | | 5 | Þ | _ |
| Traffic Volume (vph) | 41 | 396 | 62 | 196 | 310 | 17 | 76 | 156 | 123 | 22 | 169 | 29 |
| Future Volume (vph) | 41 | 396 | 62 | 196 | 310 | 17 | 76 | 156 | 123 | 22 | 169 | 29 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | 2% | | | -2% | | | -2% | | | 2% | |
| Storage Length (ft) | 200 | | 100 | 350 | | 0 | 125 | | 0 | 250 | | 0 |
| Storage Lanes | 1 | | 1 | 1 | | 0 | 1 | | 0 | 1 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | 0.850 | | 0.992 | | | 0.934 | | | 0.978 | |
| Flt Protected | 0.950 | | | 0.950 | | | 0.950 | | | 0.950 | | |
| Satd. Flow (prot) | 1752 | 1844 | 1567 | 1787 | 1866 | 0 | 1787 | 1757 | 0 | 1752 | 1804 | 0 |
| Flt Permitted | 0.418 | | | 0.263 | | | 0.495 | | | 0.380 | | |
| Satd. Flow (perm) | 771 | 1844 | 1567 | 495 | 1866 | 0 | 931 | 1757 | 0 | 701 | 1804 | 0 |
| Right Turn on Red | | | No | | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 20 | | | 35 | |
| Link Distance (ft) | | 1453 | | | 677 | | | 1822 | | | 478 | |
| Travel Time (s) | | 28.3 | | | 13.2 | | | 62.1 | | | 9.3 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 46 | 440 | 69 | 218 | 344 | 19 | 84 | 173 | 137 | 24 | 188 | 32 |
| Shared Lane Traffic (%) | | | | | | - | | | - | | | |
| Lane Group Flow (vph) | 46 | 440 | 69 | 218 | 363 | 0 | 84 | 310 | 0 | 24 | 220 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | | 12 | | | 12 | | | 12 | | | 12 | |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | _ |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | 1 0 1 | Yes | 1 0 1 | 0.00 | Yes | 0.00 | 0.00 | 0.00 | 0.00 | 1.04 | Yes | 1.04 |
| Headway Factor | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 1.01 15 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 D.P+P | NIA | 9 | 15 D.P+P | NA | 9 | 15 D.P+P | NIA | 9 | D.P+P | NIA | 9 |
| Turn Type Protected Phases | | NA 2 | pm+ov 3 | D.P+P 1 | NA 6 | | D.P+P 3 | NA 8 | | | NA | |
| | 5 | 2 | | | 0 | | | 0 | | 7 | 4 | |
| Permitted Phases Detector Phase | 6 5 | 2 | 2 | 2 | 6 | | 4 | 8 | | 8 7 | 4 | |
| Switch Phase | 5 | Z | 3 | 1 | 0 | | 3 | 0 | | 1 | 4 | |
| Minimum Initial (s) | 7.0 | 10.0 | 7.0 | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 12.4 | 22.8 | 12.4 | 12.8 | 15.8 | | 12.4 | 22.5 | | 12.3 | 12.5 | |
| Total Split (s) | 12.4 | 30.0 | 25.0 | 12.0 | 30.0 | | 25.0 | 40.0 | | 12.5 | 30.0 | |
| Total Split (%) | 15.0% | 30.0% | 25.0% | 15.0% | 30.0% | | 25.0% | 40.0% | | 15.0% | 30.0% | |
| Maximum Green (s) | 9.6 | 24.2 | 19.6 | 9.2 | 24.2 | | 19.6 | 34.5 | | 9.7 | 24.5 | |
| Yellow Time (s) | 3.0 | 4.0 | 3.0 | 3.0 | 4.0 | | 3.0 | 4.0 | | 3.0 | 4.0 | |
| All-Red Time (s) | 2.4 | 1.8 | 2.4 | 2.8 | 1.8 | | 2.4 | 1.5 | | 2.3 | 1.5 | |
| Lost Time Adjust (s) | -0.4 | -0.8 | -0.4 | -0.8 | -0.8 | | -0.4 | -0.5 | | -0.3 | -0.5 | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| Lead/Lag | Lead | Lag | Lead | Lead | Lag | | Lead | Lag | | Lead | Lag | |
| Lead-Lag Optimize? | Loud | Lug | Loud | Loud | Lug | | Loud | Lug | | Loud | Lug | |
| Vehicle Extension (s) | 2.0 | 3.0 | 2.0 | 2.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | None | Min | None | None | Min | | None | None | | None | None | |
| Walk Time (s) | | 7.0 | | | | | | 7.0 | | | | |
| Flash Dont Walk (s) | | 10.0 | | | | | | 10.0 | | | | |
| Pedestrian Calls (#/hr) | | 0 | | | | | | 0 | | | | |
| Act Effct Green (s) | 36.0 | 23.5 | 36.9 | 33.5 | 31.9 | | 22.3 | 21.8 | | 24.3 | 16.3 | |
| ······································ | | | | | | | | | | • | | |

2022 Existing AM Peak Hour Timmons Group

Synchro 11 Report

Zebulon South TIA

| 2: NC-96 | (Arendell Avenue) |) & NC-97 (| (Gannon Avenue) |) |
|----------|-------------------|-------------|-----------------|---|
|----------|-------------------|-------------|-----------------|---|

12/04/2023

| | ٨ | → | 7 | 4 | + | * | 1 | 1 | 1 | 4 | ŧ | 4 |
|-------------------------------|-------------|------------|----------|-----------|------------|------------|------|------|-----------|------|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Actuated g/C Ratio | 0.48 | 0.31 | 0.49 | 0.45 | 0.42 | | 0.30 | 0.29 | | 0.32 | 0.22 | |
| v/c Ratio | 0.10 | 0.76 | 0.09 | 0.56 | 0.46 | | 0.23 | 0.61 | | 0.07 | 0.56 | |
| Control Delay | 12.7 | 36.0 | 13.3 | 19.2 | 22.7 | | 17.6 | 29.9 | | 15.7 | 33.6 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 12.7 | 36.0 | 13.3 | 19.2 | 22.7 | | 17.6 | 29.9 | | 15.7 | 33.6 | |
| LOS | В | D | В | В | С | | В | С | | В | С | |
| Approach Delay | | 31.2 | | | 21.4 | | | 27.3 | | | 31.9 | |
| Approach LOS | | С | | | С | | | С | | | С | |
| Queue Length 50th (ft) | 10 | 182 | 17 | 52 | 134 | | 27 | 115 | | 7 | 98 | |
| Queue Length 95th (ft) | 34 | #399 | 47 | 125 | 271 | | 54 | 233 | | 22 | 170 | |
| Internal Link Dist (ft) | | 1373 | | | 597 | | | 1742 | | | 398 | |
| Turn Bay Length (ft) | 200 | | 100 | 350 | | | 125 | | | 250 | | |
| Base Capacity (vph) | 518 | 636 | 1032 | 401 | 792 | | 592 | 849 | | 380 | 622 | |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Reduced v/c Ratio | 0.09 | 0.69 | 0.07 | 0.54 | 0.46 | | 0.14 | 0.37 | | 0.06 | 0.35 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: 0 | Other | | | | | | | | | | | |
| Cycle Length: 100 | | | | | | | | | | | | |
| Actuated Cycle Length: 75. | 1 | | | | | | | | | | | |
| Natural Cycle: 75 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | coordinated | b | | | | | | | | | | |
| Maximum v/c Ratio: 0.76 | | | | | | | | | | | | |
| Intersection Signal Delay: 2 | 7.2 | | | In | tersection | n LOS: C | | | | | | |
| Intersection Capacity Utiliza | ation 69.9% | 0 | | IC | U Level | of Service | ЭC | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | exceeds ca | apacity, c | lueue ma | y be long | jer. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 2: NC | -96 (Arend | lell Aven | ue) & NC | -97 (Gan | non Aver | nue) | | | | | | |
| 1 | | | | ì | | | | 1 | | | | |
| ▼Ø1 → 2 15 s 30 s | 02 | | | 25 : | A Ø3 | | | 30 s | 04 | | 2 | |

Ø7

ØS

Ø5

| Intersection | |
|------------------|-----|
| Int Delay, s/veh | 2.3 |

| Int Delay, 3/Ven | 2.0 | | | | | | | | | | | | |
|------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|--|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | | \$ | | | \$ | | | \$ | | | \$ | | |
| Traffic Vol, veh/h | 30 | 4 | 16 | 4 | 4 | 4 | 43 | 240 | 4 | 4 | 129 | 18 | |
| Future Vol, veh/h | 30 | 4 | 16 | 4 | 4 | 4 | 43 | 240 | 4 | 4 | 129 | 18 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None | |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - | |
| Veh in Median Storage | e, # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 33 | 4 | 18 | 4 | 4 | 4 | 48 | 267 | 4 | 4 | 143 | 20 | |
| | | | | | | | | | | | | | |

| Major/Minor | Minor2 | | 1 | Minor1 | | | Major1 | | | Major2 | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|---|--------|---|---|--|
| Conflicting Flow All | 530 | 528 | 153 | 537 | 536 | 269 | 163 | 0 | 0 | 271 | 0 | 0 | |
| Stage 1 | 161 | 161 | - | 365 | 365 | - | - | - | - | - | - | - | |
| Stage 2 | 369 | 367 | - | 172 | 171 | - | - | - | - | - | - | - | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - | 2.218 | - | - | |
| Pot Cap-1 Maneuver | 460 | 456 | 893 | 455 | 451 | 770 | 1416 | - | - | 1292 | - | - | |
| Stage 1 | 841 | 765 | - | 654 | 623 | - | - | - | - | - | - | - | |
| Stage 2 | 651 | 622 | - | 830 | 757 | - | - | - | - | - | - | - | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | |
| Mov Cap-1 Maneuver | 439 | 436 | 893 | 428 | 432 | 770 | 1416 | - | - | 1292 | - | - | |
| Mov Cap-2 Maneuver | • 439 | 436 | - | 428 | 432 | - | - | - | - | - | - | - | |
| Stage 1 | 807 | 763 | - | 628 | 598 | - | - | - | - | - | - | - | |
| Stage 2 | 617 | 597 | - | 806 | 755 | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | |

| Approach | EB | WB | NB | SB | |
|----------------------|------|------|-----|-----|--|
| HCM Control Delay, s | 12.7 | 12.3 | 1.1 | 0.2 | |
| HCM LOS | В | В | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1V | VBLn1 | SBL | SBT | SBR | |
|-----------------------|-------|-----|-----|--------|-------|-------|-----|-----|--|
| Capacity (veh/h) | 1416 | - | - | 524 | 504 | 1292 | - | - | |
| HCM Lane V/C Ratio | 0.034 | - | - | 0.106 | 0.026 | 0.003 | - | - | |
| HCM Control Delay (s) | 7.6 | 0 | - | 12.7 | 12.3 | 7.8 | 0 | - | |
| HCM Lane LOS | А | А | - | В | В | А | А | - | |
| HCM 95th %tile Q(veh) | 0.1 | - | - | 0.4 | 0.1 | 0 | - | - | |

Zebulon South TIA <u>6: NC-96 (Arendell Avenue) & Perry Curtis Road</u>

| Intersection | | | | | | |
|------------------------|--------|------|------|------|------|------|
| Int Delay, s/veh | 2.3 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | ţ, | | | ŧ |
| Traffic Vol, veh/h | 12 | 62 | 205 | 4 | 24 | 106 |
| Future Vol, veh/h | 12 | 62 | 205 | 4 | 24 | 106 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 13 | 69 | 228 | 4 | 27 | 118 |
| | | | | | | |

| Major/Minor | Minor1 | Ν | /lajor1 | N | Major2 | | J | | | | |
|----------------------|--------|-------|---------|----------|--------|-------|---|--|--|--|--|
| Conflicting Flow All | 402 | 230 | 0 | 0 | 232 | 0 | | | | | |
| Stage 1 | 230 | - | - | - | - | - | | | | | |
| Stage 2 | 172 | - | - | - | - | - | | | | | |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - | | | | | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | | | | | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | | | | | |
| Follow-up Hdwy | | 3.318 | - | - | 2.218 | - | | | | | |
| Pot Cap-1 Maneuver | 604 | 809 | - | - | 1336 | - | | | | | |
| Stage 1 | 808 | - | - | - | - | - | | | | | |
| Stage 2 | 858 | - | - | - | - | - | | | | | |
| Platoon blocked, % | | | - | - | | - | | | | | |
| Mov Cap-1 Maneuver | | 809 | - | - | 1336 | - | | | | | |
| Mov Cap-2 Maneuver | | - | - | - | - | - | | | | | |
| Stage 1 | 808 | - | - | - | - | - | | | | | |
| Stage 2 | 839 | - | - | - | - | - | | | | | |
| | | | | | | | | | | | |
| Approach | WB | | NB | | SB | | | | | | |
| HCM Control Delay, s | | | 0 | | 1.4 | | | | | | |
| HCM LOS | B | | v | | | | | | | | |
| | 5 | | | | | | | | | | |
| | | | | . | 0.01 | 0.0.7 | | | | | |
| Minor Lane/Major Mv | mt | NBT | NBRW | | SBL | SBT | | | | | |
| Capacity (veh/h) | | - | - | 763 | 1336 | - | | | | | |
| HCM Lane V/C Ratio | | - | - (| 0.108 | 0.02 | - | | | | | |
| HCM Control Delay (s | s) | - | - | 10.3 | 7.7 | 0 | | | | | |
| HCM Lane LOS | | - | - | В | Α | Α | | | | | |

0.4

-

-

0.1

-

HCM 95th %tile Q(veh)

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/04/2023

| | ٠ | + | 7 | 4 | + | * | 1 | 1 | 1 | 4 | ŧ | ~ |
|--|--------|-------|-------|--------|-------|--------|-------------|-------|-------|--------|-------------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | ef 🕺 | | 7 | ef 👘 | | | 4 | | | \$ | |
| Traffic Volume (vph) | 9 | 594 | 100 | 49 | 496 | 4 | 80 | 9 | 60 | 6 | 17 | 26 |
| Future Volume (vph) | 9 | 594 | 100 | 49 | 496 | 4 | 80 | 9 | 60 | 6 | 17 | 26 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.978 | | | 0.999 | | | 0.946 | | | 0.929 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.974 | | | 0.994 | |
| Satd. Flow (prot) | 1778 | 1831 | 0 | 1770 | 1861 | 0 | 0 | 1691 | 0 | 0 | 1712 | 0 |
| Flt Permitted | 0.405 | 1001 | Ū | 0.256 | 1001 | • | Ū | 0.803 | Ŭ | Ū | 0.948 | Ū |
| Satd. Flow (perm) | 758 | 1831 | 0 | 477 | 1861 | 0 | 0 | 1394 | 0 | 0 | 1632 | 0 |
| Right Turn on Red | 100 | 1001 | No | | 1001 | No | Ū | 1001 | No | Ū | 1002 | No |
| Satd. Flow (RTOR) | | | 110 | | | 110 | | | 110 | | | 110 |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 10 | 660 | 111 | 54 | 551 | 4 | 89 | 10 | 67 | 0.30 | 19 | 29 |
| Shared Lane Traffic (%) | 10 | 000 | | 54 | 551 | - | 03 | 10 | 01 | ' | 15 | 23 |
| Lane Group Flow (vph) | 10 | 771 | 0 | 54 | 555 | 0 | 0 | 166 | 0 | 0 | 55 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | Leit | 12 | Night | Leit | 12 | rtight | Len | 0 | Night | Leit | 0 | rugin |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | Yes | | | 10 | | | 10 | |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 | 0.99 | 0.99 | 1.00 | 1.00 | 9 | 1.02 | 1.02 | 9 | 1.01 | 1.01 | 9 |
| Turn Type | Perm | NA | 9 | Perm | NA | 9 | Perm | NA | 9 | Perm | NA | 9 |
| Protected Phases | Feilii | 2 | | Feilli | 6 | | Feilli | 8 | | Feilli | 4 | |
| | 0 | Z | | 6 | 0 | | 0 | 0 | | 1 | 4 | |
| Permitted Phases Detector Phase | 2 | 2 | | 6 6 | 6 | | 8 8 | 8 | | 4 | 4 | |
| Switch Phase | 2 | Z | | 0 | 0 | | 0 | 0 | | 4 | 4 | |
| | 10.0 | 10.0 | | 10.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Initial (s) Minimum Split (s) | 14.9 | 14.9 | | 14.8 | 14.8 | | 7.0 11.8 | 11.8 | | 11.9 | 7.0 11.9 | |
| | | | | | | | | | | | | |
| Total Split (s) | 45.0 | 45.0 | | 45.0 | 45.0 | | 20.0 | 20.0 | | 20.0 | 20.0 | |
| Total Split (%) | 69.2% | 69.2% | | 69.2% | 69.2% | | 30.8% | 30.8% | | 30.8% | 30.8% | |
| Maximum Green (s) | 40.1 | 40.1 | | 40.2 | 40.2 | | 15.2 | 15.2 | | 15.1 | 15.1 | |
| Yellow Time (s) | 3.9 | 3.9 | | 3.8 | 3.8 | | 3.7 | 3.7 | | 3.1 | 3.1 | |
| All-Red Time (s) | 1.0 | 1.0 | | 1.0 | 1.0 | | 1.1 | 1.1 | | 1.8 | 1.8 | |
| Lost Time Adjust (s) | 0.1 | 0.1 | | 0.2 | 0.2 | | | 0.2 | | | 0.1 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | _ |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | | Min | Min | | None | None | | None | None | |
| Act Effct Green (s) | 28.4 | 28.4 | | 28.4 | 28.4 | | | 10.0 | | | 10.1 | |
| Actuated g/C Ratio | 0.64 | 0.64 | | 0.64 | 0.64 | | | 0.23 | | | 0.23 | |
| L D C | 0.02 | 0.66 | | 0.18 | 0.46 | | | 0.53 | | | 0.15 | |
| v/c Ratio Control Delay | 5.0 | 10.8 | | 7.1 | 7.7 | | | 24.6 | | | 17.9 | |

2022 Existing PM Peak Hour Timmons Group

Synchro 11 Report

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/04/2023

| | ٨ | → | 1 | 4 | + | * | 1 | Ť | 1 | 4 | ţ | ~ |
|------------------------------|-------------|----------|-----|------|----------|------------|-----|------|-----|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 5.0 | 10.8 | | 7.1 | 7.7 | | | 24.6 | | | 17.9 | |
| LOS | А | В | | А | А | | | С | | | В | |
| Approach Delay | | 10.8 | | | 7.7 | | | 24.6 | | | 17.9 | |
| Approach LOS | | В | | | А | | | С | | | В | |
| Queue Length 50th (ft) | 1 | 122 | | 6 | 73 | | | 33 | | | 10 | |
| Queue Length 95th (ft) | 6 | 291 | | 24 | 170 | | | 113 | | | 44 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 664 | 1605 | | 418 | 1631 | | | 509 | | | 596 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.02 | 0.48 | | 0.13 | 0.34 | | | 0.33 | | | 0.09 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: | Other | | | | | | | | | | | |
| Cycle Length: 65 | | | | | | | | | | | | |
| Actuated Cycle Length: 44 | .3 | | | | | | | | | | | |
| Natural Cycle: 50 | | | | | | | | | | | | |
| Control Type: Actuated-Un | coordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.66 | | | | | | | | | | | | |
| Intersection Signal Delay: | | | | | | n LOS: B | | | | | | |
| Intersection Capacity Utiliz | ation 64.3% | 6 | | IC | CU Level | of Service | эC | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| | | | / . | | | | | | | | | |

Splits and Phases: 1: S Wakefield Street & NC-97 (Gannon Avenue)

| → _{Ø2} | Ø4 |
|------------------------|-------------|
| 45 s | 20 s |
| ₩ Ø6 | 1 ø8 |
| 45 s | 20 s |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/04/2023

| - | ٠ | - | ¥ | 4 | + | * | 1 | t | 1 | 1 | ţ | ~ |
|-------------------------------|-------|---------|------------|-------|---------|-------|------------|-------|-------|-------|-------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ٦ | ↑ | 1 | 3 | 4Î | | ٦ | f, | | 5 | Þ | |
| Traffic Volume (vph) | 59 | 313 | 64 | 218 | 439 | 51 | 70 | 272 | 137 | 45 | 177 | 44 |
| Future Volume (vph) | 59 | 313 | 64 | 218 | 439 | 51 | 70 | 272 | 137 | 45 | 177 | 44 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | 2% | | | -2% | | | -2% | | | 2% | |
| Storage Length (ft) | 200 | | 100 | 350 | | 0 | 125 | | 0 | 250 | | 0 |
| Storage Lanes | 1 | | 1 | 1 | | 0 | 1 | | 0 | 1 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | 0.850 | | 0.984 | 1.00 | 1.00 | 0.950 | 1.00 | 1.00 | 0.970 | 1.00 |
| Flt Protected | 0.950 | | 0.000 | 0.950 | 0.001 | | 0.950 | 0.000 | | 0.950 | 0.010 | |
| Satd. Flow (prot) | 1752 | 1844 | 1567 | 1787 | 1851 | 0 | 1787 | 1787 | 0 | 1752 | 1789 | 0 |
| Flt Permitted | 0.150 | | 1001 | 0.326 | 1001 | Ū | 0.496 | | Ū | 0.224 | | Ū |
| Satd. Flow (perm) | 277 | 1844 | 1567 | 613 | 1851 | 0 | 933 | 1787 | 0 | 413 | 1789 | 0 |
| Right Turn on Red | 211 | 1011 | No | 010 | 1001 | No | 000 | 1101 | No | 110 | 1100 | No |
| Satd. Flow (RTOR) | | | 110 | | | 110 | | | 110 | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 20 | | | 35 | |
| Link Distance (ft) | | 1453 | | | 677 | | | 1822 | | | 478 | |
| Travel Time (s) | | 28.3 | | | 13.2 | | | 62.1 | | | 9.3 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 66 | 348 | 71 | 242 | 488 | 57 | 78 | 302 | 152 | 50 | 197 | 49 |
| Shared Lane Traffic (%) | 00 | 540 | 11 | 272 | 400 | 51 | 70 | 502 | 152 | 50 | 131 | 45 |
| Lane Group Flow (vph) | 66 | 348 | 71 | 242 | 545 | 0 | 78 | 454 | 0 | 50 | 246 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | Leit | 12 | Right | Leit | 12 | Right | Leit | 12 | Right | Leit | 12 | Right |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| () | | Yes | | | Yes | | | 10 | | | Yes | |
| Two way Left Turn Lane | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Headway Factor | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | | 0.99 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | D.P+P | NIA | | D.P+P | NA | 9 | D.P+P | NA | 9 | D.P+P | NA | 9 |
| Turn Type Protected Phases | | NA 2 | pm+ov 3 | | NA 6 | | D.P+P 3 | | | | | |
| | 5 | Z | - | 1 | 0 | | | 8 | | 7 | 4 | |
| Permitted Phases | 6 | 0 | 2 | 2 | c | | 4 | 0 | | 8 | 4 | |
| Detector Phase | 5 | 2 | 3 | 1 | 6 | | 3 | 8 | | 7 | 4 | |
| Switch Phase | 7.0 | 10.0 | 7.0 | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Initial (s) | 7.0 | 10.0 | 7.0 | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 12.4 | 22.8 | 12.4 | 12.8 | 15.8 | | 12.4 | 22.5 | | 12.3 | 12.5 | |
| Total Split (s) | 15.0 | 30.0 | 25.0 | 15.0 | 30.0 | | 25.0 | 40.0 | | 15.0 | 30.0 | |
| Total Split (%) | 15.0% | 30.0% | 25.0% | 15.0% | 30.0% | | 25.0% | 40.0% | | 15.0% | 30.0% | |
| Maximum Green (s) | 9.6 | 24.2 | 19.6 | 9.2 | 24.2 | | 19.6 | 34.5 | | 9.7 | 24.5 | |
| Yellow Time (s) | 3.0 | 4.0 | 3.0 | 3.0 | 4.0 | | 3.0 | 4.0 | | 3.0 | 4.0 | |
| All-Red Time (s) | 2.4 | 1.8 | 2.4 | 2.8 | 1.8 | | 2.4 | 1.5 | | 2.3 | 1.5 | _ |
| Lost Time Adjust (s) | -0.4 | -0.8 | -0.4 | -0.8 | -0.8 | | -0.4 | -0.5 | | -0.3 | -0.5 | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| Lead/Lag | Lead | Lag | Lead | Lead | Lag | | Lead | Lag | | Lead | Lag | |
| Lead-Lag Optimize? | • • | • • | | | • • | | | • • | | • • | • • | _ |
| Vehicle Extension (s) | 2.0 | 3.0 | 2.0 | 2.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | None | Min | None | None | Min | | None | None | | None | None | |
| Walk Time (s) | | 7.0 | | | | | | 7.0 | | | | |
| Flash Dont Walk (s) | | 10.0 | | | | | | 10.0 | | | | |
| Pedestrian Calls (#/hr) | | 0 | | | | | | 0 | | | | |
| Act Effct Green (s) | 33.0 | 21.2 | 34.9 | 31.4 | 27.2 | | 28.8 | 26.0 | | 29.9 | 22.9 | |

2022 Existing PM Peak Hour Timmons Group

Synchro 11 Report

Zebulon South TIA

12/04/2023

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|--|-------------|------------|-------------|-----------|------------|------------|------|------|-----|------|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Actuated g/C Ratio | 0.41 | 0.27 | 0.44 | 0.39 | 0.34 | | 0.36 | 0.33 | | 0.37 | 0.29 | |
| v/c Ratio | 0.25 | 0.71 | 0.10 | 0.62 | 0.87 | | 0.18 | 0.78 | | 0.17 | 0.48 | |
| Control Delay | 18.4 | 38.4 | 17.4 | 26.0 | 46.7 | | 15.2 | 36.2 | | 15.3 | 28.2 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 18.4 | 38.4 | 17.4 | 26.0 | 46.7 | | 15.2 | 36.2 | | 15.3 | 28.2 | |
| LOS | В | D | В | С | D | | В | D | | В | С | |
| Approach Delay | | 32.6 | | | 40.3 | | | 33.1 | | | 26.0 | |
| Approach LOS | | С | | | D | | | С | | | С | |
| Queue Length 50th (ft) | 21 | 173 | 24 | 84 | ~306 | | 23 | 227 | | 15 | 107 | |
| Queue Length 95th (ft) | 52 | #321 | 56 | #175 | #599 | | 50 | 357 | | 35 | 184 | |
| Internal Link Dist (ft) | | 1373 | | | 597 | | | 1742 | | | 398 | |
| Turn Bay Length (ft) | 200 | | 100 | 350 | | | 125 | | | 250 | | |
| Base Capacity (vph) | 316 | 616 | 939 | 401 | 629 | | 633 | 836 | | 340 | 659 | |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Reduced v/c Ratio | 0.21 | 0.56 | 0.08 | 0.60 | 0.87 | | 0.12 | 0.54 | | 0.15 | 0.37 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: | Other | | | | | | | | | | | |
| Cycle Length: 100 | | | | | | | | | | | | |
| Actuated Cycle Length: 79 | .9 | | | | | | | | | | | |
| Natural Cycle: 90 | | | | | | | | | | | | |
| Control Type: Actuated-Un | coordinated | t | | | | | | | | | | |
| Maximum v/c Ratio: 0.87 | | | | | | | | | | | | |
| Intersection Signal Delay: 3 | 34.7 | | | | tersection | | | | | | | |
| Intersection Capacity Utiliz | ation 77.2% | 0 | | IC | U Level | of Service | e D | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| Volume exceeds capac | | | ically infi | nite. | | | | | | | | |
| Queue shown is maxim | | | | | | | | | | | | |
| # 95th percentile volume | exceeds ca | apacity, q | ueue ma | y be long | jer. | | | | | | | |

Queue shown is maximum after two cycles.

Splits and Phases: 2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

| Ø1 | | \$ Ø3 | | |
|------|---------|--------------|------|--|
| 15 s | 30 s | 25 s | 30 s | |
| | ≠ Ø6 | Ø7 | 08 | |
| 15 s | 30 s | 15 s 40 | S | |

PD-2024-01 Zebulon South TIA 3: NC-96 (Arendell Avenue) & Barbee Street

Intersection Int Delay, s/veh

| Int Delay, s/veh | 3.1 | | | | | | | | | | | |
|------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | \$ | | | \$ | | | \$ | | | \$ | |
| Traffic Vol, veh/h | 45 | 4 | 65 | 4 | 4 | 6 | 32 | 200 | 4 | 8 | 330 | 42 |
| Future Vol, veh/h | 45 | 4 | 65 | 4 | 4 | 6 | 32 | 200 | 4 | 8 | 330 | 42 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage | e, # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 50 | 4 | 72 | 4 | 4 | 7 | 36 | 222 | 4 | 9 | 367 | 47 |

| Major/Minor | Minor2 | | | Minor1 | | | Major1 | | I | /lajor2 | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|---|---------|---|---|--|
| Conflicting Flow All | 711 | 707 | 391 | 743 | 728 | 224 | 414 | 0 | 0 | 226 | 0 | 0 | |
| Stage 1 | 409 | 409 | - | 296 | 296 | - | - | - | - | - | - | - | |
| Stage 2 | 302 | 298 | - | 447 | 432 | - | - | - | - | - | - | - | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - | 2.218 | - | - | |
| Pot Cap-1 Maneuver | 348 | 360 | 658 | 331 | 350 | 815 | 1145 | - | - | 1342 | - | - | |
| Stage 1 | 619 | 596 | - | 712 | 668 | - | - | - | - | - | - | - | |
| Stage 2 | 707 | 667 | - | 591 | 582 | - | - | - | - | - | - | - | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | |
| Mov Cap-1 Maneuver | 330 | 344 | 658 | 282 | 334 | 815 | 1145 | - | - | 1342 | - | - | |
| Mov Cap-2 Maneuver | 330 | 344 | - | 282 | 334 | - | - | - | - | - | - | - | |
| Stage 1 | 597 | 591 | - | 686 | 644 | - | - | - | - | - | - | - | |
| Stage 2 | 671 | 643 | - | 517 | 577 | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | |

| Approach | EB | WB | NB | SB | |
|----------------------|------|------|-----|-----|--|
| HCM Control Delay, s | 15.7 | 13.9 | 1.1 | 0.2 | |
| HCM LOS | С | В | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1V | VBLn1 | SBL | SBT | SBR |
|-----------------------|-------|-----|-----|--------|-------|-------|-----|-----|
| Capacity (veh/h) | 1145 | - | - | 462 | 418 | 1342 | - | - |
| HCM Lane V/C Ratio | 0.031 | - | - | 0.274 | 0.037 | 0.007 | - | - |
| HCM Control Delay (s) | 8.2 | 0 | - | 15.7 | 13.9 | 7.7 | 0 | - |
| HCM Lane LOS | А | А | - | С | В | А | А | - |
| HCM 95th %tile Q(veh) | 0.1 | - | - | 1.1 | 0.1 | 0 | - | - |

Zebulon South TIA 6: NC-96 (Arendell Avenue) & Perry Curtis Road

| 12/04/2023 | |
|------------|--|
|------------|--|

| Int Delay, s/veh | 2.2 | | | | | | |
|------------------------|------|------|------|------|------|------|---|
| Movement | WBL | WBR | NBT | NBR | SBL | SBT | Г |
| Lane Configurations | Y | | 4 | | | ŧ | 1 |
| Traffic Vol, veh/h | 5 | 42 | 149 | 6 | 96 | 250 |) |
| Future Vol, veh/h | 5 | 42 | 149 | 6 | 96 | 250 |) |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |) |
| Sign Control | Stop | Stop | Free | Free | Free | Free | Э |
| RT Channelized | - | None | - | None | - | None | Э |
| Storage Length | 0 | - | - | - | - | - | - |
| Veh in Median Storage | ,#0 | - | 0 | - | - | 0 | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |) |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 6 | 47 | 166 | 7 | 107 | 278 | 8 |

| Major/Minor | Minor1 | Ν | /lajor1 | N | Major2 | |
|----------------------|--------|-------|---------|-----|--------|-----|
| Conflicting Flow All | 662 | 170 | 0 | 0 | 173 | 0 |
| Stage 1 | 170 | - | - | - | - | - |
| Stage 2 | 492 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 427 | 874 | - | - | 1404 | - |
| Stage 1 | 860 | - | - | - | - | - |
| Stage 2 | 615 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | | 874 | - | - | 1404 | - |
| Mov Cap-2 Maneuver | | - | - | - | - | - |
| Stage 1 | 860 | - | - | - | - | - |
| Stage 2 | 560 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | | | 0 | | 2.2 | |
| HCM LOS | B | | Ŭ | | | |
| | _ | | | | | |
| | | NDT | | | 0.01 | ODT |
| Minor Lane/Major Mv | mt | NBT | NBRW | | SBL | SBT |
| Capacity (veh/h) | | - | - | 772 | 1404 | - |
| HCM Lane V/C Ratio | | - | - | | 0.076 | - |
| HCM Control Delay (s | 5) | - | - | 10 | 7.8 | 0 |
| HCM Lane LOS | | - | - | В | A | А |

-

_

0.2

0.2

-

HCM 95th %tile Q(veh)

2026 Background Traffic Volumes

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/01/2023

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|----------------------------|-------|-------|------------|--------------|-------|-------|-------|-------|------------|-------|-------|------------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | ef. | | 2 | ef. | | | \$ | | | \$ | |
| Traffic Volume (vph) | 6 | 482 | 41 | 46 | 517 | 4 | 91 | 10 | 113 | 4 | 5 | 19 |
| Future Volume (vph) | 6 | 482 | 41 | 46 | 517 | 4 | 91 | 10 | 113 | 4 | 5 | 19 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.988 | | | 0.999 | | | 0.929 | | | 0.909 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.979 | | | 0.994 | |
| Satd. Flow (prot) | 1778 | 1850 | 0 | 1770 | 1861 | 0 | 0 | 1669 | 0 | 0 | 1675 | 0 |
| Flt Permitted | 0.447 | | | 0.950 | | | | 0.848 | | | 0.951 | |
| Satd. Flow (perm) | 837 | 1850 | 0 | 1770 | 1861 | 0 | 0 | 1445 | 0 | 0 | 1602 | 0 |
| Right Turn on Red | | | No | | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 7 | 536 | 46 | 51 | 574 | 4 | 101 | 11 | 126 | 4 | 6 | 21 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 7 | 582 | 0 | 51 | 578 | 0 | 0 | 238 | 0 | 0 | 31 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | | 12 | J - | | 12 | J. | | 0 | J - | | 0 | J - |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | Yes | | | | | | | |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 | | 9 | 15 | | 9 | 15 | | 9 | 15 | | 9 |
| Turn Type | Perm | NA | | Prot | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 8 | | | 4 | |
| Permitted Phases | 2 | | | | | | 8 | | | 4 | | |
| Detector Phase | 2 | 2 | | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | | | | | | | | | | | | |
| Minimum Initial (s) | 10.0 | 10.0 | | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 17.0 | 17.0 | | 14.0 | 17.0 | | 14.0 | 14.0 | | 14.0 | 14.0 | |
| Total Split (s) | 47.0 | 47.0 | | 14.0 | 61.0 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Total Split (%) | 52.2% | 52.2% | | 15.6% | 67.8% | | 32.2% | 32.2% | | 32.2% | 32.2% | |
| Maximum Green (s) | 40.0 | 40.0 | | 7.0 | 54.0 | | 22.0 | 22.0 | | 22.0 | 22.0 | |
| Yellow Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | | -2.0 | -2.0 | | | -2.0 | | | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | | None | Min | | None | None | | None | None | |
| Act Effct Green (s) | 28.1 | 28.1 | | 10.3 | 34.8 | | | 17.6 | | | 17.6 | |
| Actuated g/C Ratio | 0.44 | 0.44 | | 0.16 | 0.55 | | | 0.28 | | | 0.28 | |
| v/c Ratio | 0.02 | 0.72 | | 0.18 | 0.57 | | | 0.60 | | | 0.07 | |
| Control Delay | 13.3 | 22.4 | | 34.7 | 11.5 | | | 31.3 | | | 23.3 | |
| | .0.0 | | | V 1.1 | | | | 01.0 | | | 20.0 | |

2026 Background AM Peak Hour Timmons Group

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/01/2023

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|------------------------------|--------------|------------|----------|----------|---------|------------|-----|------|-----|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 13.3 | 22.4 | | 34.7 | 11.5 | | | 31.3 | | | 23.3 | |
| LOS | В | С | | С | В | | | С | | | С | |
| Approach Delay | | 22.3 | | | 13.4 | | | 31.3 | | | 23.3 | |
| Approach LOS | | С | | | В | | | С | | | С | |
| Queue Length 50th (ft) | 2 | 213 | | 20 | 129 | | | 92 | | | 10 | |
| Queue Length 95th (ft) | 10 | 368 | | 63 | 241 | | | 199 | | | 35 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 598 | 1322 | | 286 | 1526 | | | 622 | | | 690 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.01 | 0.44 | | 0.18 | 0.38 | | | 0.38 | | | 0.04 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 63 | 3.8 | | | | | | | | | | | |
| Natural Cycle: 60 | | | | | | | | | | | | |
| Control Type: Actuated-U | ncoordinate | b | | | | | | | | | | |
| Maximum v/c Ratio: 0.72 | | | | | | | | | | | | |
| Intersection Signal Delay: | | | | | | n LOS: B | | | | | | |
| Intersection Capacity Utiliz | zation 65.7% | 0 | | IC | U Level | of Service | эC | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| Splits and Phases: 1: S | Wakefield S | Street & N | IC-97 (G | annon Av | venue) | | | | | | | |

Splits and Phases: 1: S Wakefield Street & NC-97 (Gannon Avenue)

| Ø1 | A 102 | Ø4 |
|----------------|-------|----------------------|
| 14 s | 47 s | 29 s |
| ← Ø6 | | √1 <i>ø</i> 8 |
| 61s | | 29 s |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/01/2023

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|--|-------|---------|-------|-------|---------|-------|-------|---------|-------|-------|---------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ٦ | 1 | 1 | 7 | ¢Î | | ۲ | Þ | | ٦ | ¢Î | |
| Traffic Volume (vph) | 46 | 446 | 70 | 221 | 349 | 19 | 86 | 176 | 138 | 25 | 190 | 33 |
| Future Volume (vph) | 46 | 446 | 70 | 221 | 349 | 19 | 86 | 176 | 138 | 25 | 190 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | 2% | | | -2% | | | -2% | | | 2% | |
| Storage Length (ft) | 200 | | 100 | 350 | | 0 | 125 | | 0 | 250 | | 0 |
| Storage Lanes | 1 | | 1 | 1 | | 0 | 1 | | 0 | 1 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | 0.850 | | 0.992 | | | 0.934 | | | 0.978 | |
| | 0.950 | | | 0.950 | | | 0.950 | | | 0.950 | | |
| · · · · · | 1752 | 1844 | 1567 | 1787 | 1866 | 0 | 1787 | 1757 | 0 | 1752 | 1804 | 0 |
| | 0.522 | | | 0.950 | | | 0.481 | | | 0.314 | | |
| Satd. Flow (perm) | 963 | 1844 | 1567 | 1787 | 1866 | 0 | 905 | 1757 | 0 | 579 | 1804 | 0 |
| Right Turn on Red | | | No | | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 20 | | | 35 | |
| Link Distance (ft) | | 1453 | | | 677 | | | 1822 | | | 478 | |
| Travel Time (s) | | 28.3 | | | 13.2 | | | 62.1 | | | 9.3 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 51 | 496 | 78 | 246 | 388 | 21 | 96 | 196 | 153 | 28 | 211 | 37 |
| Shared Lane Traffic (%) | 54 | 100 | 70 | 0.10 | 400 | 0 | 00 | 0.40 | • | 00 | 0.40 | 0 |
| Lane Group Flow (vph) | 51 | 496 | 78 | 246 | 409 | 0 | 96 | 349 | 0 | 28 | 248 | 0 |
| Enter Blocked Intersection | No | No | No |
| Lane Alignment | Left | Left | Right |
| Median Width(ft) | | 12 | | | 12 | | | 12 | | | 12 | |
| Link Offset(ft) | | 0 16 | | | 0 16 | | | 0 16 | | | 0 16 | |
| Crosswalk Width(ft) | | Yes | | | Yes | | | 10 | | | Yes | |
| Two way Left Turn Lane Headway Factor | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | 0.99 | 15 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| | Perm | NA | Perm | Prot | NA | 9 | Perm | NA | 3 | Perm | NA | 9 |
| Protected Phases | r enn | 2 | r enn | 1 | 6 | | r enn | 8 | | r enn | 4 | |
| Permitted Phases | 2 | 2 | 2 | 1 | 0 | | 8 | 0 | | 4 | - T | |
| Detector Phase | 2 | 2 | 2 | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | 2 | 2 | 2 | 1 | U | | U | U | | Т | | |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 24.0 | 24.0 | 24.0 | 14.0 | 17.0 | | 24.0 | 24.0 | | 14.0 | 14.0 | |
| Total Split (s) | 38.0 | 38.0 | 38.0 | 23.0 | 61.0 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| | 12.2% | 42.2% | 42.2% | 25.6% | 67.8% | | 32.2% | 32.2% | | 32.2% | 32.2% | |
| Maximum Green (s) | 31.0 | 31.0 | 31.0 | 16.0 | 54.0 | | 22.0 | 22.0 | | 22.0 | 22.0 | |
| Yellow Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | | -2.0 | -2.0 | | -2.0 | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| Lead/Lag | Lag | Lag | Lag | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 2.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | Min | None | Min | | None | None | | None | None | |
| Walk Time (s) | 7.0 | 7.0 | 7.0 | | | | 7.0 | 7.0 | | | | |
| Flash Dont Walk (s) | 10.0 | 10.0 | 10.0 | | | | 10.0 | 10.0 | | | | |
| . / | | | | | | | 0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | 0 | 0 | | | | U | 0 | | | | |

2026 Background AM Peak Hour Timmons Group

Zebulon South TIA

| 2: NC-96 (Arendell Avenue) |) & NC-97 (| (Gannon Avenue) |
|----------------------------|-------------|-----------------|
|----------------------------|-------------|-----------------|

12/01/2023

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|-------------------------------|-------------|------------|----------|------------|------------|------------|------|------|-----|------|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBF |
| Actuated g/C Ratio | 0.34 | 0.34 | 0.34 | 0.20 | 0.61 | | 0.26 | 0.26 | | 0.26 | 0.26 | |
| v/c Ratio | 0.16 | 0.79 | 0.15 | 0.69 | 0.36 | | 0.41 | 0.76 | | 0.19 | 0.52 | |
| Control Delay | 20.7 | 34.6 | 19.9 | 42.3 | 9.1 | | 32.4 | 40.2 | | 28.8 | 31.0 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 20.7 | 34.6 | 19.9 | 42.3 | 9.1 | | 32.4 | 40.2 | | 28.8 | 31.0 | |
| LOS | С | С | В | D | А | | С | D | | С | С | |
| Approach Delay | | 31.6 | | | 21.6 | | | 38.6 | | | 30.8 | |
| Approach LOS | | С | | | С | | | D | | | С | |
| Queue Length 50th (ft) | 19 | 239 | 29 | 123 | 100 | | 42 | 171 | | 12 | 113 | |
| Queue Length 95th (ft) | 45 | 360 | 60 | #226 | 155 | | 92 | #304 | | 36 | 195 | |
| Internal Link Dist (ft) | | 1373 | | | 597 | | | 1742 | | | 398 | |
| Turn Bay Length (ft) | 200 | | 100 | 350 | | | 125 | | | 250 | | |
| Base Capacity (vph) | 417 | 798 | 678 | 422 | 1370 | | 285 | 553 | | 182 | 568 | |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Reduced v/c Ratio | 0.12 | 0.62 | 0.12 | 0.58 | 0.30 | | 0.34 | 0.63 | | 0.15 | 0.44 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: (| Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 78.8 | 8 | | | | | | | | | | | |
| Natural Cycle: 65 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | coordinated | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.79 | | | | | | | | | | | | |
| Intersection Signal Delay: 2 | 9.8 | | | In | tersection | n LOS: C | | | | | | |
| Intersection Capacity Utiliza | ation 75.9% | , 0 | | IC | U Level | of Service | e D | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | exceeds ca | apacity, q | lueue ma | iy be long | jer. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 2: NC | -96 (Arend | lell Aveni | ue) & NC | -97 (Gan | non Aver | nue) | | | | | | |
| 1 | | | | 5. (Our | | | | ~ | | | | |
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| Intersection | |
|------------------|---|
| Int Delay, s/veh | 2 |

| Intersection | | | | | | | | | | | | | |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|--|
| Int Delay, s/veh | 2.3 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | | \$ | | | \$ | | | \$ | | | \$ | | |
| Traffic Vol, veh/h | 34 | 4 | 18 | 4 | 4 | 4 | 48 | 270 | 4 | 4 | 145 | 20 | |
| Future Vol, veh/h | 34 | 4 | 18 | 4 | 4 | 4 | 48 | 270 | 4 | 4 | 145 | 20 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | |
| RT Channelized | - | - | None | |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - | |
| Veh in Median Storage, | ,# - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 38 | 4 | 20 | 4 | 4 | 4 | 53 | 300 | 4 | 4 | 161 | 22 | |
| | | | | | | | | | | | | | |

| Major/Minor | Minor2 | | 1 | Minor1 | | | Major1 | | | Major2 | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|---|--------|---|---|--|
| Conflicting Flow All | 592 | 590 | 172 | 600 | 599 | 302 | 183 | 0 | 0 | 304 | 0 | 0 | |
| Stage 1 | 180 | 180 | - | 408 | 408 | - | - | - | - | - | - | - | |
| Stage 2 | 412 | 410 | - | 192 | 191 | - | - | - | - | - | - | - | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - | 2.218 | - | - | |
| Pot Cap-1 Maneuver | 418 | 420 | 872 | 413 | 415 | 738 | 1392 | - | - | 1257 | - | - | |
| Stage 1 | 822 | 750 | - | 620 | 597 | - | - | - | - | - | - | - | |
| Stage 2 | 617 | 595 | - | 810 | 742 | - | - | - | - | - | - | - | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | |
| Mov Cap-1 Maneuver | r 396 | 399 | 872 | 385 | 394 | 738 | 1392 | - | - | 1257 | - | - | |
| Mov Cap-2 Maneuver | r 396 | 399 | - | 385 | 394 | - | - | - | - | - | - | - | |
| Stage 1 | 784 | 747 | - | 591 | 570 | - | - | - | - | - | - | - | |
| Stage 2 | 581 | 568 | - | 784 | 739 | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | |

| Approach | EB | WB | NB | SB | |
|----------------------|------|----|-----|-----|--|
| HCM Control Delay, s | 13.6 | 13 | 1.1 | 0.2 | |
| HCM LOS | В | В | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1V | VBLn1 | SBL | SBT | SBR | |
|-----------------------|-------|-----|-----|--------|-------|-------|-----|-----|--|
| Capacity (veh/h) | 1392 | - | - | 481 | 462 | 1257 | - | - | |
| HCM Lane V/C Ratio | 0.038 | - | - | 0.129 | 0.029 | 0.004 | - | - | |
| HCM Control Delay (s) | 7.7 | 0 | - | 13.6 | 13 | 7.9 | 0 | - | |
| HCM Lane LOS | А | А | - | В | В | А | А | - | |
| HCM 95th %tile Q(veh) | 0.1 | - | - | 0.4 | 0.1 | 0 | - | - | |

Zebulon South TIA 6: NC-96 (Arendell Avenue) & Perry Curtis Road

| Intersection | | | | | | | |
|------------------------|--------|------|------|------|------|------|----------|
| Int Delay, s/veh | 2.4 | | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT | - |
| Lane Configurations | Y | | Þ | | | ÷. | 1 |
| Traffic Vol, veh/h | 14 | 70 | 231 | 4 | 27 | 119 |) |
| Future Vol, veh/h | 14 | 70 | 231 | 4 | 27 | 119 |) |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |) |
| Sign Control | Stop | Stop | Free | Free | Free | Free | e |
| RT Channelized | - | None | - | None | - | None | ę |
| Storage Length | 0 | - | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |) |
| Grade, % | 0 | - | 0 | - | - | 0 |) |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |) |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | <u>,</u> |
| Mvmt Flow | 16 | 78 | 257 | 4 | 30 | 132 | > |
| | | | | | | | |

| Major/Minor | Minor1 | Ν | /lajor1 | 1 | Major2 | |
|----------------------|-------------|-------|---------|--------|--------|-----|
| Conflicting Flow All | 451 | 259 | 0 | 0 | 261 | 0 |
| Stage 1 | 259 | - | - | - | - | - |
| Stage 2 | 192 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 566 | 780 | - | - | 1303 | - |
| Stage 1 | 784 | - | - | - | - | - |
| Stage 2 | 841 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | | 780 | - | - | 1303 | - |
| Mov Cap-2 Maneuver | | - | - | - | - | - |
| Stage 1 | 784 | - | - | - | - | - |
| Stage 2 | 820 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | 10.7 | | 0 | | 1.4 | |
| HCM LOS | В | | - | | | |
| | | | | | | |
| Minor Lane/Major Mv | mt | NBT | NBRW | 'Bl n1 | SBL | SBT |
| | IIII | IDI | NDRW | | | |
| Capacity (veh/h) | | - | - | 730 | 1303 | - |
| HCM Lane V/C Ratio | | - | - (| | 0.023 | - |
| HCM Control Delay (s | 5) | - | - | 10.7 | 7.8 | 0 |
| HCM Lane LOS | | - | - | В | А | А |

HCM 95th %tile Q(veh)

-

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0.4

0.1

-

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/01/2023

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|--------------------------------------|-------------|--------------|-------|-------|-------|-------|--------|---------|-------|-------|----------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | f. | | 7 | f, | | | \$ | | | \$ | |
| Traffic Volume (vph) | 10 | 669 | 113 | 55 | 558 | 4 | 90 | 10 | 68 | 7 | 19 | 29 |
| Future Volume (vph) | 10 | 669 | 113 | 55 | 558 | 4 | 90 | 10 | 68 | 7 | 19 | 29 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.978 | | | 0.999 | | | 0.945 | | | 0.929 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.974 | | | 0.993 | |
| Satd. Flow (prot) | 1778 | 1831 | 0 | 1770 | 1861 | 0 | 0 | 1689 | 0 | 0 | 1710 | 0 |
| Flt Permitted | 0.428 | | | 0.950 | | | | 0.813 | | | 0.952 | |
| Satd. Flow (perm) | 801 | 1831 | 0 | 1770 | 1861 | 0 | 0 | 1410 | 0 | 0 | 1639 | 0 |
| Right Turn on Red | | | No | | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 11 | 743 | 126 | 61 | 620 | 4 | 100 | 11 | 76 | 8 | 21 | 32 |
| Shared Lane Traffic (%) | | | - | | | - | - | | - | - | . | |
| Lane Group Flow (vph) | 11 | 869 | 0 | 61 | 624 | 0 | 0 | 187 | 0 | 0 | 61 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | | 12 | | | 12 | | | 0 | | | 0 | |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | 0.00 | Yes | 0.00 | 4.00 | Yes | 4 00 | 4.00 | 4.00 | 4.00 | 4.04 | 4.04 | 4.04 |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 | NIA | 9 | 15 | NIA | 9 | 15 | NLA | 9 | 15 | NLA | 9 |
| Turn Type Protected Phases | Perm | NA 2 | | Prot | NA | | Perm | NA 8 | | Perm | NA | |
| Protected Phases Permitted Phases | 0 | Z | | 1 | 6 | | 0 | 0 | | 1 | 4 | |
| Detector Phase | 2 2 | 2 | | 1 | 6 | | 8 8 | 8 | | 4 | 1 | |
| Switch Phase | 2 | Z | | 1 | 0 | | 0 | 0 | | 4 | 4 | |
| Minimum Initial (s) | 10.0 | 10.0 | | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 17.0 | 17.0 | | 14.0 | 17.0 | | 14.0 | 14.0 | | 14.0 | 14.0 | |
| Total Split (s) | 56.0 | 56.0 | | 14.0 | 70.0 | | 20.0 | 20.0 | | 20.0 | 20.0 | |
| Total Split (%) | 62.2% | 62.2% | | 15.6% | 77.8% | | 20.0 | 20.0 | | 22.2% | 20.0 | |
| Maximum Green (s) | 49.0 | 49.0 | | 7.0 | 63.0 | | 13.0 | 13.0 | | 13.0 | 13.0 | |
| Yellow Time (s) | 49.0 5.0 | 49.0 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | | -2.0 | -2.0 | | 2.0 | -2.0 | | 2.0 | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | |
| Lead/Lag | Lag | Lag | | Lead | 0.0 | | | 0.0 | | | 0.0 | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | | None | Min | | None | None | | None | None | |
| Act Effct Green (s) | 45.2 | 45.2 | | 9.3 | 55.7 | | | 14.3 | | | 14.3 | |
| Actuated g/C Ratio | 0.56 | 43.2 0.56 | | 0.12 | 0.69 | | | 0.18 | | | 0.18 | |
| v/c Ratio | 0.00 | 0.84 | | 0.12 | 0.09 | | | 0.75 | | | 0.10 | |
| Control Delay | 8.9 | 25.3 | | 41.2 | 6.9 | | | 54.1 | | | 33.8 | |
| | 0.0 | 20.0 | | 71.4 | 0.5 | | | J. I | | | 00.0 | |

2026 Background PM Peak Hour Timmons Group

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/01/2023

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|-------------------------------|-------------|------------|----------|-----------|------------|------------|-----|------|-----|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 8.9 | 25.3 | | 41.2 | 6.9 | | | 54.1 | | | 33.8 | |
| LOS | А | С | | D | А | | | D | | | С | |
| Approach Delay | | 25.1 | | | 10.0 | | | 54.1 | | | 33.8 | |
| Approach LOS | | С | | | А | | | D | | | С | |
| Queue Length 50th (ft) | 3 | 380 | | 32 | 124 | | | 99 | | | 29 | |
| Queue Length 95th (ft) | 10 | #602 | | 72 | 184 | | | #213 | | | 67 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 524 | 1200 | | 204 | 1497 | | | 272 | | | 316 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.02 | 0.72 | | 0.30 | 0.42 | | | 0.69 | | | 0.19 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 80. | 3 | | | | | | | | | | | |
| Natural Cycle: 80 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | coordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.84 | | | | | | | | | | | | |
| Intersection Signal Delay: 2 | | | | | tersection | | | | | | | |
| Intersection Capacity Utiliza | ation 70.4% | , 0 | | IC | U Level | of Service | эC | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | | | lueue ma | y be long | jer. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 1: S V | Vakefield S | Street & N | IC-97 (G | annon Av | (enue) | | | | | | | |
| | 72 | | | | | | | | N | 04 | | |

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| 14 s | 56 s | 20 s |
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| 70 s | | 20 s |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/01/2023

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2026 Background PM Peak Hour Timmons Group

Zebulon South TIA

| 2: NC-96 | (Arendell Avenue) |) & NC-97 (| (Gannon Avenue) |) |
|----------|-------------------|-------------|-----------------|---|
|----------|-------------------|-------------|-----------------|---|

12/01/2023

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|-------------------------------|-------------|------------|----------|------------|----------|------------|------|------|-----|------|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBF |
| Actuated g/C Ratio | 0.27 | 0.27 | 0.27 | 0.20 | 0.53 | | 0.34 | 0.34 | | 0.34 | 0.34 | |
| v/c Ratio | 0.33 | 0.78 | 0.19 | 0.77 | 0.62 | | 0.28 | 0.84 | | 0.46 | 0.45 | |
| Control Delay | 30.2 | 40.1 | 25.4 | 49.6 | 17.3 | | 23.6 | 39.4 | | 36.1 | 24.5 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 30.2 | 40.1 | 25.4 | 49.6 | 17.3 | | 23.6 | 39.4 | | 36.1 | 24.5 | |
| LOS | С | D | С | D | В | | С | D | | D | С | |
| Approach Delay | | 36.6 | | | 27.2 | | | 37.1 | | | 26.4 | |
| Approach LOS | | D | | | С | | | D | | | С | |
| Queue Length 50th (ft) | 32 | 200 | 34 | 148 | 229 | | 35 | 256 | | 24 | 117 | |
| Queue Length 95th (ft) | 72 | #309 | 70 | #276 | 339 | | 74 | #422 | | 66 | 189 | |
| Internal Link Dist (ft) | | 1373 | | | 597 | | | 1742 | | | 398 | |
| Turn Bay Length (ft) | 200 | | 100 | 350 | | | 125 | | | 250 | | |
| Base Capacity (vph) | 259 | 598 | 508 | 379 | 1109 | | 367 | 713 | | 146 | 714 | |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Reduced v/c Ratio | 0.28 | 0.65 | 0.16 | 0.72 | 0.55 | | 0.24 | 0.72 | | 0.39 | 0.39 | |
| Intersection Summary | | | | | | | | | | | | |
| 71 | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 82 | | | | | | | | | | | | |
| Natural Cycle: 70 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | coordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.84 | | | | | | | | | | | | |
| Intersection Signal Delay: 3 | | | | | | n LOS: C | | | | | | |
| Intersection Capacity Utiliza | ation 85.8% | 6 | | IC | U Level | of Service | θE | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | | | ueue ma | iy be long | jer. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 2: NC | -96 (Arend | dell Aveni | ue) & NC | -97 (Gan | non Aver | nue) | | | | | | |
| √ Ø1 | 2 | 02 | | | | | Ø4 | | | | | |
| 22 s | 31 s | ~~ | | | | 37 | s | | | | | |
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| Ø6 | | | | | | | Ø8 | | | | | |

| Intersection | |
|------------------|-----|
| Int Delay, s/veh | 3.5 |

| Intersection | | | | | | | | | | | | | |
|------------------------|-------|------|------|------|------|------|------|------|------|------|------|------|--|
| Int Delay, s/veh | 3.5 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | | |
| Traffic Vol, veh/h | 51 | 5 | 73 | 4 | 4 | 7 | 36 | 225 | 4 | 9 | 371 | 47 | |
| Future Vol, veh/h | 51 | 5 | 73 | 4 | 4 | 7 | 36 | 225 | 4 | 9 | 371 | 47 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None | |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - | |
| Veh in Median Storage | , # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 57 | 6 | 81 | 4 | 4 | 8 | 40 | 250 | 4 | 10 | 412 | 52 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |

| Major/Minor | Minor2 | | | Vinor1 | | | Major1 | | | Major2 | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|---|--------|---|---|--|
| Conflicting Flow All | 796 | 792 | 438 | 834 | 816 | 252 | 464 | 0 | 0 | 254 | 0 | 0 | |
| Stage 1 | 458 | 458 | - | 332 | 332 | - | - | - | - | - | - | - | |
| Stage 2 | 338 | 334 | - | 502 | 484 | - | - | - | - | - | - | - | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - | 2.218 | - | - | |
| Pot Cap-1 Maneuver | 305 | 322 | 619 | 288 | 311 | 787 | 1097 | - | - | 1311 | - | - | |
| Stage 1 | 583 | 567 | - | 681 | 644 | - | - | - | - | - | - | - | |
| Stage 2 | 676 | 643 | - | 552 | 552 | - | - | - | - | - | - | - | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | |
| Mov Cap-1 Maneuver | 287 | 305 | 619 | 237 | 295 | 787 | 1097 | - | - | 1311 | - | - | |
| Mov Cap-2 Maneuver | 287 | 305 | - | 237 | 295 | - | - | - | - | - | - | - | |
| Stage 1 | 559 | 561 | - | 652 | 617 | - | - | - | - | - | - | - | |
| Stage 2 | 637 | 616 | - | 470 | 546 | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | |

| Approach | EB | WB | NB | SB | |
|----------------------|------|------|-----|-----|--|
| HCM Control Delay, s | 18.3 | 14.9 | 1.1 | 0.2 | |
| HCM LOS | С | В | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1V | VBLn1 | SBL | SBT | SBR |
|-----------------------|-------|-----|-----|--------|-------|-------|-----|-----|
| Capacity (veh/h) | 1097 | - | - | 413 | 381 | 1311 | - | - |
| HCM Lane V/C Ratio | 0.036 | - | - | 0.347 | 0.044 | 0.008 | - | - |
| HCM Control Delay (s) | 8.4 | 0 | - | 18.3 | 14.9 | 7.8 | 0 | - |
| HCM Lane LOS | А | А | - | С | В | А | А | - |
| HCM 95th %tile Q(veh) | 0.1 | - | - | 1.5 | 0.1 | 0 | - | - |

Zebulon South TIA 6: NC-96 (Arendell Avenue) & Perry Curtis Road

| Intersection | • * | | | | | |
|------------------------|--------|------|------|------|------|------|
| Int Delay, s/veh | 2.3 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | ħ | | | ÷. |
| Traffic Vol, veh/h | 6 | 47 | 168 | 7 | 108 | 281 |
| Future Vol, veh/h | 6 | 47 | 168 | 7 | 108 | 281 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 7 | 52 | 187 | 8 | 120 | 312 |
| | | | | | | |

| Major/Minor | Minor1 | Ν | /lajor1 | | Major2 | |
|----------------------|--------|-------|---------|-------|--------|-----|
| Conflicting Flow All | 743 | 191 | 0 | 0 | 195 | 0 |
| Stage 1 | 191 | - | - | - | - | - |
| Stage 2 | 552 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 383 | 851 | - | - | 1378 | - |
| Stage 1 | 841 | - | - | - | - | - |
| Stage 2 | 577 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | 343 | 851 | - | - | 1378 | - |
| Mov Cap-2 Maneuver | 343 | - | - | - | - | - |
| Stage 1 | 841 | - | - | - | - | - |
| Stage 2 | 516 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | | | 0 | | 2.2 | |
| HCM LOS | B | | U | | 2.2 | |
| | U | | | | | |
| | | | | | | |
| Minor Lane/Major Mv | mt | NBT | NBRW | 'BLn1 | SBL | SBT |
| Capacity (veh/h) | | - | - | 729 | 1378 | - |
| HCM Lane V/C Ratio | | - | - (| | 0.087 | - |
| HCM Control Delay (s | 5) | - | - | 10.4 | 7.9 | 0 |
| HCM Lane LOS | | - | - | В | А | А |

0.3

-

-

0.3

-

HCM 95th %tile Q(veh)

2026 Build Traffic Volumes

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/01/2023

| | ٠ | - | 7 | * | + | * | 1 | 1 | 1 | 1 | Ŧ | 4 |
|----------------------------------|--------|-------------|-------|-------|-------------|-------|--------|---------|-------|--------|---------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 2 | ţ, | | 2 | ef. | | | \$ | | | \$ | |
| Traffic Volume (vph) | 6 | 487 | 52 | 51 | 534 | 4 | 126 | 10 | 130 | 4 | 5 | 19 |
| Future Volume (vph) | 6 | 487 | 52 | 51 | 534 | 4 | 126 | 10 | 130 | 4 | 5 | 19 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.985 | | | 0.999 | | | 0.934 | | | 0.909 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.977 | | | 0.994 | |
| Satd. Flow (prot) | 1778 | 1844 | 0 | 1770 | 1861 | 0 | 0 | 1674 | 0 | 0 | 1675 | 0 |
| | 0.439 | | | 0.950 | | | | 0.833 | | | 0.952 | |
| Satd. Flow (perm) | 822 | 1844 | 0 | 1770 | 1861 | 0 | 0 | 1428 | 0 | 0 | 1604 | 0 |
| Right Turn on Red | | | No | | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 7 | 541 | 58 | 57 | 593 | 4 | 140 | 11 | 144 | 4 | 6 | 21 |
| Shared Lane Traffic (%) | _ | | | | | | | | | | • | |
| Lane Group Flow (vph) | 7 | 599 | 0 | 57 | 597 | 0 | 0 | 295 | 0 | 0 | 31 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | | 12 | | | 12 | | | 0 | | | 0 | |
| Link Offset(ft) | | 0 16 | | | 0 16 | | | 0 16 | | | 0 16 | |
| Crosswalk Width(ft) | | | | | - | | | 10 | | | 10 | |
| Two way Left Turn Lane | 0.99 | Yes 0.99 | 0.99 | 1.00 | Yes 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) Turn Type | Perm | NA | 9 | Prot | NA | 9 | Perm | NA | 9 | Perm | NA | 9 |
| Protected Phases | Feilii | 2 | | 1 | 6 | | Feilii | 8 | | Feilii | 4 | |
| Permitted Phases | 2 | 2 | | 1 | 0 | | 8 | 0 | | 4 | 4 | |
| Detector Phase | 2 | 2 | | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | 2 | 2 | | | 0 | | 0 | 0 | | т | | |
| Minimum Initial (s) | 10.0 | 10.0 | | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 17.0 | 17.0 | | 14.0 | 17.0 | | 14.0 | 14.0 | | 14.0 | 14.0 | |
| Total Split (s) | 45.0 | 45.0 | | 14.0 | 59.0 | | 31.0 | 31.0 | | 31.0 | 31.0 | |
| | 50.0% | 50.0% | | 15.6% | 65.6% | | 34.4% | 34.4% | | 34.4% | 34.4% | |
| Maximum Green (s) | 38.0 | 38.0 | | 7.0 | 52.0 | | 24.0 | 24.0 | | 24.0 | 24.0 | |
| Yellow Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | | -2.0 | -2.0 | | | -2.0 | | | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | | None | Min | | None | None | | None | None | |
| Act Effct Green (s) | 29.8 | 29.8 | | 10.2 | 36.6 | | | 21.2 | | | 21.2 | |
| Actuated g/C Ratio | 0.43 | 0.43 | | 0.15 | 0.53 | | | 0.31 | | | 0.31 | |
| v/c Ratio | 0.02 | 0.75 | | 0.22 | 0.61 | | | 0.68 | | | 0.06 | |
| Control Delay | 14.5 | 25.5 | | 37.2 | 13.6 | | | 33.9 | | | 22.5 | |

2026 Build AM Peak Hour Timmons Group

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/01/2023

| | ٠ | - | 7 | 1 | + | * | 1 | t | 1 | 4 | ŧ | 4 |
|-------------------------------|-------------|------------|----------|-----------|------------|------------|-----|------|-----|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 14.5 | 25.5 | | 37.2 | 13.6 | | | 33.9 | | | 22.5 | |
| LOS | В | С | | D | В | | | С | | | С | |
| Approach Delay | | 25.4 | | | 15.7 | | | 33.9 | | | 22.5 | |
| Approach LOS | | С | | | В | | | С | | | С | |
| Queue Length 50th (ft) | 2 | 252 | | 26 | 164 | | | 129 | | | 11 | |
| Queue Length 95th (ft) | 10 | 403 | | 68 | 272 | | | #264 | | | 34 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 519 | 1164 | | 261 | 1452 | | | 609 | | | 684 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.01 | 0.51 | | 0.22 | 0.41 | | | 0.48 | | | 0.05 | |
| Intersection Summary | | | | | | | | | | | | |
| Area Type: 0 | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 69.7 | 1 | | | | | | | | | | | |
| Natural Cycle: 65 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | oordinate | b | | | | | | | | | | |
| Maximum v/c Ratio: 0.75 | | | | | | | | | | | | |
| Intersection Signal Delay: 2 | | | | In | tersection | n LOS: C | | | | | | |
| Intersection Capacity Utiliza | tion 71.3% | 0 | | IC | U Level | of Service | эC | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume e | | | ueue ma | y be long | jer. | | | | | | | |
| Queue shown is maximu | m after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 1: S V | Vakefield S | Stroot & N | IC-07 (C | annon Av | | | | | | | | |
| | | | 0) 16-01 | | | | | | | | | |

| √ Ø1 | <u>→</u> _{Ø2} | Ø4 |
|----------------|------------------------|--------------|
| 14 s | 45 s | 31 s |
| ← Ø6 | | √1 ø8 |
| 59 s | | 31 s |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/01/2023

| ` | ٨ | - | 7 | 4 | + | * | 1 | t | 1 | 4 | ţ | ~ |
|----------------------------|-------|-------|-------|-------|---------|-------|-------|---------------------------------------|--------|-------|-------|--------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ٦ | 1 | 1 | ٦ | ţ, | | 3 | Þ | | ٦ | Þ | |
| Traffic Volume (vph) | 46 | 463 | 75 | 232 | 354 | 19 | 103 | 201 | 172 | 25 | 198 | 33 |
| Future Volume (vph) | 46 | 463 | 75 | 232 | 354 | 19 | 103 | 201 | 172 | 25 | 198 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | 2% | | | -2% | | | -2% | | | 2% | |
| Storage Length (ft) | 200 | | 100 | 350 | | 0 | 125 | | 0 | 250 | | 0 |
| Storage Lanes | 1 | | 1 | 1 | | 0 | 1 | | 0 | 1 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | 0.850 | | 0.992 | | | 0.931 | | | 0.978 | |
| Flt Protected | 0.950 | | | 0.950 | | | 0.950 | | | 0.950 | | |
| Satd. Flow (prot) | 1752 | 1844 | 1567 | 1787 | 1866 | 0 | 1787 | 1752 | 0 | 1752 | 1804 | 0 |
| Flt Permitted | 0.520 | | | 0.950 | | - | 0.479 | | - | 0.243 | | - |
| Satd. Flow (perm) | 959 | 1844 | 1567 | 1787 | 1866 | 0 | 901 | 1752 | 0 | 448 | 1804 | 0 |
| Right Turn on Red | | | No | | | No | | | No | | | No |
| Satd. Flow (RTOR) | | | | | | | | | | | | |
| Link Speed (mph) | | 35 | | | 35 | | | 20 | | | 35 | |
| Link Distance (ft) | | 1453 | | | 677 | | | 1822 | | | 478 | |
| Travel Time (s) | | 28.3 | | | 13.2 | | | 62.1 | | | 9.3 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 51 | 514 | 83 | 258 | 393 | 21 | 114 | 223 | 191 | 28 | 220 | 37 |
| Shared Lane Traffic (%) | 0. | 011 | | 200 | 000 | | | 220 | 101 | 20 | 220 | 01 |
| Lane Group Flow (vph) | 51 | 514 | 83 | 258 | 414 | 0 | 114 | 414 | 0 | 28 | 257 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | Lon | 12 | rugru | Lon | 12 | rugin | Lon | 12 | rugitu | Lon | 12 | rugitt |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | Yes | | | | | | Yes | |
| Headway Factor | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 | | 9 | 15 | 0.00 | 9 | 15 | | 9 | 15 | | 9 |
| Turn Type | Perm | NA | Perm | Prot | NA | Ŭ | Perm | NA | Ŭ | Perm | NA | Ū |
| Protected Phases | | 2 | | 1 | 6 | | | 8 | | | 4 | |
| Permitted Phases | 2 | _ | 2 | · | | | 8 | , , , , , , , , , , , , , , , , , , , | | 4 | · | |
| Detector Phase | 2 | 2 | 2 | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | _ | _ | _ | | • | | Ū | Ū | | | | |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 24.0 | 24.0 | 24.0 | 14.0 | 17.0 | | 24.0 | 24.0 | | 14.0 | 14.0 | |
| Total Split (s) | 38.0 | 38.0 | 38.0 | 21.0 | 59.0 | | 31.0 | 31.0 | | 31.0 | 31.0 | |
| Total Split (%) | 42.2% | 42.2% | 42.2% | 23.3% | 65.6% | | 34.4% | 34.4% | | 34.4% | 34.4% | |
| Maximum Green (s) | 31.0 | 31.0 | 31.0 | 14.0 | 52.0 | | 24.0 | 24.0 | | 24.0 | 24.0 | |
| Yellow Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | | -2.0 | -2.0 | | -2.0 | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| Lead/Lag | Lag | Lag | Lag | Lead | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 2.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | Min | None | Min | | None | None | | None | None | |
| Walk Time (s) | 7.0 | 7.0 | 7.0 | NONE | IVIII I | | 7.0 | 7.0 | | | | |
| Flash Dont Walk (s) | 10.0 | 10.0 | 10.0 | | | | 10.0 | 10.0 | | | | |
| Pedestrian Calls (#/hr) | 0.0 | 0.0 | 0.0 | | | | 0.0 | 10.0 | | | | |
| Act Effct Green (s) | 28.0 | 28.0 | 28.0 | 15.3 | 48.4 | | 23.4 | 23.4 | | 23.4 | 23.4 | |
| | 20.0 | 20.0 | 20.0 | 10.0 | 40.4 | | 20.4 | 20.4 | | 20.4 | 20.4 | |

2026 Build AM Peak Hour Timmons Group

Zebulon South TIA

| 2: NC-96 (Arendell Avenue) |) & NC-97 (| (Gannon Avenue) |
|----------------------------|-------------|-----------------|
|----------------------------|-------------|-----------------|

12/01/2023

| | ٠ | - | 7 | 4 | - | * | 1 | t | 1 | 4 | ţ | ~ |
|-------------------------------|-------------|------------|----------|------------|----------|------------|------|------|-----|------|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Actuated g/C Ratio | 0.34 | 0.34 | 0.34 | 0.19 | 0.59 | | 0.29 | 0.29 | | 0.29 | 0.29 | |
| v/c Ratio | 0.16 | 0.82 | 0.16 | 0.77 | 0.38 | | 0.44 | 0.83 | | 0.22 | 0.50 | |
| Control Delay | 20.8 | 37.0 | 20.2 | 51.0 | 10.2 | | 32.1 | 44.3 | | 29.4 | 29.4 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 20.8 | 37.0 | 20.2 | 51.0 | 10.2 | | 32.1 | 44.3 | | 29.4 | 29.4 | |
| LOS | С | D | С | D | В | | С | D | | С | С | |
| Approach Delay | | 33.6 | | | 25.9 | | | 41.6 | | | 29.4 | |
| Approach LOS | | С | | | С | | | D | | | С | |
| Queue Length 50th (ft) | 19 | 253 | 31 | 138 | 111 | | 51 | 211 | | 12 | 116 | |
| Queue Length 95th (ft) | 45 | 378 | 63 | #268 | 168 | | 106 | #370 | | 36 | 195 | |
| Internal Link Dist (ft) | | 1373 | | | 597 | | | 1742 | | | 398 | |
| Turn Bay Length (ft) | 200 | | 100 | 350 | | | 125 | | | 250 | | |
| Base Capacity (vph) | 393 | 756 | 642 | 355 | 1252 | | 291 | 566 | | 144 | 583 | |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Reduced v/c Ratio | 0.13 | 0.68 | 0.13 | 0.73 | 0.33 | | 0.39 | 0.73 | | 0.19 | 0.44 | |
| Intersection Summary | | | | | | | | | | | | |
| | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 82 | | | | | | | | | | | | |
| Natural Cycle: 65 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | coordinate | b | | | | | | | | | | |
| Maximum v/c Ratio: 0.83 | | | | | | | | | | | | |
| Intersection Signal Delay: 3 | | | | | | n LOS: C | | | | | | |
| Intersection Capacity Utiliza | ation 80.8% | 0 | | IC | U Level | of Service | эD | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | exceeds c | apacity, c | lueue ma | iy be long | jer. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 2: NC | -96 (Areno | dell Aven | ue) & NC | -97 (Gan | non Aver | nue) | | | | | | |
| 1. | A | | | | | | | | | | | |
| Ø1 | | Ø2 | | | | | | Ø4 | | | | |
| 215 | 38 s | | | | | | 31 | s | | | | |
| Ø6 | | | | | | | 1 | Øs | | | | |
| 59 s | | | | | | | 21 | | | | | |

PD-2024-01 Zebulon South TIA 3: NC-96 (Arendell Avenue) & Barbee Street

| Intersection | | | | | | | | | | | | | |
|------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|--|
| Int Delay, s/veh | 2.3 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | | \$ | | | 4 | | | 4 | | | 4 | | |
| Traffic Vol, veh/h | 34 | 4 | 21 | 4 | 4 | 4 | 56 | 346 | 4 | 4 | 169 | 20 | |
| Future Vol, veh/h | 34 | 4 | 21 | 4 | 4 | 4 | 56 | 346 | 4 | 4 | 169 | 20 | |
| Conflicting Peds. #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |

| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
|------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|--|
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None | |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - | |
| Veh in Median Storage | e, # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | 90 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 38 | 4 | 23 | 4 | 4 | 4 | 62 | 384 | 4 | 4 | 188 | 22 | |

| Major/Minor | Minor2 | | 1 | Minor1 | | | Major1 | | Μ | lajor2 | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|-----|--------|---|---|--|
| Conflicting Flow All | 721 | 719 | 199 | 731 | 728 | 386 | 210 | 0 | 0 | 388 | 0 | 0 | |
| Stage 1 | 207 | 207 | - | 510 | 510 | - | - | - | - | - | - | - | |
| Stage 2 | 514 | 512 | - | 221 | 218 | - | - | - | - | - | - | - | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - 1 | 2.218 | - | - | |
| Pot Cap-1 Maneuver | 343 | 354 | 842 | 337 | 350 | 662 | 1361 | - | - | 1170 | - | - | |
| Stage 1 | 795 | 731 | - | 546 | 538 | - | - | - | - | - | - | - | |
| Stage 2 | 543 | 536 | - | 781 | 723 | - | - | - | - | - | - | - | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | |
| Mov Cap-1 Maneuver | · 321 | 332 | 842 | 309 | 328 | 662 | 1361 | - | - | 1170 | - | - | |
| Mov Cap-2 Maneuver | · 321 | 332 | - | 309 | 328 | - | - | - | - | - | - | - | |
| Stage 1 | 749 | 728 | - | 514 | 507 | - | - | - | - | - | - | - | |
| Stage 2 | 504 | 505 | - | 752 | 720 | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | |

| Approach | EB | WB | NB | SB | |
|----------------------|------|------|-----|-----|--|
| HCM Control Delay, s | 15.4 | 14.7 | 1.1 | 0.2 | |
| HCM LOS | С | В | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1V | VBLn1 | SBL | SBT | SBR |
|-----------------------|-------|-----|-----|--------|-------|-------|-----|-----|
| Capacity (veh/h) | 1361 | - | - | 413 | 385 | 1170 | - | - |
| HCM Lane V/C Ratio | 0.046 | - | - | 0.159 | 0.035 | 0.004 | - | - |
| HCM Control Delay (s) | 7.8 | 0 | - | 15.4 | 14.7 | 8.1 | 0 | - |
| HCM Lane LOS | А | А | - | С | В | Α | А | - |
| HCM 95th %tile Q(veh) | 0.1 | - | - | 0.6 | 0.1 | 0 | - | - |

| Intersection | | | | | | |
|------------------------|---------|------|---------|------|---------|------|
| Int Delay, s/veh | 1.2 | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y | | | ÷. | Þ | |
| Traffic Vol, veh/h | 42 | 8 | 4 | 343 | 160 | 13 |
| Future Vol, veh/h | 42 | 8 | 4 | 343 | 160 | 13 |
| Conflicting Peds, #/hr | r 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storag | ge, # 0 | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 9 | 4 | 381 | 178 | 14 |
| | | | | | | |
| Major/Minor | Minor2 | Ν | /lajor1 | Ν | /lajor2 | |
| Conflicting Flow All | 574 | 185 | 192 | 0 | - | 0 |

| Conflicting Flow All | 574 | 185 | 192 | 0 | - | 0 |
|----------------------|-----------|-------|-------|---|----|---|
| Stage 1 | 185 | - | - | - | - | - |
| Stage 2 | 389 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| 1 2 | 3.518 | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 480 | 857 | 1381 | - | - | - |
| Stage 1 | 847 | - | - | - | - | - |
| Stage 2 | 685 | - | - | - | - | - |
| Platoon blocked, % | | | | - | - | - |
| Mov Cap-1 Maneuver | 478 | 857 | 1381 | - | - | - |
| Mov Cap-2 Maneuver | 478 | - | - | - | - | - |
| Stage 1 | 844 | - | - | - | - | - |
| Stage 2 | 685 | - | - | - | - | - |
| | | | | | | |
| Approach | EB | | NB | | SB | |
| HCM Control Delay, s | | | 0.1 | | 0 | |
| HCM LOS | 12.3 B | | 0.1 | | 0 | |
| | D | | | | | |
| | | | | | | |

| Minor Lane/Major Mvmt | NBL | NBT EBLn1 | SBT | SBR | |
|-----------------------|-------|-----------|-----|-----|--|
| Capacity (veh/h) | 1381 | - 514 | - | - | |
| HCM Lane V/C Ratio | 0.003 | - 0.108 | - | - | |
| HCM Control Delay (s) | 7.6 | 0 12.9 | - | - | |
| HCM Lane LOS | А | A B | - | - | |
| HCM 95th %tile Q(veh) | 0 | - 0.4 | - | - | |

| Intersection | | | | | | |
|------------------------|--------|-------|--------|------|---------|------|
| Int Delay, s/veh | 1.4 | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y | | | ÷. | Þ | |
| Traffic Vol, veh/h | 42 | 17 | 6 | 304 | 154 | 14 |
| Future Vol, veh/h | 42 | 17 | 6 | 304 | 154 | 14 |
| Conflicting Peds, #/hr | | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storag | - | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 19 | 7 | 338 | 171 | 16 |
| | | 15 | 1 | 000 | 17.1 | 10 |
| | | | | | | |
| Major/Minor I | Minor2 | 1 | Major1 | Ν | /lajor2 | |
| Conflicting Flow All | 531 | 179 | 187 | 0 | - | 0 |
| Stage 1 | 179 | - | - | - | - | - |
| Stage 2 | 352 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 509 | 864 | 1387 | - | - | - |
| Stage 1 | 852 | - | - | - | - | - |

| Stage | 852 | - | - | - | - | - |
|------------------|-----------|-----|------|---|----|---|
| Stage 2 | 712 | - | - | - | - | - |
| Platoon blocked, | % | | | - | - | - |
| Mov Cap-1 Mane | euver 506 | 864 | 1387 | - | - | - |
| Mov Cap-2 Mane | euver 506 | - | - | - | - | - |
| Stage 1 | 847 | - | - | - | - | - |
| Stage 2 | 712 | - | - | - | - | - |
| | | | | | | |
| Approach | EB | | NB | | SB | |
| | | | 0.4 | | | |

| HCM Control Delay, s | s 12.1 | 0.1 | 0 | | |
|----------------------|--------|-----|---|--|--|
| HCM LOS | В | | | | |
| | | | | | |

| Minor Lane/Major Mvmt | NBL | NBT EBLn1 | SBT | SBR | |
|-----------------------|-------|-----------|-----|-----|--|
| Capacity (veh/h) | 1387 | - 575 | - | - | |
| HCM Lane V/C Ratio | 0.005 | - 0.114 | - | - | |
| HCM Control Delay (s) | 7.6 | 0 12.1 | - | - | |
| HCM Lane LOS | А | A B | - | - | |
| HCM 95th %tile Q(veh) | 0 | - 0.4 | - | - | |

Zebulon South TIA 6: NC-96 (Arendell Avenue) & Perry Curtis Road

| Intersection | | | | | | |
|------------------------|--------|------|------|------|------|------|
| Int Delay, s/veh | 2.4 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | Þ | | | र्स |
| Traffic Vol, veh/h | 14 | 73 | 237 | 4 | 35 | 136 |
| Future Vol, veh/h | 14 | 73 | 237 | 4 | 35 | 136 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 16 | 81 | 263 | 4 | 39 | 151 |
| | | | | | | |

| Major/Minor I | Minor1 | Ν | /lajor1 | Ν | Major2 | |
|----------------------|--------|-------|---------|-------|--------|-----|
| Conflicting Flow All | 494 | 265 | 0 | 0 | 267 | 0 |
| Stage 1 | 265 | - | - | - | - | - |
| Stage 2 | 229 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 535 | 774 | - | - | 1297 | - |
| Stage 1 | 779 | - | - | - | - | - |
| Stage 2 | 809 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | | 774 | - | - | 1297 | - |
| Mov Cap-2 Maneuver | | - | - | - | - | - |
| Stage 1 | 779 | - | - | - | - | - |
| Stage 2 | 782 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | 10.8 | | 0 | | 1.6 | |
| HCM LOS | В | | • | | | |
| | | | | | | |
| | 1 | NDT | | | | ODT |
| Minor Lane/Major Mvr | nt | NBT | NBRW | | SBL | SBT |
| Capacity (veh/h) | | - | - | 717 | 1297 | - |
| HCM Lane V/C Ratio | | - | - (| 0.135 | 0.03 | - |
| HCM Control Delay (s | 5) | - | - | 10.8 | 7.9 | 0 |
| HCM Lane LOS | | - | - | В | Α | А |

-

-

0.5

0.1

-

HCM 95th %tile Q(veh)

Peak Hour Factor

Heavy Vehicles, % Mvmt Flow 90

2

9

90

2

58

90

2

238

90

2

4

90

2

18

| Intersection | | | | | | | |
|------------------------|--------|------|------|------|------|------|--|
| Int Delay, s/veh | 1.9 | | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT | |
| Lane Configurations | Y | | et i | | | ŧ | |
| Traffic Vol, veh/h | 8 | 52 | 214 | 4 | 16 | 92 | |
| Future Vol, veh/h | 8 | 52 | 214 | 4 | 16 | 92 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Stop | Stop | Free | Free | Free | Free | |
| RT Channelized | - | None | - | None | - | None | |
| Storage Length | 0 | - | - | - | - | - | |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 | |
| Grade, % | 0 | - | 0 | - | - | 0 | |

90

2

102

| Major/Minor | Minor1 | Ν | /lajor1 | Ν | Major2 | |
|----------------------|--------|-------|---------|------|--------|-----|
| Conflicting Flow All | 378 | 240 | 0 | 0 | 242 | 0 |
| Stage 1 | 240 | - | - | - | - | - |
| Stage 2 | 138 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 624 | 799 | - | - | 1324 | - |
| Stage 1 | 800 | - | - | - | - | - |
| Stage 2 | 889 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuve | r 615 | 799 | - | - | 1324 | - |
| Mov Cap-2 Maneuve | r 615 | - | - | - | - | - |
| Stage 1 | 800 | - | - | - | - | - |
| Stage 2 | 877 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| Approach | | | | | | |
| HCM Control Delay, s | | | 0 | | 1.1 | |
| HCM LOS | В | | | | | |
| | | | | | | |
| Minor Lane/Major Mv | rmt | NBT | NBRW | BLn1 | SBL | SBT |
| Capacity (veh/h) | | - | - | 768 | 1324 | - |
| HCM Lane V/C Ratio | | - | - (| | 0.013 | - |
| HCM Control Doloy (| | | | 10.1 | 7.0 | ٥ |

| | | | 100 | 1024 | | |
|-----------------------|---|-----|------|-------|---|--|
| HCM Lane V/C Ratio | - | - 0 | .087 | 0.013 | - | |
| HCM Control Delay (s) | - | - | 10.1 | 7.8 | 0 | |
| HCM Lane LOS | - | - | В | А | А | |
| HCM 95th %tile Q(veh) | - | - | 0.3 | 0 | - | |
| | | | | | | |

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/05/2023

| | ٠ | → | 7 | 1 | + | * | 1 | 1 | 1 | 4 | Ļ | ~ |
|----------------------------|-------|----------|---------------------------------------|-------|--------------|-------|-------|--------------|-------|-------|-------|------------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ٦ | eî 🔒 | | 7 | ef 👘 | | | \$ | | | \$ | |
| Traffic Volume (vph) | 10 | 688 | 150 | 73 | 569 | 4 | 113 | 10 | 79 | 7 | 19 | 29 |
| Future Volume (vph) | 10 | 688 | 150 | 73 | 569 | 4 | 113 | 10 | 79 | 7 | 19 | 29 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | | -1% | | | 0% | | | 3% | | | 1% | |
| Storage Length (ft) | 125 | | 0 | 125 | | 0 | 0 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (ft) | 25 | | , , , , , , , , , , , , , , , , , , , | 25 | | Ŭ | 25 | | Ŭ | 25 | | Ū |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.973 | | | 0.999 | | | 0.947 | | | 0.929 | |
| Flt Protected | 0.950 | 0.010 | | 0.950 | 0.000 | | | 0.973 | | | 0.993 | |
| Satd. Flow (prot) | 1778 | 1822 | 0 | 1770 | 1861 | 0 | 0 | 1691 | 0 | 0 | 1710 | 0 |
| Flt Permitted | 0.423 | 1022 | U | 0.950 | 1001 | 0 | U | 0.804 | 0 | 0 | 0.954 | U |
| Satd. Flow (perm) | 792 | 1822 | 0 | 1770 | 1861 | 0 | 0 | 1397 | 0 | 0 | 1643 | 0 |
| Right Turn on Red | 152 | 1022 | No | 1110 | 1001 | No | U | 1001 | No | 0 | 1040 | No |
| Satd. Flow (RTOR) | | | NU | | | NU | | | NU | | | NO |
| Link Speed (mph) | | 35 | | | 35 | | | 25 | | | 25 | |
| Link Distance (ft) | | 774 | | | 1453 | | | 1831 | | | 462 | |
| Travel Time (s) | | 15.1 | | | 28.3 | | | 49.9 | | | 12.6 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 20.3 0.90 | 0.90 | 0.90 | 49.9 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| | 0.90 | 764 | 167 | 0.90 | 632 | | 126 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 32 |
| Adj. Flow (vph) | 11 | 704 | 107 | 01 | 032 | 4 | 120 | 11 | 00 | 0 | 21 | 32 |
| Shared Lane Traffic (%) | 11 | 024 | 0 | 04 | 636 | 0 | 0 | 005 | 0 | 0 | 61 | 0 |
| Lane Group Flow (vph) | 11 | 931 | 0 | 81 | | 0 | 0 | 225 | 0 | 0 | 61 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | | 12 | | | 12 | | | 0 | | | 0 | |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | 0.00 | Yes | | 4.00 | Yes | 4 00 | 4.00 | 4.00 | 4.00 | | 4.04 | 4.04 |
| Headway Factor | 0.99 | 0.99 | 0.99 | 1.00 | 1.00 | 1.00 | 1.02 | 1.02 | 1.02 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | 15 | | 9 | 15 | | 9 | 15 | | 9 | 15 | | 9 |
| Turn Type | Perm | NA | | Prot | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 8 | | | 4 | |
| Permitted Phases | 2 | | | | | | 8 | | | 4 | | |
| Detector Phase | 2 | 2 | | 1 | 6 | | 8 | 8 | | 4 | 4 | |
| Switch Phase | | | | | | | | | | | | |
| Minimum Initial (s) | 10.0 | 10.0 | | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Split (s) | 17.0 | 17.0 | | 14.0 | 17.0 | | 14.0 | 14.0 | | 14.0 | 14.0 | |
| Total Split (s) | 52.0 | 52.0 | | 14.0 | 66.0 | | 24.0 | 24.0 | | 24.0 | 24.0 | |
| Total Split (%) | 57.8% | 57.8% | | 15.6% | 73.3% | | 26.7% | 26.7% | | 26.7% | 26.7% | |
| Maximum Green (s) | 45.0 | 45.0 | | 7.0 | 59.0 | | 17.0 | 17.0 | | 17.0 | 17.0 | |
| Yellow Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| All-Red Time (s) | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Lost Time Adjust (s) | -2.0 | -2.0 | | -2.0 | -2.0 | | | -2.0 | | | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | | 5.0 | 5.0 | | | 5.0 | | | 5.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | | None | Min | | None | None | | None | None | |
| Act Effct Green (s) | 46.2 | 46.2 | | 9.2 | 56.7 | | | 17.5 | | | 17.5 | |
| Actuated g/C Ratio | 0.55 | 0.55 | | 0.11 | 0.67 | | | 0.21 | | | 0.21 | |
| v/c Ratio | 0.03 | 0.93 | | 0.42 | 0.51 | | | 0.21 | | | 0.21 | |
| Control Delay | 10.8 | 37.8 | | 45.6 | 8.7 | | | 53.0 | | | 30.8 | |
| | 10.0 | 51.0 | | -J.U | 0.7 | | | 55.0 | | | 50.0 | |

2026 Build PM Peak Hour Timmons Group

Zebulon South TIA

1: S Wakefield Street & NC-97 (Gannon Avenue)

12/05/2023

| | ٠ | → | 7 | 4 | + | * | 1 | t | 1 | 4 | Ļ | ~ |
|-------------------------------|-------------|------------|----------|------------|---------|------------|-----|------|-----|-----|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBF |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | 0.0 | |
| Total Delay | 10.8 | 37.8 | | 45.6 | 8.7 | | | 53.0 | | | 30.8 | |
| LOS | В | D | | D | А | | | D | | | С | |
| Approach Delay | | 37.5 | | | 12.9 | | | 53.0 | | | 30.8 | |
| Approach LOS | | D | | | В | | | D | | | С | |
| Queue Length 50th (ft) | 3 | 489 | | 44 | 154 | | | 121 | | | 29 | |
| Queue Length 95th (ft) | 11 | #774 | | 90 | 229 | | | #234 | | | 63 | |
| Internal Link Dist (ft) | | 694 | | | 1373 | | | 1751 | | | 382 | |
| Turn Bay Length (ft) | 125 | | | 125 | | | | | | | | |
| Base Capacity (vph) | 449 | 1034 | | 192 | 1371 | | | 320 | | | 376 | |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | 0 | |
| Reduced v/c Ratio | 0.02 | 0.90 | | 0.42 | 0.46 | | | 0.70 | | | 0.16 | |
| Intersection Summary | | | | | | | | | | | | |
| | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 84. | 4 | | | | | | | | | | | |
| Natural Cycle: 90 | | | | | | | | | | | | |
| Control Type: Actuated-Uno | coordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.93 | | | | | | | | | | | | |
| Intersection Signal Delay: 3 | | | | | | n LOS: C | | | | | | |
| Intersection Capacity Utiliza | ation 81.9% | 0 | | IC | U Level | of Service | эD | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | | | ueue ma | iy be long | jer. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 1: S V | Nakefield S | Street & N | IC-97 (G | annon Av | /enue) | | | | | | | |
| √ Ø1 ▲ | 22 | | | | , | | | | Ø4 | | | |

| Ø1 | | Ø4 |
|------|------|--------------|
| 14 s | 52 s | 24 s |
| ←Ø6 | | ∞1 Ø8 |
| 66 s | | 24 s |

Zebulon South TIA

2: NC-96 (Arendell Avenue) & NC-97 (Gannon Avenue)

12/05/2023

| | ٢ | + | 1 | 1 | Ļ | * | • | t | 1 | 1 | ţ | ~ |
|---|-------------|------------|------------|------------|------------|------------|------------|--------------|--------------|------------|------------|-------|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 3 | 1 | 1 | 5 | 1 | WBR | 5 | 1 | NB IX | 5 | 1 | |
| Traffic Volume (vph) | 66 | 363 | 91 | 282 | 512 | 57 | 90 | 322 | 176 | 51 | 227 | 50 |
| Future Volume (vph) | 66 | 363 | 91 | 282 | 512 | 57 | 90 | 322 | 176 | 51 | 227 | 50 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Grade (%) | 1000 | 2% | 1000 | 1000 | -2% | 1000 | 1000 | -2% | 1000 | 1000 | 2% | 1000 |
| Storage Length (ft) | 200 | 2 /0 | 100 | 350 | 270 | 0 | 125 | 270 | 0 | 250 | 270 | 0 |
| Storage Lanes | 200 | | 100 | 1 | | 0 | 123 | | 0 | 1 | | 0 |
| Taper Length (ft) | 25 | | | 25 | | U | 25 | | 0 | 25 | | Ū |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 0.850 | 1.00 | 0.985 | 1.00 | 1.00 | 0.947 | 1.00 | 1.00 | 0.973 | 1.00 |
| Flt Protected | 0.950 | | 0.000 | 0.950 | 0.000 | | 0.950 | 0.547 | | 0.950 | 0.070 | |
| Satd. Flow (prot) | 1752 | 1844 | 1567 | 1787 | 1853 | 0 | 1787 | 1782 | 0 | 1752 | 1794 | 0 |
| Flt Permitted | 0.425 | 1044 | 1007 | 0.950 | 1000 | 0 | 0.450 | 1702 | U | 0.157 | 1754 | U |
| Satd. Flow (perm) | 784 | 1844 | 1567 | 1787 | 1853 | 0 | 847 | 1782 | 0 | 290 | 1794 | 0 |
| Right Turn on Red | 704 | 1044 | No | 1707 | 1000 | No | 047 | 1702 | No | 230 | 1154 | No |
| Satd. Flow (RTOR) | | | NO | | | NU | | | NU | | | NO |
| Link Speed (mph) | | 35 | | | 35 | | | 20 | | | 35 | |
| Link Distance (ft) | | 1453 | | | 677 | | | 1822 | | | 478 | |
| Travel Time (s) | | 28.3 | | | 13.2 | | | 62.1 | | | 9.3 | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 02.1 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 73 | 403 | 101 | 313 | 569 | 63 | 100 | 358 | 196 | 0.90 | 252 | 0.90 |
| Shared Lane Traffic (%) | 13 | 403 | 101 | 313 | 509 | 05 | 100 | 300 | 190 | 57 | 202 | 50 |
| | 73 | 403 | 101 | 313 | 632 | 0 | 100 | 554 | 0 | 57 | 308 | 0 |
| Lane Group Flow (vph) Enter Blocked Intersection | No | 403 No | No | No | No | No | No | No | No | No | No | No |
| | Left | Left | | Left | Left | | Left | | | Left | Left | |
| Lane Alignment | Leit | 12 | Right | Leit | 12 | Right | Leit | Left 12 | Right | Leit | 12 | Right |
| Median Width(ft) Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | Yes | | | 10 | | | Yes | |
| | 1.01 | 1.01 | 1.01 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Headway Factor | 1.01 | 1.01 | 9 | 0.99 | 0.99 | 0.99 | 15 | 0.99 | 0.99 | 1.01 | 1.01 | 1.01 |
| Turning Speed (mph) | Perm | NA | Perm | Prot | NA | 9 | Perm | NA | 9 | Perm | NA | 9 |
| Turn Type Protected Phases | Feilii | 2 | Feilii | 1 | 6 | | Feilli | NA 8 | | Feilli | 4 | |
| Permitted Phases | 2 | 2 | 2 | I | 0 | | 0 | 0 | | 4 | 4 | |
| Detector Phase | 2 | 2 | 2 | 1 | 6 | | 8 8 | 8 | | 4 | 4 | |
| Switch Phase | 2 | 2 | 2 | I | 0 | | 0 | 0 | | 4 | 4 | |
| | 10.0 | 10.0 | 10.0 | 7.0 | 10.0 | | 7.0 | 7.0 | | 7.0 | 7.0 | |
| Minimum Initial (s) Minimum Split (s) | 24.0 | 24.0 | 24.0 | 14.0 | 17.0 | | 24.0 | 24.0 | | 14.0 | 14.0 | |
| Total Split (s) | 24.0 | 24.0 | 24.0 | 24.0 | 53.0 | | 37.0 | 24.0 37.0 | | 37.0 | 37.0 | |
| | 32.2% | 32.2% | 32.2% | 24.0 | 58.9% | | 41.1% | 41.1% | | 41.1% | 41.1% | |
| Total Split (%) Maximum Green (s) | 22.0 | 22.0 | 22.0 | 17.0 | | | 30.0 | 30.0 | | 30.0 | 30.0 | |
| | 22.0 5.0 | | | | 46.0 | | | 50.0 5.0 | | | | |
| Yellow Time (s) All-Red Time (s) | 5.0 2.0 | 5.0 | 5.0 2.0 | 5.0 2.0 | 5.0 2.0 | | 5.0 2.0 | 5.0 2.0 | | 5.0 2.0 | 5.0 2.0 | |
| | | 2.0 | | | | | | | | | | |
| Lost Time Adjust (s) | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | | -2.0 | -2.0 | | -2.0 | -2.0 | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 | | 5.0 | 5.0 | |
| Lead/Lag | Lag | Lag | Lag | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | 2.0 | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Vehicle Extension (s) | 3.0 Min | 3.0 Min | 3.0 Min | 2.0 | 3.0 Min | | 2.0 | 2.0 | | 2.0 | 2.0 | |
| Recall Mode | Min | Min | Min | None | Min | | None | None | | None | None | |
| Walk Time (s) | 7.0 | 7.0 | 7.0 | | | | 7.0 | 7.0 | | | | |
| Flash Dont Walk (s) | 10.0 | 10.0 | 10.0 | | | | 10.0 | 10.0 | | | | |
| Pedestrian Calls (#/hr) | 0 | 0 | 0 | 40.4 | | | 0 | 0 | | 20.0 | 20.0 | |
| Act Effct Green (s) | 22.4 | 22.4 | 22.4 | 18.1 | 45.6 | | 30.0 | 30.0 | | 30.0 | 30.0 | |

2026 Build PM Peak Hour Timmons Group

Zebulon South TIA

| 2: NC-96 (Arendell Avenue) |) & NC-97 (| (Gannon Avenue) |
|----------------------------|-------------|-----------------|
|----------------------------|-------------|-----------------|

12/05/2023

| | ٠ | - | 7 | 1 | + | * | 1 | t | 1 | 4 | ţ | 4 |
|-------------------------------|-------------|-----------|----------|------------|------------|------------|------|------|-----|------|------|-----|
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBI |
| Actuated g/C Ratio | 0.26 | 0.26 | 0.26 | 0.21 | 0.53 | | 0.35 | 0.35 | | 0.35 | 0.35 | |
| v/c Ratio | 0.36 | 0.84 | 0.25 | 0.83 | 0.64 | | 0.34 | 0.89 | | 0.56 | 0.49 | |
| Control Delay | 32.8 | 47.5 | 27.8 | 53.4 | 18.3 | | 25.2 | 45.2 | | 48.5 | 25.5 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 32.8 | 47.5 | 27.8 | 53.4 | 18.3 | | 25.2 | 45.2 | | 48.5 | 25.5 | |
| LOS | С | D | С | D | В | | С | D | | D | С | |
| Approach Delay | | 42.2 | | | 29.9 | | | 42.1 | | | 29.1 | |
| Approach LOS | | D | | | С | | | D | | | С | |
| Queue Length 50th (ft) | 34 | 215 | 45 | 171 | 240 | | 41 | 290 | | 26 | 134 | |
| Queue Length 95th (ft) | 75 | #365 | 87 | #309 | 356 | | 85 | #481 | | #84 | 212 | |
| Internal Link Dist (ft) | | 1373 | | | 597 | | | 1742 | | | 398 | |
| Turn Bay Length (ft) | 200 | | 100 | 350 | | | 125 | | | 250 | | |
| Base Capacity (vph) | 222 | 522 | 443 | 401 | 1050 | | 320 | 673 | | 109 | 677 | |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Reduced v/c Ratio | 0.33 | 0.77 | 0.23 | 0.78 | 0.60 | | 0.31 | 0.82 | | 0.52 | 0.45 | |
| Intersection Summary | | | | | | | | | | | | |
| 71 | Other | | | | | | | | | | | |
| Cycle Length: 90 | | | | | | | | | | | | |
| Actuated Cycle Length: 85. | 7 | | | | | | | | | | | |
| Natural Cycle: 75 | | | | | | | | | | | | |
| Control Type: Actuated-Unc | coordinate | d | | | | | | | | | | |
| Maximum v/c Ratio: 0.89 | | | | | | | | | | | | |
| Intersection Signal Delay: 3 | | | | | itersectio | | | | | | | |
| Intersection Capacity Utiliza | ation 88.9% | 6 | | IC | CU Level | of Service | эE | | | | | |
| Analysis Period (min) 15 | | | | | | | | | | | | |
| # 95th percentile volume | | | | iy be long | ger. | | | | | | | |
| Queue shown is maximu | um after tw | o cycles. | | | | | | | | | | |
| Splits and Phases: 2: NC | C-96 (Areno | dell Aven | ue) & NC | -97 (Gan | non Aver | nue) | | | | | | |
| √ Ø1 | | A | | | | | a | | | | | |
| 24s | | 5 Ø2 | | | | 37 | Ø4 | | | | | |
| | 2 | ~ ~ | | | | 57 | | | | | | |

| ♥ Ø1 | ₩ Ø2 | ▼ Ø4 | |
|----------|--|------|--|
| 24 s | 29 s | 37 s | |
| - | 12.2 million (* 19. m | | |
| Ø6 | | Ø8 | |
| 53 s | | 37 s | |

|--|

Int Delay, s/veh 4 EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR Movement Lane Configurations 4 4 4 4 Traffic Vol, veh/h 51 5 82 4 4 41 274 4 9 455 47 7 Future Vol, veh/h 51 5 82 4 4 41 274 4 9 7 455 47 Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 Stop Sign Control Stop Stop Stop Stop Stop Free Free Free Free Free Free **RT** Channelized - None None None None _ --_ _ --Storage Length ---. 0 Veh in Median Storage, # -0 0 0 ---_ -_ -Grade, % 0 0 0 0 --------Peak Hour Factor 90 90 90 90 90 90 90 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 Mvmt Flow 57 6 91 4 4 8 46 304 4 10 506 52

| Major/Minor | Minor2 | | | Vinor1 | | | Major1 | | N | lajor2 | | | |
|----------------------|--------|-------|-------|--------|-------|-------|--------|---|-----|--------|---|---|--|
| Conflicting Flow All | 956 | 952 | 532 | 999 | 976 | 306 | 558 | 0 | 0 | 308 | 0 | 0 | |
| Stage 1 | 552 | 552 | - | 398 | 398 | - | - | - | - | - | - | - | |
| Stage 2 | 404 | 400 | - | 601 | 578 | - | - | - | - | - | - | - | |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - | - | 4.12 | - | - | |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - | - 3 | 2.218 | - | - | |
| Pot Cap-1 Maneuver | 238 | 259 | 547 | 222 | 251 | 734 | 1013 | - | - | 1253 | - | - | |
| Stage 1 | 518 | 515 | - | 628 | 603 | - | - | - | - | - | - | - | |
| Stage 2 | 623 | 602 | - | 487 | 501 | - | - | - | - | - | - | - | |
| Platoon blocked, % | | | | | | | | - | - | | - | - | |
| Mov Cap-1 Maneuver | 220 | 242 | 547 | 172 | 234 | 734 | 1013 | - | - | 1253 | - | - | |
| Mov Cap-2 Maneuver | 220 | 242 | - | 172 | 234 | - | - | - | - | - | - | - | |
| Stage 1 | 490 | 509 | - | 593 | 570 | - | - | - | - | - | - | - | |
| Stage 2 | 578 | 569 | - | 397 | 495 | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | |

| Approach | EB | WB | NB | SB | |
|----------------------|------|------|-----|-----|--|
| HCM Control Delay, s | 23.7 | 17.7 | 1.1 | 0.1 | |
| HCM LOS | С | С | | | |

| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1 | VBLn1 | SBL | SBT | SBR | |
|-----------------------|-------|-----|-----|-------|-------|-------|-----|-----|--|
| Capacity (veh/h) | 1013 | - | - | 343 | 301 | 1253 | - | - | |
| HCM Lane V/C Ratio | 0.045 | - | - | 0.447 | 0.055 | 0.008 | - | - | |
| HCM Control Delay (s) | 8.7 | 0 | - | 23.7 | 17.7 | 7.9 | 0 | - | |
| HCM Lane LOS | А | А | - | С | С | А | А | - | |
| HCM 95th %tile Q(veh) | 0.1 | - | - | 2.2 | 0.2 | 0 | - | - | |

| Intersection | | | | | | | |
|------------------------|--------|------|------|------|------|------|---|
| Int Delay, s/veh | 0.8 | | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR | { |
| Lane Configurations | Y | | | ÷ | ħ | | |
| Traffic Vol, veh/h | 27 | 5 | 9 | 242 | 436 | 46 | 5 |
| Future Vol, veh/h | 27 | 5 | 9 | 242 | 436 | 46 | 6 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |) |
| Sign Control | Stop | Stop | Free | Free | Free | Free |) |
| RT Channelized | - | None | - | None | - | None |) |
| Storage Length | 0 | - | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | - | 0 | 0 | - | - |
| Grade, % | 0 | - | - | 0 | 0 | - | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |) |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 30 | 6 | 10 | 269 | 484 | 51 | l |
| | | | | | | | |

| Major/Minor | Minor2 | ľ | Major1 | Ν | 1ajor2 | | |
|----------------------|--------|-------|--------|------|--------|-----|--|
| Conflicting Flow All | 799 | 510 | 535 | 0 | - | 0 | |
| Stage 1 | 510 | - | - | - | - | - | |
| Stage 2 | 289 | - | - | - | - | - | |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | |
| Follow-up Hdwy | | 3.318 | 2.218 | - | - | - | |
| Pot Cap-1 Maneuver | 355 | 563 | 1033 | - | - | - | |
| Stage 1 | 603 | - | - | - | - | - | |
| Stage 2 | 760 | - | - | - | - | - | |
| Platoon blocked, % | | | | - | - | - | |
| Mov Cap-1 Maneuve | r 351 | 563 | 1033 | - | - | - | |
| Mov Cap-2 Maneuve | r 351 | - | - | - | - | - | |
| Stage 1 | 596 | - | - | - | - | - | |
| Stage 2 | 760 | - | - | - | - | - | |
| | | | | | | | |
| Approach | EB | | NB | | SB | | |
| HCM Control Delay, s | s 15.7 | | 0.3 | | 0 | | |
| HCM LOS | С | | | | | | |
| | | | | | | | |
| Minor Lane/Major Mv | mt | NBL | NBTE | BLn1 | SBT | SBR | |

| Minor Lane/Major WVmt | INBL | INBLEBLUI | SBI | SBR | |
|-----------------------|------|-----------|-----|-----|--|
| Capacity (veh/h) | 1033 | - 373 | - | - | |
| HCM Lane V/C Ratio | 0.01 | - 0.095 | - | - | |
| HCM Control Delay (s) | 8.5 | 0 15.7 | - | - | |
| HCM Lane LOS | А | A C | - | - | |
| HCM 95th %tile Q(veh) | 0 | - 0.3 | - | - | |

| Intersection | | | | | | | |
|------------------------|--------|------|------|------|------|------|---|
| Int Delay, s/veh | 1 | | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR | 2 |
| Lane Configurations | Y | | | ÷. | ħ | | |
| Traffic Vol, veh/h | 27 | 11 | 18 | 224 | 394 | 47 | 7 |
| Future Vol, veh/h | 27 | 11 | 18 | 224 | 394 | 47 | 1 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |) |
| Sign Control | Stop | Stop | Free | Free | Free | Free | ę |
| RT Channelized | - | None | - | None | - | None | , |
| Storage Length | 0 | - | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | - | 0 | 0 | - | - |
| Grade, % | 0 | - | - | 0 | 0 | - | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |) |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |) |
| Mvmt Flow | 30 | 12 | 20 | 249 | 438 | 52 | > |

| Major/Minor | Minor2 | | Major1 | Ν | 1ajor2 | | | |
|----------------------|--------|-------|--------|-------|--------|-----|--|--|
| Conflicting Flow All | 753 | 464 | 490 | 0 | - | 0 | | |
| Stage 1 | 464 | - | - | - | - | - | | |
| Stage 2 | 289 | - | - | - | - | - | | |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - | | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | | |
| Follow-up Hdwy | | 3.318 | | - | - | - | | |
| Pot Cap-1 Maneuver | | 598 | 1073 | - | - | - | | |
| Stage 1 | 633 | - | - | - | - | - | | |
| Stage 2 | 760 | - | - | - | - | - | | |
| Platoon blocked, % | | | | - | - | - | | |
| Mov Cap-1 Maneuve | | 598 | 1073 | - | - | - | | |
| Mov Cap-2 Maneuve | | - | - | - | - | - | | |
| Stage 1 | 619 | - | - | - | - | - | | |
| Stage 2 | 760 | - | - | - | - | - | | |
| | | | | | | | | |
| Approach | EB | | NB | | SB | | | |
| HCM Control Delay, | s 14.7 | | 0.6 | | 0 | | | |
| HCM LOS | В | | | | | | | |
| | | | | | | | | |
| Minor Lane/Maior My | umt | NRI | NRTE | ⊇ln 1 | SBT | SBB | | |

| Minor Lane/Major Mvmt | NBL | NBT EBLn1 | SBT | SBR | |
|-----------------------|-------|-----------|-----|-----|--|
| Capacity (veh/h) | 1073 | - 415 | - | - | |
| HCM Lane V/C Ratio | 0.019 | - 0.102 | - | - | |
| HCM Control Delay (s) | 8.4 | 0 14.7 | - | - | |
| HCM Lane LOS | А | A B | - | - | |
| HCM 95th %tile Q(veh) | 0.1 | - 0.3 | - | - | |

Zebulon South TIA 6: NC-96 (Arendell Avenue) & Perry Curtis Road

0.3

-

-

0.3

-

| Intersection | | | | | | |
|------------------------|--------|------|------|------|------|------|
| Int Delay, s/veh | 2.3 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | ţ, | | | र्स |
| Traffic Vol, veh/h | 6 | 56 | 186 | 7 | 113 | 292 |
| Future Vol, veh/h | 6 | 56 | 186 | 7 | 113 | 292 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 7 | 62 | 207 | 8 | 126 | 324 |

| Major/Minor I | Minor1 | Ν | /lajor1 | 1 | Major2 | |
|----------------------|--------|-------|---------|------|--------|-----|
| Conflicting Flow All | 787 | 211 | 0 | 0 | 215 | 0 |
| Stage 1 | 211 | - | - | - | - | - |
| Stage 2 | 576 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 360 | 829 | - | - | 1355 | - |
| Stage 1 | 824 | - | - | - | - | - |
| Stage 2 | 562 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | 319 | 829 | - | - | 1355 | - |
| Mov Cap-2 Maneuver | 319 | - | - | - | - | - |
| Stage 1 | 824 | - | - | - | - | - |
| Stage 2 | 498 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| Approach | | | | | | |
| HCM Control Delay, s | | | 0 | | 2.2 | |
| HCM LOS | В | | | | | |
| | | | | | | |
| Minor Lane/Major Mvr | nt | NBT | NBRW | BLn1 | SBL | SBT |
| Capacity (veh/h) | - | - | - | 718 | 1355 | _ |
| HCM Lane V/C Ratio | | - | - (| | 0.093 | - |
| HCM Control Delay (s | ;) | - | - | 10.5 | 7.9 | 0 |
| HCM Lane LOS | ') | - | - | B | A | Ă |
| | | | | | ,, | |

HCM 95th %tile Q(veh)

| lr | nte | rse | ctio | n | | | |
|----|-----|-----|------|---|--|--|--|
| _ | | | | | | | |

| Int Delay, s/veh | 1.8 | | | | | |
|------------------------|--------|------|------|------|------|------|
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | 1. | | | र्स |
| Traffic Vol, veh/h | 5 | 34 | 168 | 9 | 55 | 187 |
| Future Vol, veh/h | 5 | 34 | 168 | 9 | 55 | 187 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 6 | 38 | 187 | 10 | 61 | 208 |

| Major/Minor | Minor1 | Ν | /lajor1 | Ν | /lajor2 | |
|----------------------|--------|-------|---------|---------|---------|-----|
| Conflicting Flow All | 522 | 192 | 0 | 0 | 197 | 0 |
| Stage 1 | 192 | - | - | - | - | - |
| Stage 2 | 330 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 515 | 850 | - | - | 1376 | - |
| Stage 1 | 841 | - | - | - | - | - |
| Stage 2 | 728 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuve | er 489 | 850 | - | - | 1376 | - |
| Mov Cap-2 Maneuve | er 489 | - | - | - | - | - |
| Stage 1 | 841 | - | - | - | - | - |
| Stage 2 | 692 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, | | | 0 | | 1.8 | |
| HCM LOS | A 0.0 | | | | 1.0 | |
| | ,, | | | | | |
| Minor Long/Maior M | unat | NDT | | /DI - 4 | CDI | ODT |
| Minor Lane/Major M | /mt | NBT | NBRW | IRFU1 | SBL | SBT |
| Capacity (veh/h) | | - | - | 777 | 1376 | - |

| Capacity (ven/n) | - | - /// | 1376 | - | | |
|-----------------------|---|---------|-------|---|--|--|
| HCM Lane V/C Ratio | - | - 0.056 | 0.044 | - | | |
| HCM Control Delay (s) | - | - 9.9 | 7.7 | 0 | | |
| HCM Lane LOS | - | - A | А | А | | |
| HCM 95th %tile Q(veh) | - | - 0.2 | 0.1 | - | | |

2026 Build + Improvements Traffic Volumes

| 12/04/20 | 23 |
|----------|----|
|----------|----|

| Intersection | | | | | | |
|------------------------|--------|------|------|------|------|------|
| Int Delay, s/veh | 1.2 | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y | | | ÷. | • | 1 |
| Traffic Vol, veh/h | 42 | 8 | 4 | 343 | 160 | 13 |
| Future Vol, veh/h | 42 | 8 | 4 | 343 | 160 | 13 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | 50 |
| Veh in Median Storage | e, # 0 | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 9 | 4 | 381 | 178 | 14 |
| | | | | | | |

| Major/Minor | Minor2 | | Major1 | Maj | or2 | |
|----------------------|--------|-------|--------|-----|-----|---|
| Conflicting Flow All | 567 | 178 | 192 | 0 | - | 0 |
| Stage 1 | 178 | - | - | - | - | - |
| Stage 2 | 389 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | | 3.318 | | - | - | - |
| Pot Cap-1 Maneuver | | 865 | 1381 | - | - | - |
| Stage 1 | 853 | - | - | - | - | - |
| Stage 2 | 685 | - | - | - | - | - |
| Platoon blocked, % | | | | - | - | - |
| Mov Cap-1 Maneuve | | 865 | 1381 | - | - | - |
| Mov Cap-2 Maneuve | | - | - | - | - | - |
| Stage 1 | 850 | - | - | - | - | - |
| Stage 2 | 685 | - | - | - | - | - |
| | | | | | | |
| Approach | EB | | NB | | SB | |
| HCM Control Delay, | s 12.7 | | 0.1 | | 0 | |
| HCM LOS | В | | | | | |
| | | | | | | |
| | | | | | | |

| Minor Lane/Major Mvmt | NBL | NBT EBLn1 | SBT | SBR | |
|-----------------------|-------|-----------|-----|-----|--|
| Capacity (veh/h) | 1381 | - 520 | - | - | |
| HCM Lane V/C Ratio | 0.003 | - 0.107 | - | - | |
| HCM Control Delay (s) | 7.6 | 0 12.7 | - | - | |
| HCM Lane LOS | А | A B | - | - | |
| HCM 95th %tile Q(veh) | 0 | - 0.4 | - | - | |

| Intersection | | | | | | |
|------------------------|--------|--------|--------|-------|----------|------|
| Int Delay, s/veh | 1.4 | | | | | |
| - | | | | NDT | ODT | 000 |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y | 4- | ^ | र्भ | † | 1 |
| Traffic Vol, veh/h | 42 | 17 | 6 | 304 | 154 | 14 |
| Future Vol, veh/h | 42 | 17 | 6 | 304 | 154 | 14 |
| Conflicting Peds, #/hr | | 0 | _ 0 | _ 0 | _ 0 | _ 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | 110110 | | None | | None |
| Storage Length | 0 | - | - | - | - | 50 |
| Veh in Median Storag | | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 19 | 7 | 338 | 171 | 16 |
| | | | | | | |
| Major/Minor | Minor2 | I | Major1 | Ν | /lajor2 | |
| Conflicting Flow All | 523 | 171 | 187 | 0 | - | 0 |
| Stage 1 | 171 | - | - | - | - | - |
| Stage 2 | 352 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | _ |
| Follow-up Hdwy | | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | | 873 | 1387 | - | - | - |
| Stage 1 | 859 | - | - | - | - | - |
| Stage 2 | 712 | - | - | - | - | - |
| Platoon blocked, % | 112 | | | - | - | - |
| Mov Cap-1 Maneuve | r 511 | 873 | 1387 | _ | - | _ |
| Mov Cap-2 Maneuve | | | 1007 | _ | _ | _ |
| Stage 1 | 854 | | | - | - | |
| Stage 2 | 712 | - | - | - | - | - |
| Slage 2 | 112 | - | - | - | - | - |
| | | | | | | |
| Approach | EB | | NB | | SB | |
| HCM Control Delay, s | s 12 | | 0.1 | | 0 | |
| HCM LOS | В | | | | | |
| | | | | | | |
| Minor Lane/Major Mv | mt | NBL | NBT | EBLn1 | SBT | SBR |
| Capacity (veh/h) | t | 1387 | - | | | ODIX |
| HCM Lane V/C Ratio | | 0.005 | | 0.113 | - | - |
| HCM Control Delay (s | | 7.6 | 0 | 12 | - | - |
| HCM Lane LOS | 5) | | A | B | | |
| | | Α | A | D | - | - |

0.4

-

-

-

0

HCM 95th %tile Q(veh)

| Intersection | | |
|------------------|-----|--|
| Int Delay, s/veh | 1.9 | |

| Int Delay, s/veh | 1.9 | | | | | |
|------------------------|------|------|------|------|------|------|
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | f, | | ٦ | 1 |
| Traffic Vol, veh/h | 8 | 52 | 214 | 4 | 16 | 92 |
| Future Vol, veh/h | 8 | 52 | 214 | 4 | 16 | 92 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 50 | - |
| Veh in Median Storage | e,#0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 58 | 238 | 4 | 18 | 102 |

| Major/Minor | Minor1 | Ν | /lajor1 | 1 | Major2 | |
|----------------------|--------|-------|---------|------|--------|-----|
| Conflicting Flow All | 378 | 240 | 0 | 0 | 242 | 0 |
| Stage 1 | 240 | - | - | - | - | - |
| Stage 2 | 138 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuve | r 624 | 799 | - | - | 1324 | - |
| Stage 1 | 800 | - | - | - | - | - |
| Stage 2 | 889 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuve | er 615 | 799 | - | - | 1324 | - |
| Mov Cap-2 Maneuve | er 615 | - | - | - | - | - |
| Stage 1 | 800 | - | - | - | - | - |
| Stage 2 | 877 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, | s 10.1 | | 0 | | 1.1 | |
| HCM LOS | В | | | | | |
| | | | | | | |
| Minor Lane/Major M | vmt | NBT | NBRW | 3Ln1 | SBL | SBT |
| Capacity (veh/h) | | - | - | 768 | 1324 | - |
| HCM Lane V/C Ratio | C | - | - 0 | .087 | 0.013 | - |
| HCM Control Delay | (s) | - | - | 10.1 | 7.8 | - |

В

0.3

-

-

-

-

А

0

-

-

HCM Lane LOS

HCM 95th %tile Q(veh)

| Intersection | | | | | | |
|------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0.7 | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y | | | ŧ | 1 | 1 |
| Traffic Vol, veh/h | 27 | 5 | 9 | 242 | 436 | 46 |
| Future Vol, veh/h | 27 | 5 | 9 | 242 | 436 | 46 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | 50 |
| Veh in Median Storage | e,#0 | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 30 | 6 | 10 | 269 | 484 | 51 |
| | | | | | | |

| Major/Minor | Minor2 | | Major1 | Ν | lajor2 | | |
|----------------------|--------|-------|--------|------|--------|-----|--|
| Conflicting Flow All | 773 | 484 | 535 | 0 | - | 0 | |
| Stage 1 | 484 | - | - | - | - | - | |
| Stage 2 | 289 | - | - | - | - | - | |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - | |
| Pot Cap-1 Maneuver | 367 | 583 | 1033 | - | - | - | |
| Stage 1 | 620 | - | - | - | - | - | |
| Stage 2 | 760 | - | - | - | - | - | |
| Platoon blocked, % | | | | - | - | - | |
| Mov Cap-1 Maneuver | 363 | 583 | 1033 | - | - | - | |
| Mov Cap-2 Maneuver | | - | - | - | - | - | |
| Stage 1 | 613 | - | - | - | - | - | |
| Stage 2 | 760 | - | - | - | - | - | |
| | | | | | | | |
| Approach | EB | | NB | | SB | | |
| HCM Control Delay, s | 5 15.3 | | 0.3 | | 0 | | |
| HCM LOS | С | | | | | | |
| | | | | | | | |
| Minor Lane/Major Mv | mt | NBL | NBTE | BLn1 | SBT | SBR | |
| Capacity (veh/h) | | 1033 | - | 386 | - | _ | |

| Capacity (veh/h) | 1033 | - 386 | - | - |
|-----------------------|------|---------|---|---|
| HCM Lane V/C Ratio | 0.01 | - 0.092 | - | - |
| HCM Control Delay (s) | 8.5 | 0 15.3 | - | - |
| HCM Lane LOS | А | A C | - | - |
| HCM 95th %tile Q(veh) | 0 | - 0.3 | - | - |

| Intersection | | | | | | |
|---------------------------------------|--------|---------|------|---------|---------|------|
| Int Delay, s/veh | 1 | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y | | | 4 | • | 1 |
| Traffic Vol, veh/h | 27 | 11 | 18 | 224 | 394 | 47 |
| Future Vol, veh/h | 27 | 11 | 18 | 224 | 394 | 47 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | 50 |
| Veh in Median Storage | e, # 0 | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 30 | 12 | 20 | 249 | 438 | 52 |
| Peak Hour Factor Heavy Vehicles, % | 2 | 90 2 | 2 | 90 2 | 90 2 | 2 |

| Major/Minor | Minor2 | 1 | Major1 | Ν | lajor2 | | |
|----------------------|--------|-------|--------|-------|--------|-----|--|
| Conflicting Flow All | 727 | 438 | 490 | 0 | - | 0 | |
| Stage 1 | 438 | - | - | - | - | - | |
| Stage 2 | 289 | - | - | - | - | - | |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - | |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - | |
| Follow-up Hdwy | | 3.318 | | - | - | - | |
| Pot Cap-1 Maneuver | | 619 | 1073 | - | - | - | |
| Stage 1 | 651 | - | - | - | - | - | |
| Stage 2 | 760 | - | - | - | - | - | |
| Platoon blocked, % | | | | - | - | - | |
| Mov Cap-1 Maneuve | | 619 | 1073 | - | - | - | |
| Mov Cap-2 Maneuve | | - | - | - | - | - | |
| Stage 1 | 637 | - | - | - | - | - | |
| Stage 2 | 760 | - | - | - | - | - | |
| | | | | | | | |
| Approach | EB | | NB | | SB | | |
| HCM Control Delay, s | s 14.3 | | 0.6 | | 0 | | |
| HCM LOS | В | | | | | | |
| | | | | | | | |
| Minor Lane/Major Mv | mt | NBL | NBT | EBLn1 | SBT | SBR | |
| Capacity (veh/h) | | 1073 | - | 430 | - | - | |

| Capacity (veh/h) | 1073 | - 430 | - | - | |
|-----------------------|-------|---------|---|---|--|
| HCM Lane V/C Ratio | 0.019 | - 0.098 | - | - | |
| HCM Control Delay (s) | 8.4 | 0 14.3 | - | - | |
| HCM Lane LOS | А | A B | - | - | |
| HCM 95th %tile Q(veh) | 0.1 | - 0.3 | - | - | |

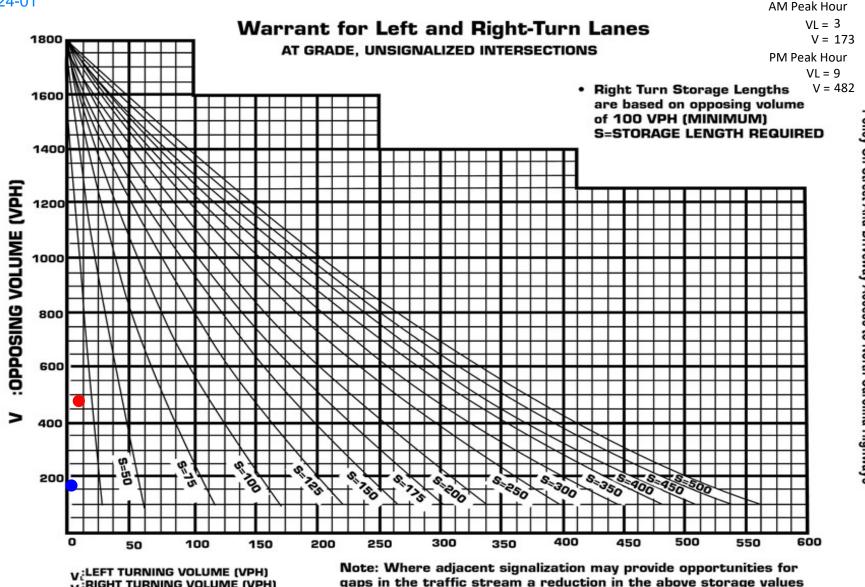
| In | ters | sect | ion | |
|----|------|------|-----|--|
| | | | | |

| Int Delay, s/veh | 1.8 | | | | | |
|------------------------|--------|------|------|------|------|------|
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Y | | Þ | | ٦ | 1 |
| Traffic Vol, veh/h | 5 | 34 | 168 | 9 | 55 | 187 |
| Future Vol, veh/h | 5 | 34 | 168 | 9 | 55 | 187 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 50 | - |
| Veh in Median Storage | e, # 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 6 | 38 | 187 | 10 | 61 | 208 |

| Major/Minor | Minor1 | ٨ | /lajor1 | Ν | /lajor2 | |
|----------------------|----------|-------|---------|-------|---------|-----|
| Conflicting Flow All | 522 | 192 | 0 | 0 | 197 | 0 |
| Stage 1 | 192 | - | - | - | - | - |
| Stage 2 | 330 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | | 850 | - | - | 1376 | - |
| Stage 1 | 841 | - | - | - | - | - |
| Stage 2 | 728 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuve | | 850 | - | - | 1376 | - |
| Mov Cap-2 Maneuve | | - | - | - | - | - |
| Stage 1 | 841 | - | - | - | - | - |
| Stage 2 | 696 | - | - | - | - | - |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, | s 9.9 | | 0 | | 1.8 | |
| HCM LOS | А | | | | | |
| | | | | | | |
| Minor Lane/Major M | vmt | NBT | NBRW | BLn1 | SBL | SBT |
| Capacity (veh/h) | | - | - | 777 | 1376 | - |
| HCM Lane V/C Ratio | ` | _ | - (| 0.056 | | _ |

| HCM Lane V/C Ratio | - | - 0.056 (| 0.044 | - | | | |
|-----------------------|---|-----------|-------|---|--|--|--|
| HCM Control Delay (s) | - | - 9.9 | 7.7 | - | | | |
| HCM Lane LOS | - | - A | А | - | | | |
| HCM 95th %tile Q(veh) | - | - 0.2 | 0.1 | - | | | |

Appendix E – NCDOT Nomographs



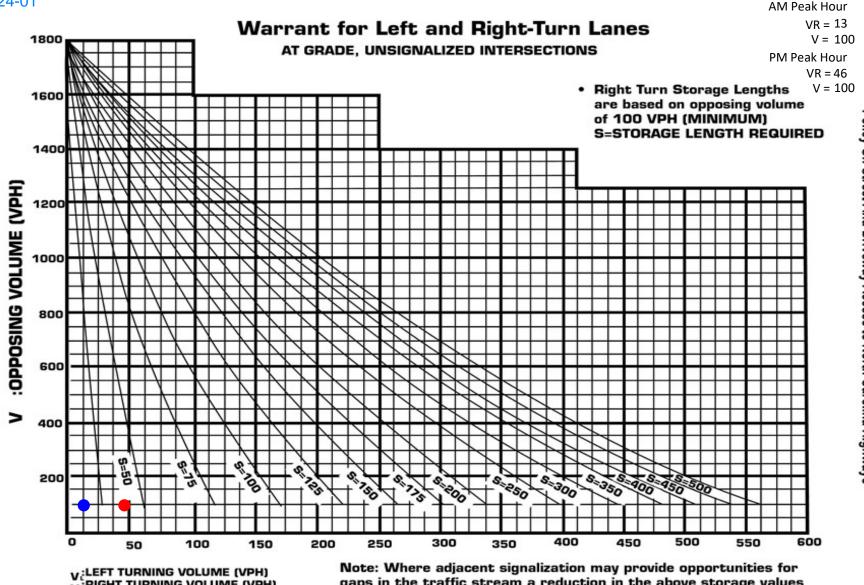
VERIGHT TURNING VOLUME (VPH)

can be considered on a case by case basis.

NC-96 (Arendell Ave) / Site Access 1

Northbound Left Turn

2026 Build AM and PM Peak Hours



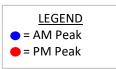
V RIGHT TURNING VOLUME (VPH)

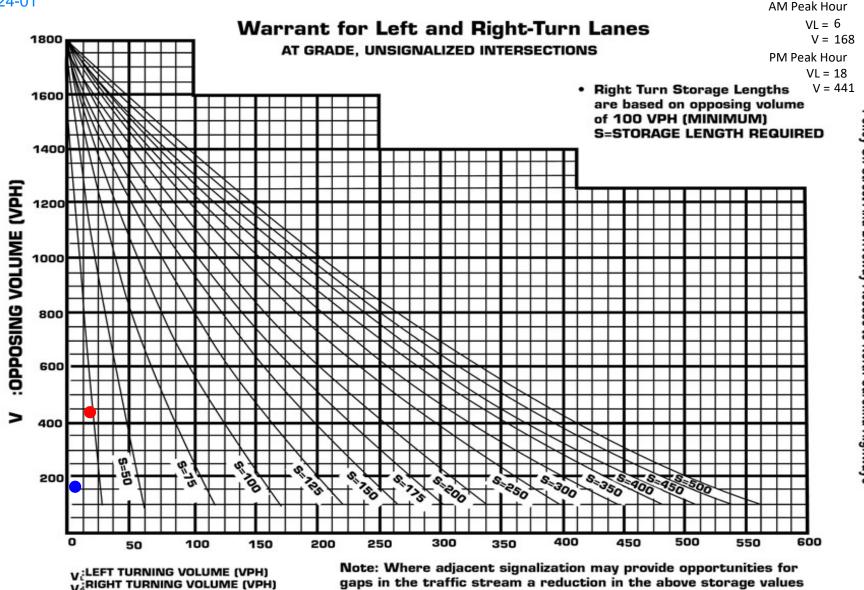
gaps in the traffic stream a reduction in the above storage values can be considered on a case by case basis.

NC-96 (Arendell Ave) / Site Access 1

Southbound Right Turn

2026 Build AM and PM Peak Hours



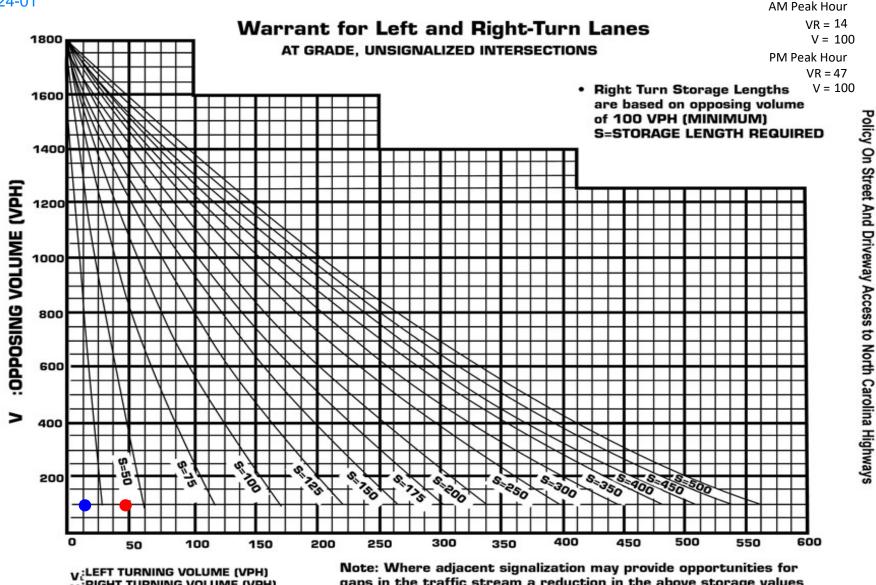


can be considered on a case by case basis.

NC-96 (Arendell Ave) / Site Access 2

Northbound Left Turn

2026 Build AM and PM Peak Hours



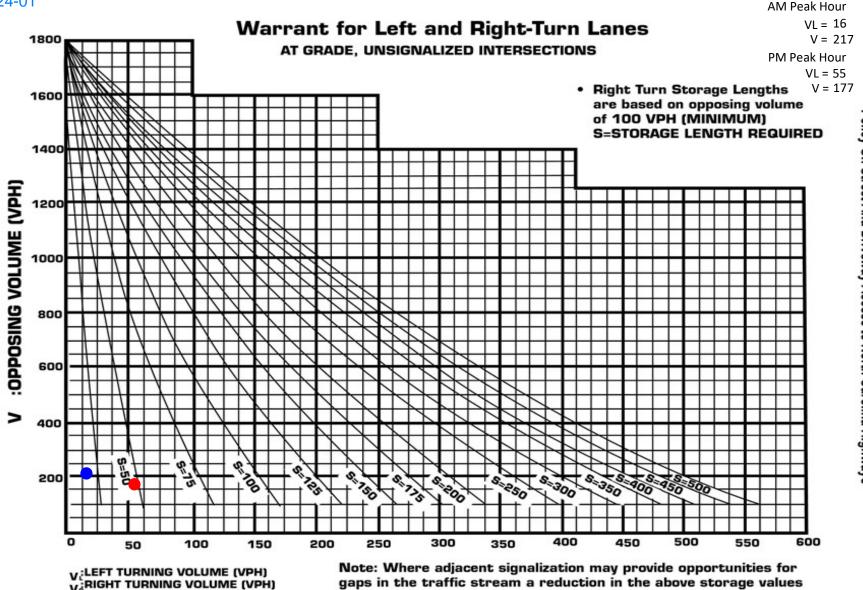
V RIGHT TURNING VOLUME (VPH)

gaps in the traffic stream a reduction in the above storage values can be considered on a case by case basis.

NC-96 (Arendell Ave) / Site Access 2

Southbound Right Turn

2026 Build AM and PM Peak Hours



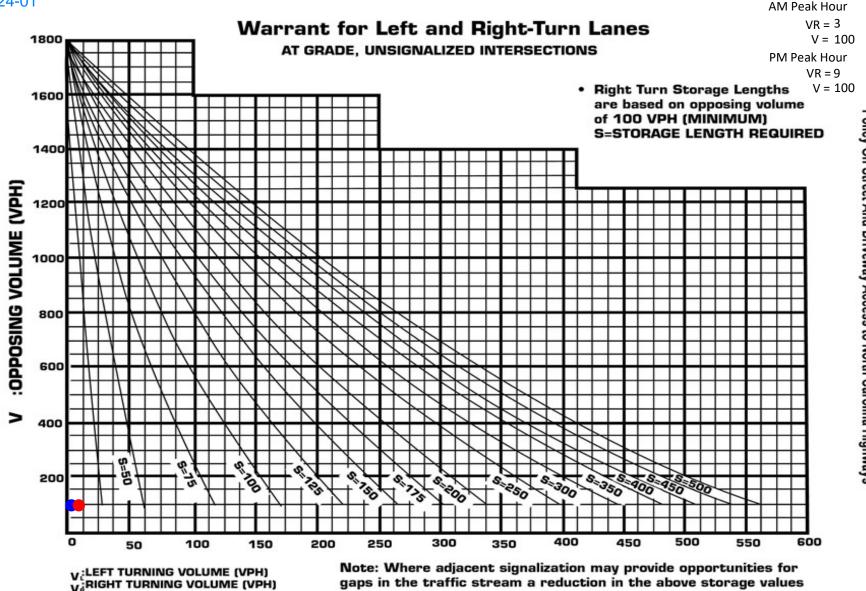


gaps in the traffic stream a reduction in the above storage values can be considered on a case by case basis.

S Wakefield Street / Site Access 3

Southbound Left Turn

2026 Build AM and PM Peak Hours





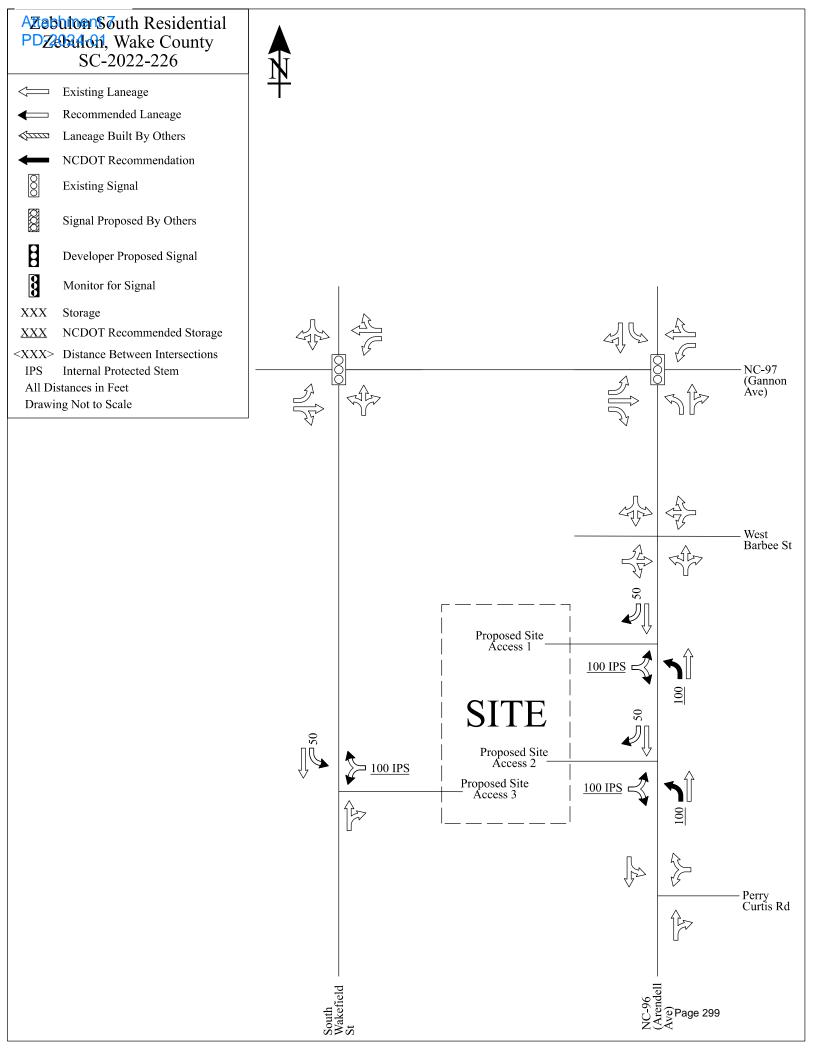
can be considered on a case by case basis.

S Wakefield Street / Site Access 3

Northbound Right Turn

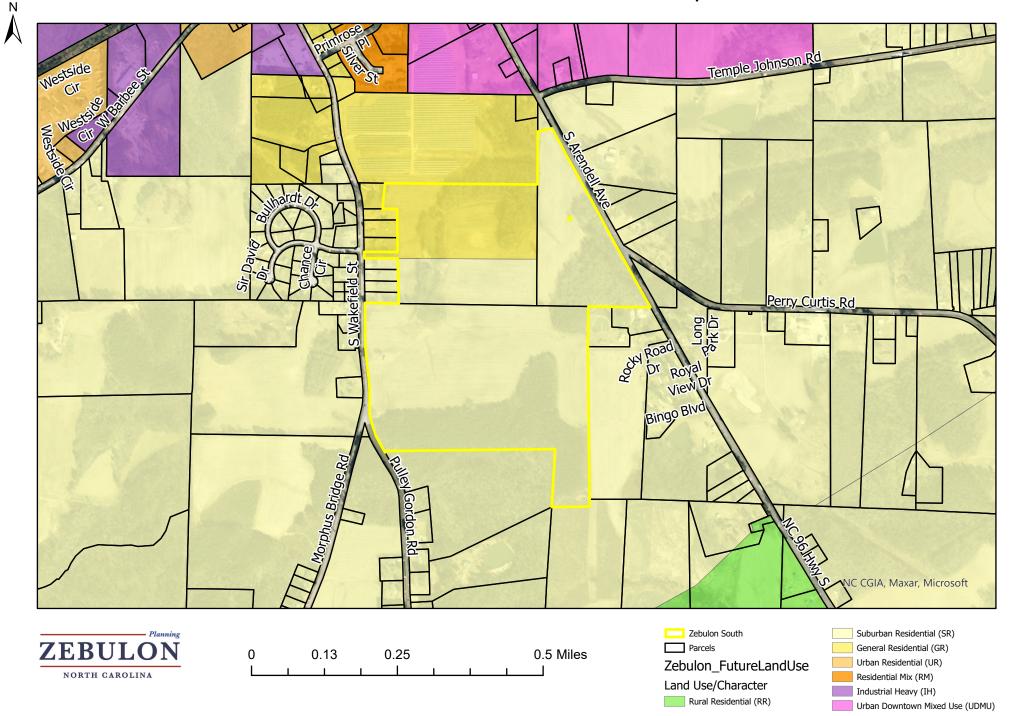
2026 Build AM and PM Peak Hours

Appendix F – NCDOT Requirements



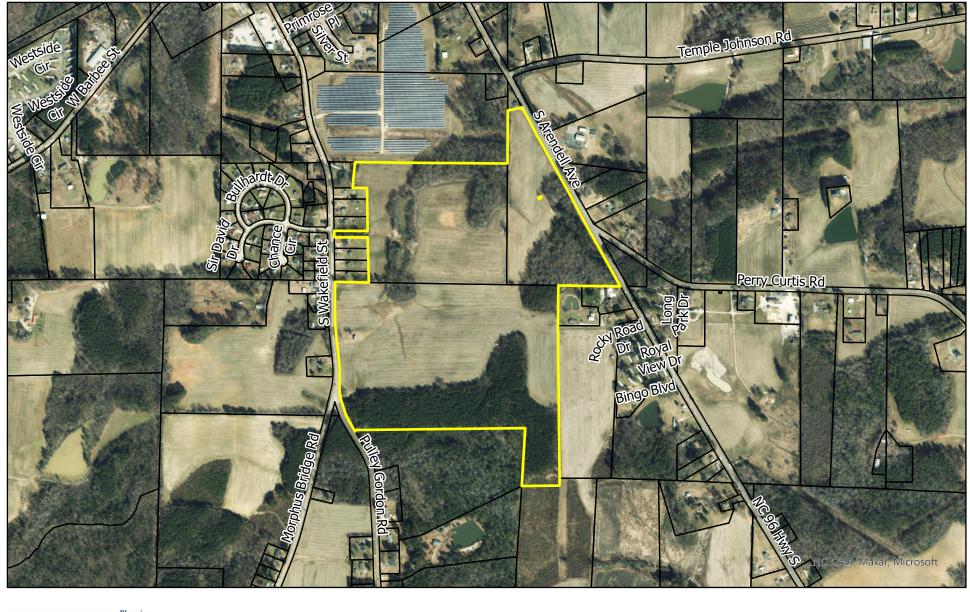
Attachment 8 PD-2024-01

Future Land Use and Character Map



N

Aerial Map





0

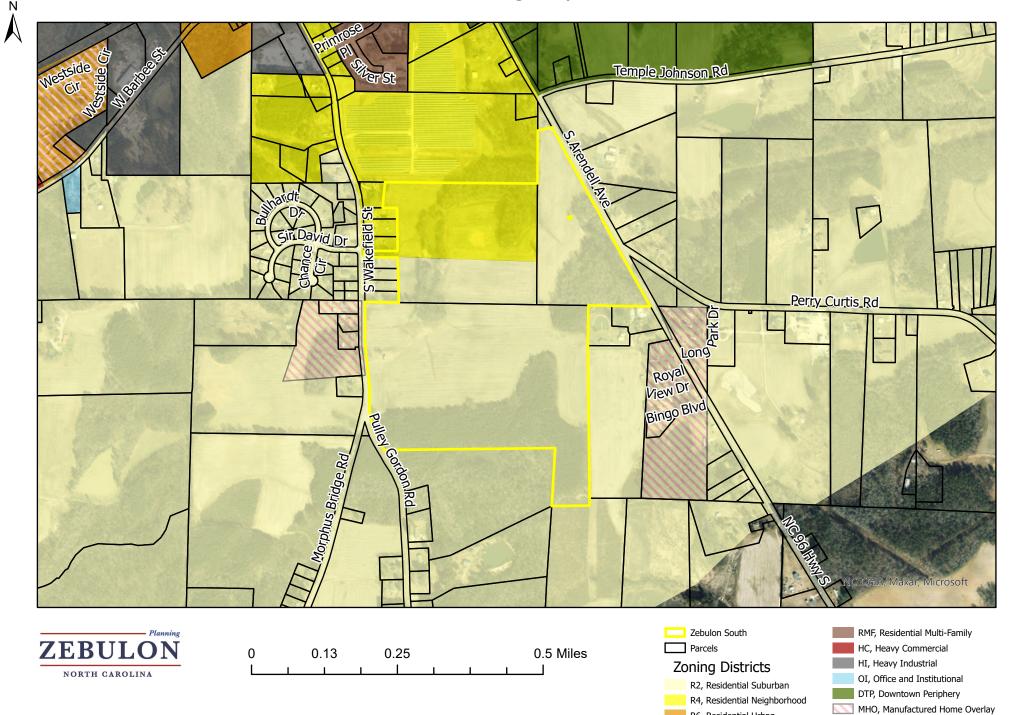
| 0. | 13 | 0.2 | 25 | | |
|----|----|-----|----|--|--|
| | | | | | |

0.5 Miles

Parcels
Zebulon South

Attachment 10 PD-2024-01

Zoning Map



Page 305

R6, Residential Urban



Site photo taken from side closest to Hwy 96



Site photo taken from the S. Wakefield side of the site.



Some existing structures from the prior agricultural use on site

ZEBULON

NORTH CAROLINA

CASE # PD 2024-01 IDT# 886895- Zebulon South

PROJECT ADDRESS 751 S Wakefield St

PIN NUMBER: 2704492511/ 2705512202/ 2705413075

HEARING DATE: February 12, 2024

State of North Carolina

County of Wake

BEFORE ME, the undersigned Notary, <u>Lisa M. Markland</u> on this <u>30th</u> day of <u>January</u> 20<u>34</u>, personally appeared Michael J. Clark, known to me to be a credible person and of lawful age, who being by me first duly sworn, on his oath, deposes and says:

I Michael J. Clark, acting as the Planning Director for the Town of Zebulon, affirm that the following Public Notice Procedures have been completed in accordance with applicable North Carolina General Statute and Town of Zebulon Unified Development Ordinance Section 2.3.6 have been satisfied for the above referenced hearing.

• First Class Mailing Sent on 1/29/2024 (see attached mailing list and copy of mailing)

304

- Advertisement in a Paper of General Circulation sent on 1/29/2024 (Wake weekly, publication dates 2/2 & 2/9/2024)
- Posting Public Hearing Signage on Property on 1/29/2024 (pictures attached)
- Posted to Planning Department Website 1/29/2024
- Sent to E-Mail Distribution List on 1/29/2024

Michael J. Clark, AICP, CNU-A Subscribed and supern to before me, this ____

Date

day of January

-isa M. Markland

[signature of Notary]

[printed name of Notary]

NOTARY PUBLIC

My commission expires: <u>3/29</u>, 20<u>25</u>.









Attachment 12 PD-2024-01



Notice of Public Hearing

Notice is hereby given pursuant to the provisions of Article 2.2.6 of the Town of Zebulon Unified Development Ordinance that a public hearing will be held on **February 12, 2024 at 6:00 PM** at the **Zebulon Municipal Complex, 1003 N. Arendell Avenue**, and will be conducted by the Board of Commissioners and Planning Board of the Town of Zebulon for the purpose of considering the following items:

IDT Project Number 886895 - PD 2024-01 – Zebulon South (751 S Wakefield St)

PIN # 2704492511, 2705512202, 2705413075. A request by Andrew Suriano of Deacon Development on behalf of property owners Harold Narron and Fred Corbett, Joseph Temple Sr and Alexander Harrison, Watson Family II LLC, for a rezoning to the Planned Development (PD) zoning district for the development of a 320 unit Planned Development.

IDT Project Number 1195805 - RZ 2024-01 - 321 Hospital Rd

PIN # 2705191832. A request by Germano Architecture and Interiors, PLLC on behalf of the property owners MiCy LLC., for a Zoning Map Amendment to the General Commercial (GC) zoning district.

Public comments may be submitted to Deputy Town Clerk Stacie Paratore at <u>SParatore@TownofZebulon.org</u> no later than 12:00 Noon on the day of the hearing to be read into the record. Links will be provided along with the full application packet and documentation on the Planning Department web page at <u>https://www.townofzebulon.org/departments/planning/public-hearing-information</u> For questions or additional information, please contact us at (919) 823-1816.

Wake Weekly February 2nd & 9th

3.5. General Mixed Use Zoning Districts

3.5.5. PLANNED DEVELOPMENT (PD) DISTRICT

A. PURPOSE AND INTENT

The Planned Development (PD) districts are established and intended to encourage innovative land planning and site design concepts that support a high quality of life and achieve a high quality of development, environmental sensitivity, energy efficiency, and other Town goals and objectives by:

- **a**. Reducing or diminishing the inflexibility or uniform design that sometimes results from strict application of zoning and development standards designed primarily for individual lots;
- **b.** Allowing greater freedom in selecting the means of providing access, open space, and design amenities;
- **C.** Allowing greater freedom in providing a well-integrated mix of residential and nonresidential land uses in the same development, including a mix of housing types, lot sizes, and densities;
- **d.** Creating a system of incentives for redevelopment and infill in order to revitalize established areas;
- **e.** Promoting a vibrant public realm by placing increased emphasis on active ground floor uses, pedestrian-oriented building façade design, intensive use of sidewalks, and establishment of public gathering areas;
- **f.** Providing for efficient use of land resulting in smaller networks of utilities and streets and thereby lowering development and housing costs; and
- **g.** Promoting quality design and environmentally sensitive development that respects surrounding established land use character and respects and takes advantage of a site's natural and man-made features, such as trees, estuaries, shorelines, special flood hazard area, and historic features.

B. GENERAL STANDARDS FOR ALL PLANNED DEVELOPMENTS

1. HOW ESTABLISHED

A planned development is established in a manner similar to the establishment of a conditional zoning district in accordance with the procedures and requirements in <u>Section 2.2.13</u>, Planned Development.

2. MASTER PLAN REQUIRED

All development configured as a PD shall be subject to a master plan submitted and approved as part of the application to establish the district. The master plan shall:

- **a.** Include a statement of planning objectives for the district;
- **b.** Describe the specific ways in which any modifications to the generally applicable standards in this Ordinance will result in a development of higher quality than would have otherwise resulted if the development was established without any proposed modifications to the standards in this Ordinance.
- **C.** Identify the general location of individual development areas, identified by land use(s) and/or development density or intensity;
- **d.** Depict the general configuration and relationship of the principal elements of the proposed development, including general building types;
- **e.** Identify for the entire district and each development area the acreage, types and mix of land uses, number of residential units (by use type), nonresidential floor area (by use type), residential density, and nonresidential intensity;
- **f.** Identify the general location, amount, and type (whether designated for active, passive, or urban) of open space;
- **g.** Identify the location of environmentally sensitive lands, wildlife habitat, and resource protection lands;
- **h.** Identify the on-site transportation circulation system, including the general location of all public and private streets, existing or projected transit service, pedestrian and vehicular circulation features, and how they will connect with existing and planned systems;
- **i.** Identify the general location of on-site potable water and wastewater facilities, and how they will connect to existing systems;
- **j.** Identify the general location of on-site stormwater management facilities, and how they will connect to existing public systems; and

3.5. General Mixed Use Zoning Districts

k. Identify the general location of all other on-site public facilities serving the development, including but not limited to parks, schools, bus shelters, and facilities for fire protection, police protection, EMS, and solid waste management.

3. COMPLIANCE WITH SUBDIVISION STANDARDS

Planned developments that include the division of land into two or more lots shall be subject to the subdivision standards in <u>Article 6: Subdivisions</u>, and shall be subject to the requirements of <u>Section</u> 2.2.14, Preliminary Plat, and <u>Section 2.2.10</u>, Final Plat, prior to the issuance of a building permit.

4. SITE PLAN REVIEW

- **a.** The planned development master plan may take the form of a generalized concept plan for development that provides a general indication of building and site feature location, or may it may be configured to the level of detail associated with site plans and construction drawings depicting exact building placement, location and profile of public infrastructure, and configuration of site features like parking, landscaping, and similar elements.
- **b.** In cases where the master plan is more general or conceptual in nature, the development proposed in the planned development designation shall also undergo site plan review in accordance with <u>Section 2.2.17</u>, Site Plan.
- **C.** In cases where the master plan is detailed and meets the minimum requirements for a site plan in the opinion of the Board of Commissioners, the applicant shall request, and the Board of Commissioners may grant an exemption from subsequent site plan review.
- **d.** If a site plan review exemption is granted by the Board of Commissioners, the proposed development shall fully comply with the development configuration depicted in the planned development master plan. Failure to comply with the approved master plan configuration shall require an amendment of the planned development application in accordance with <u>Section</u> 2.2.17.I, Amendment.

5. **DENSITIES/INTENSITIES**

The densities for residential development and the intensities for nonresidential development applicable in each development area of a PD district shall be as established in the master plan, and shall be consistent with adopted policy guidance.

6. DIMENSIONAL STANDARDS

The dimensional standards applicable in each development area of a PD district shall be as established in the master plan. The master plan shall include at least the following types of dimensional standards:

- **a.** Minimum lot area;
- **b.** Minimum lot width;
- **C.** Minimum and maximum setbacks;
- **d.** Maximum lot coverage;
- e. Maximum building height;
- f. Maximum individual building size;
- **g.** Floor area ratio; and
- **h.** Minimum setbacks from adjoining residential development or residential zoning districts.

7. DEVELOPMENT STANDARDS

- **a.** All development in a PD district shall comply with the development standards of <u>Article 5:</u> <u>Development Standards</u>, and the subdivision and infrastructure design standards of <u>Article 6:</u> <u>Subdivisions</u>, unless modified in accordance with this section.
- **b.** In no instance shall a planned development district seek to modify, waive, or reduce any of the following standards:
 - i. <u>Section 3.8, Overlay Zoning Districts; or</u>
 - **ii.** <u>Section 6.5, Owners' Associations</u>.
- **C.** In cases where a planned development district is proposed as part of redevelopment of an existing site and the existing site does not comply with the standards in subsection (b) above, the development contemplated in the planned development shall not be required to achieve full

3.5. General Mixed Use Zoning Districts

3.5.5 Planned Development (PD) District

compliance, but shall not increase the degree to which the development fails to comply with the standards in subsection (b) above.

8. CONSISTENCY WITH ADOPTED POLICY GUIDANCE

The PD zoning district designation, the master plan, and the terms and conditions document should be consistent with the Comprehensive Plan, and any applicable functional plans and small area plans adopted by the Town.

9. COMPATIBILITY WITH SURROUNDING AREAS

Development along the perimeter of a PD district shall be compatible with adjacent existing or proposed development. Where there are issues of compatibility, the master plan shall provide for transition areas at the edges of the PD district that provide for appropriate buffering and/or ensure a complementary character of uses. Determination of complementary character shall be based on densities/intensities, lot size and dimensions, building height, building mass and scale, hours of operation, exterior lighting, siting of service areas, or other aspects identified by the Board of Commissioners.

10. DEVELOPMENT PHASING PLAN

If development in the PD district is proposed to be phased, the master plan shall include a development phasing plan that identifies the general sequence or phases in which the district is proposed to be developed, including how residential and nonresidential development will be timed, how infrastructure (public and private) and open space will be provided and timed, and how development will be coordinated with the Town's capital improvements program.

11. CONVERSION SCHEDULE

- **a**. The planned development application may include a conversion schedule that identifies the extent to which one type of residential use may be converted to another type of residential use or one type of nonresidential use may be converted to another type of nonresidential use (i.e., residential to residential, or nonresidential to nonresidential). These conversions may occur within development areas and between development areas, as long as they occur within the same development phase, as identified by the approved development phasing plan, and are consistent with established extents of conversion set down in the conversion schedule.
- **b.** In the event an applicant seeks to revise the development in accordance with an approved conversion schedule, the applicant shall provide a revised site plan depicting the proposed conversions to the TRC for review and approval prior to commencing any conversions.

12. ON-SITE PUBLIC FACILITIES

a. DESIGN AND CONSTRUCTION

The master plan shall establish the responsibility of the developer/landowner to design and construct or install required and proposed on-site public facilities in compliance with applicable Town, state, and federal regulations.

b. DEDICATION

The master plan shall establish the responsibility of the developer/landowner to dedicate to the public the right-of-way and easements necessary for the construction or installation of required and proposed on-site public facilities in compliance with applicable Town, state, and federal regulations.

c. MODIFICATIONS TO STREET STANDARDS

In approving a master plan, the Board of Commissioners may approve modifications or reductions of street design standards—including those for right-of-way widths, pavement widths, required materials, provision of public transit amenities, and turning radii, with NCDOT approval, on finding that:

- **i.** The master plan provides for adequate separation/integration of vehicular, pedestrian, and bicycle traffic;
- **ii.** Access for emergency service vehicles is not substantially impaired;
- iii. Adequate parking is provided for the uses proposed; and

ARTICLE 3: DISTRICTS

3.5. General Mixed Use Zoning Districts

3.5.5 Planned Development (PD) District

- **iv.** Adequate space for public utilities is provided within the street right-of-way.

13. USES

The uses allowed in a PD district are identified in <u>Table 4.2.3</u>, <u>Principal Use Table</u>, as allowed subject to a master plan. Allowed uses shall be established in the master plan. Allowed uses shall be consistent with adopted policy guidance, the purpose of the particular PD district, and subject to any additional limitations or requirements set forth in <u>Section 4.3</u>, <u>Use-Specific Standards</u>, for the PD district. Nothing shall limit an applicant from seeking to modify an otherwise applicable use-specific standard in accordance with the standards in <u>Section 3.5.5.B.2</u>, <u>Master Plan Required</u>.

C. PLANNED DEVELOPMENT TERMS AND CONDITIONS

The terms and conditions document shall incorporate by reference or include, but not be limited to:

- 1. Conditions related to approval of the application for the PD zoning district classification;
- **2.** The master plan, including any density/intensity standards, dimensional standards, and development standards established in the master plan;
- **3.** Conditions related to the approval of the master plan, including any conditions related to the form and design of development shown in the master plan;
- **4.** Provisions addressing how transportation, potable water, wastewater, stormwater management, and other infrastructure will be provided to accommodate the proposed development;
- 5. Provisions related to environmental protection and monitoring; and
- **6.** Any other provisions the Board of Commissioners determines are relevant and necessary to the development of the PD in accordance with applicable standards and regulations.

D. AMENDMENTS TO APPROVED MASTER PLAN

Amendments or modifications to a master plan shall be considered in accordance with the standards in <u>Section 2.2.13</u>, <u>Planned Development</u>.

Attachment 14 PD-2024-01

Adopted 06.07.2021

Suburban Residential

This designation is for residential areas where suburban character is established and preserved by achieving a balance between buildings and other site improvements relative to the degree of open space maintained within the neighborhood. The openness may be found in relatively large yard areas on individual lots and between homes and/or in common green spaces or water features. This distinguishes suburban character areas from more auto-oriented areas where site coverage in the form of dwellings, driveways and other paved surfaces predominates over open space.

Primary Land Use Types

- Detached residential dwellings.
- Planned developments that integrate other housing types (e.g., attached residential such as patio homes or townhomes), with increased open space to preserve an overall suburban character.
- Golf course subdivisions.



Characteristics

- Less noticeable accommodation of the automobile compared to more intensive autooriented areas, especially where driveways are on the side of homes rather than occupying a portion of the front yard and where garages are situated to the side or rear of the dwelling.
- A larger baseline minimum lot size in a Suburban Residential zoning district allows for deeper front yards and building setbacks and greater side separation between homes.
- Character-based zoning and development standards can also discourage overly standardized subdivision designs and promote conservation design by allowing for smaller lot sizes than the baseline in exchange for greater open space set-aside. This approach enables some viable use of sites partially constrained by topography or other factors. It also provides flexibility for additional housing forms that blend with the area's suburban residential character through additional on-site open space and perimeter buffering where differing housing types and densities are adjacent.
- More opportunity for natural and/or swale drainage (and storm water retention/absorption) relative to concentrated storm water conveyance in auto-oriented areas.

Subdivisions around Pippin Road in north Zebulon.

Where on the Map

Extensive coverage on the map, surrounding much of the core area of Zebulon in most directions, and all the way to the edge of the larger planning area in some locations.



General Residential

This designation covers areas of primarily single-family detached residential use where accommodation of the automobile is more visually dominant relative to more prominent green space in Suburban Residential character areas. This is typically due to relatively smaller and narrower lots, and often with limited open space set-asides or amenities for residents.

Primary Land Use Types

- Detached residential dwellings.
- Other attached residential forms (e.g., duplexes and multiplexes, patio homes, townhomes, multi-family, etc.) as permitted by zoning.
- Planned developments, potentially with a mix of housing types and varying densities, subject to compatibility and open space standards.



Characteristics

- Residential neighborhoods with less openness and separation between dwellings compared to Suburban Residential areas.
- Auto Urban character, especially where driveways and front-loading garages dominate the front yards and front facades of homes. This can be offset by landscaping, "anti-monotony" architectural standards, and limitations on "cookie cutter" subdivision layouts characterized by straight streets and uniform lot sizes and arrangement.
- Neighborhood-oriented commercial uses may emerge over time and should be encouraged on corner sites or other locations at the edge of predominantly residential areas, at a scale and with a site design that is compatible with nearby residential uses.

Shepard's Point subdivision along Old Bunn Road, adjacent to the Hamilton Acres subdivision to the east.

Where on the Map

In some central areas of Zebulon, such as west of N.C. 96 and north of West. Gannon Avenue. Also provides a transition to Suburban Residential areas in other locations, such as north of U.S. 64/ 264 and southwest of town.



