

TOWN OF ZEBULON JOINT PUBLIC HEARING AGENDA BOARD OF COMMISSIONERS AND PLANNING BOARD March 13, 2023 6:00 PM

- I. CALL TO ORDER
- II. APPROVAL OF THE AGENDA
- III. PUBLIC HEARING
 - **A.** CZ -2023-02 0 Weavers Pond Dr (Wall Purdy Tract) Conditional rezoning request for a 43.61-acre parcel from R-40W (Wake County Zoning) to Residential Neighborhood Conditional (R4-C) District for the development of an 87-lot residential subdivision.
- IV. ADJOURNMENT



STAFF REPORT CONDITIONAL ZONING 2023-02 0 WEAVERS POND DR MARCH 13, 2023

Topic:

CZ 2023-02 - 0 Weavers Pond Dr (The Wall Purdy Tract)

Speaker: From:

Michael J. Clark, CZO, AICP, Planning Director Michael J. Clark, CZO, AICP, Planning Director

Prepared by: Approved by: Maron H. Chalker, CZO, Planner II Joseph M. Moore II, PE, Town Manager

Executive Summary:

The Board of Commissioners will consider a Conditional Zoning Map Amendment for 0 Weavers Pond Dr (PIN# 1797701367). This is a legislative case.

Background:

The Applicant, Weaver's Pond Development Company, LLC, requests rezoning a 43.61-acre parcel from R-40W (Wake County zoning) to Residential Neighborhood - Conditional (R4-C) District for the development of an 87-lot residential subdivision. This property is currently outside of the Town's Planning Jurisdiction and is seeking annexation simultaneously with this rezoning application.

The property is adjacent to the Weaver's Pond subdivision and proposes connection through the extension of two Town-maintained stub streets (Weavers Pond Dr and Yulee Dr).

Discussion:

Unified Development Ordinance (UDO) Section 2.2.6.K provides the following standards for

the Board to base their decision on the rezoning request:

- Whether the proposed conditional rezoning advances the public health, safety, or welfare:
- 2. Whether and the extent to which the proposed conditional rezoning is appropriate for its proposed location, and is consistent with the purposes, goals, objectives, and policies of the Town's adopted policy guidance;
- 3. Whether an approval of the conditional rezoning is reasonable and in the public interest;
- 4. Whether and the extent to which the concept plan associated with the conditional rezoning is consistent with this Ordinance; and
- 5. Any other factors as the Board of Commissioners may determine to be relevant.

Policy Analysis:

Comprehensive Land Use Plan:

The Future Land Use and Character Map designates the future use of the property as Rural Conservation (RC) because part of the original tract of land is within the watershed protection area designated for the Little River Reservoir. The property was subdivided and the area requesting rezoning is not within the watershed protection area.

Approval of this rezoning application would amend the Future Land Use and Character Map for this parcel to Suburban Residential (SR) which is intended to be less dense and preserve more open space than what is typical in an urban residential setting.



STAFF REPORT CONDITIONAL ZONING 2023-02 0 WEAVERS POND DR MARCH 13, 2023

Unified Development Ordinance:

The Applicant proposes limiting the uses to single-family detached dwelling, community garden, park (public or private) and utility, minor. The Applicant has proposed site design and architecture conditions for the Board to consider (see Attachments).

All conditions and details on the concept plan meet the spirit and intent of the UDO. If approved, the applicant would proceed with Technical Review Committee (TRC) review of final site plan and construction drawings before development can begin.

Financial Analysis:

Rezoning the property to R4-C will allow the applicant to develop 87 single-family detached homes. Based on data from the Wake County Tax Administration, the median tax value of a single-family residence in Zebulon as of January 1, 2022 is \$216,181. Under this assumption, each home would generate \$1,243.04 in tax revenue a year, or a total of \$108,144.48 for 87 homes.

The subject property is contiguous to the Town's corporate boundaries. If approved, the 87 new homes would require the extension of Town services outside the current service boundary for Public Works, Police, and Code Enforcement. The proposed development is currently within the Fire service boundary. For Public Works, extended services include residential trash and recycling, street light electricity, and street maintenance. For Police, every 150 new homes generate the need for a new officer, priced at approximately \$125,000 including salary and equipment. This development proposal equals 58% of the demand for a new officer. Additionally, response times for Police and Code Enforcement will increase.

Any infrastructure extension and connection costs would be paid by the developer when the property is developed.

Staff Recommendation:

Staff recommends seeking public input during a joint public hearing and referring the matter to the Planning Board for recommendation.

Attachments:

- 1. Application, Site Plan, Renderings, and TIA
- 2. Future Land Use and Character Map
- 3. Aerial Map
- 4. Zoning Map
- 5. Little River Watershed Map
- 6. Site Pictures
- 7. Public Hearing Notification Affidavit



APPLICATION FOR CONDITIONAL REZONING MAP AMENDMENT

PART 1. DESCRIPTION OF REQUEST/PH	ROP	PERTY		MEQUEE.		
Street Address of the Property:			Acreage: 43.61			
0 Weavers Pond Dr Parcel Identification Number (NC PIN):	-	Deed Book:	Deed Page(s):			
1797701367		014676	00016			
Existing Zoning of the Property:		Proposed Zoning of the Property:				
R40W (Wake County) Existing Use of the Property:	-	Proposed Use of the Property:				
Vacant Parcel		Single family d	etached			
The purpose of the rezoning is to facilitate a si containing up to 2.0 dwelling units per acre wit with the requirements of the Town of Zebulon I	th as	ssociated amenities and o	open space co			
PART 2. APPLICANT/AGENT INFORMA Name of Applicant/Agent: Weaver's Pond Development Compa Street Address of Applicant/Agent: 4020 Wake Forest Rd, STE 102F			THE SECTION			
City: Raleigh		State: North Carolina	Zip Code: 27609-522	1		
Email of Applicant/Agent: jbarron@morningstarlawgroup.co	om	Telephone Number of Applicant/Agent: 919-590-0371	Fax Number of Applic	ant/Agent:		
Are you the owner of the property? Yes No Are you the owner's agent? Yes	No	Note: If you are not the owner of the property, you must obtain the Owner's consent and signature giving you permission to submit this application.				
PART 3. PROPERTY OWNER INFORMA	TIC)N		in a m		
Name of Property Owner: Wall Purdy Family LLC						
Street Address of Property Owner:	-					
3309 Felton Pl.						
City: Raleigh	State:	th Carolina	Zip Code: 27612-5001			
Email of Property Owner:	Telep	shone Number of Property Owner:	Fax Number of Property	ty Owner:		
jbblack3@gmail.com_		9-880-2029				
I hereby state that the facts related in this application a correct, and accurate to the best of my knowledge.	and a	ny documents submitted here	ewith are comple	te, true,		
Signature of Applicant:		Print Name:		Date:		
		Weaver's Pond Development Company, L	LC; Grey Berry, Agent			
Signature of Owner:		Print Name:		Date:		
My elis Droh		Phyllis Purdy				

APPLICATION FOR

CONDITIONAL REZONING MAP AMENDMENT LEGISLATIVE CONSIDERATIONS – CONDITIONAL REZONING

The applicant shall propose site-specific standards and conditions that take into account the following considerations, which are considerations that are relevant to the legislative determination of whether or not the proposed conditional zoning district rezoning is in the public interest. Therese considerations do not exclude the legislative consideration of any other factor that is relevant to the public interest. Failure to adequately address the findings below may result in denial of the application. Please attach additional pages if necessary. The petition is justified based on the facts as they relate to the Standards in Section 2.2.6.K of the UDO as follows:

1. Please explain how the proposed Conditional Rezoning advances the public health, safety, or welfare

The subject property is currently zoned R40W in Wake County. The proposed conditional rezoning to R4-C will bring much needed housing to the area while conserving the rural aesthetics of the district as designated in the Future Land Use an Character map.

2. Please explain how the proposed conditional rezoning is appropriate for its proposed location, and is consistent with the purposes, goals, objectives, and policies of the town's adopted policy guidance;

The Zebulon Comprehensive Plan Future Land Use and Character map designates the subject property as a Rural Conservation (RC) district. The applicant acknowledges that the proposed rezoning requires an amendment to the Comprehensive Plan. However, the applicant submits the change is reasonable and in the public interest. First, the subject property abuts Weaver's Pond to the south and east. Weaver's Pond has been developed to include single family detached

3. Please explain how an approval of the conditional rezoning is reasonable and in the public interest;

The proposed conditional rezoning will bring much needed housing to the area and will do so in a manner that is complimentary to the existing Weaver's Pond community, which borders the subject property to the south and the east.

4. Please explain how the concept plan associated with the conditional rezoning is consistent with this Ordinance; and

The concept plan associated with the conditional rezoning is consistent with the provisions of Chapter 5 and Chapter 6 of the UDO, along with the other relevant provisions. The plan has been developed to include access and circulation consistent with the ordinance requirements. Further, the concept plan will ensure a high quality development that addresses the open space, design and other key features of the UDO.

5. Please explain how the proposed conditional rezoning addresses any other factors as the Board of Commissioners may determine to be relevant. These include but are not limited to the proposed uses requested and any requested deviations and proposed alternative means of compliance.

The proposed rezoning provides conditions that require the proposed development to meet or exceed the requirements of the Town of Zebulon's Unified Development Ordinance. The conditions can be found on the attached conditions page.



APPLICATION FOR CONDITIONAL REZONING MAP AMENDMENT

PROPOSED CONDITIONAL USES

An application l	has been	duly filed	requesting	that t	he p	property	described	in th	nis ap	plication	be	rezoned	from
R40W (Wake County)		to R4-C	:			. It	is understo	od and	l ackn	owledged	that	if the pro	operty
is rezoned as requ	ested, the	property de	scribed in th	is requ	est w	vill be pe	rpetually b	ound t	o the	use(s) aut	aoriz	zed and si	ubject
to such condition	s as impo	sed, unless	subsequent	ly char	nged	or ame	nded as pro	ovided	for	in the Un	ified	Develop	pment
Ordinance. It is fu	irther und	erstood and	acknowledg	ged that	fina	d plans fo	or any spec	ific de	evelop	pment to b	e ma	ide pursu	ant to
any such Condition	onal Zonin	g shall be s	ubmitted for	site or	subc	division	plan approv	al. Us	se add	litional pa	ges a	is needed	l.

The Rezoned Lands may be used for, and only for, the uses listed immediately below. The permitted uses are subject to the limitations and regulations stated in the Use Table and any additional limitations or regulations stated below. For convenience, some relevant sections of the Unified Development may be referenced; such references do not imply that other sections of the Unified Development Ordinance do not apply.

1.	single family detached dwelling	25.	
2.	community garden	26.	
3.	park (public or private)	27.	
4.	utility, minor	28.	
5.		29.	
6.		30.	
7.		31.	
8.		32.	
9.		33.	
10.		34.	
11.		35.	
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23.		47.	
24.		48.	



APPLICATION FOR CONDITIONAL REZONING MAP AMENDMENT

OWNER'S CONSENT FORM

Name of Project:	The Wall Pu	urdy Tract	Submittal Date:	
OWNER'S AUTHOR I hereby give CONSEN full name of agent) to act documents, and to atten-	T to Weaver's Pond Develop t on my behalf, to sub	mit or have submitted	this application a	ype, stamp or print clearly nd all required material and aining to the application(s)
indicated above. Further conditions which may ar	ermore, I hereby give	e consent to the party	designated abov	e to agree to all terms and
I acknowledge and agree Ordinance, that lands sultapproved as part of that the land as an amendment with the procedures establimits shall comply with all other applicable stanspecifically listed as conincomplete information withdrawal of this application application of the process this accopyrighted document succonditions, which may be	be that, pursuant to Se bject to a conditional application. These state to this Ordinance and blished in this Ordinance and all Town policies reladards and regulation additions or deviations provided by me or cation, request, appropriation. I further ubmitted as a part of the imposed as part of the	Section 2.2.6 M. of the rezoning shall be subjundantly, plans, and applied the Official Zoning and the Official Zoning and the Official Zoning and the Official Zoning and the UDO will result as part of this request my agent will result val or permits. I acknowled the approval of this application for any the approval of this application of the subject to the transfer of th	he Town of Zebrect to all the stand proved conditions Map, and may only ated outside the Ted d the extension of emain applicable st. I understand the in the denial, re- nowledge that add of Zebulon to puby third party. I fur plication.	e subject of this application. ulon Unified Development dards, conditions, and plans are perpetually binding on by be changed in accordance own of Zebulon's corporate f utilities. I understand that to the subject lands unless that any false, inaccurate or evocation or administrative litional information may be lish, copy or reproduce any arther agree to all terms and
Signature of Ov	why wner	Phyllis Purdy Print Name	<u> </u>	- 2 % - 20 2 2 Date
	ments or information knowledge. I unders	made in any paper or tand this application,	related material a	nerewith are true and all attachments become and will not be returned.
Dune Om	ely	Phyllis Purdy	/	7-28-2022

Print Name

^{*}Owner of record as shown by the Wake County Revenue Department (<u>www.wakegov.com</u>). An option to purchase does not constitute ownership. If ownership has been recently transferred, a copy of the deed must accompany this form.

- 1. ALL LOTS SHALL BE A MINIMUM OF 8,700 SQUARE FEET.
- 2. ALL LOT WIDTHS SHALL BE A MINIMUM OF 70'.
- 3. ALL DWELLINGS WILL HAVE A MINIMUM TWO-CAR GARAGE.
- 4. GARAGE DOORS WILL HAVE WINDOWS AND CARRIAGE HARDWARE.
- 5. GARAGES: GARAGE DOORS SHALL BE RECESSED BEHIND THE FRONT PLAIN OF THE HOME A MINIMUM OF 8". WHERE A HOME PROVIDES A FRONT PORCH, THE GARAGE MAY EXTEND BEYOND THE FRONT PLAIN OF THE HOME, PROVIDED THE FRONT PORCH EXTENDS BEYOND THE FRONT PLAIN OF THE GARAGE A MINIMUM OF 1'.
- 6. SIDE LOADED GARAGES SHALL BE REQUIRED ON A MINIMUM OF 20% OF THE HOMES CONSTRUCTED. SIDE LOADED GARAGES AND "J" DRIVEWAYS SHALL BE ALLOWED A SIDE SETBACK OF 5'.
- 7. FOR ALL LOTS, THE ENTIRE YARD WILL BE SODDED.
- 8. EXTERIOR BUILDING MATERIALS: EXTERIOR SIDING WILL BE PRIMARILY FIBER CEMENT WITH BRICK OR STONE ACCENTS. THE USE OF VINYL SIDING SHALL BE PROHIBITED, EXCEPT FOR TRIM ELEMENTS OF THE DWELLING UNIT FACADE. SIDING STYLES WILL INCLUDE HORIZONTAL, SHAKE, OR BOARD AND BATTEN DESIGN. AT LEAST TWO (2) OF THE FOLLOWING MATERIALS SHALL BE USED ON EACH UNIT FIBER-CEMENT, MASONRY BRICK, BRICK VENEER, MASONRY STONE, STONE VENEER. OR SYNTHETIC STONE.
- 9. FOUNDATIONS: FOUNDATIONS SHALL BE RAISED ABOVE THE FINISHED GRADE AS MEASURED ALONG THE FRONT, STREET FACING FINISHED GRADE OF THE BUILDING PAD A MINIMUM OF 18". FOUNDATION TYPES TO BE MAY INCLUDE, STEM WALL, RAISED SLAB, OR CRAWL SPACE.
- 10. AMENITIES WILL INCLUDE A DOG PARK, WALKING TRAILS, AND MAINTAINED OPEN SPACE. ALL OPEN SPACE AND AMENITIES WILL BE MAINTAINED BY THE HOA
- 11. A MINIMUM OF 8" ROOF OVERHANG SHALL BE PROVIDED ALONG THE FRONT AND BACK OF EACH DWELLING UNIT.
- 12. A 10-FOOT UNDISTURBED BUFFER WILL BE MAINTAINED AROUND THE DEVELOPMENT. WHERE EXISTING PLANTS DO NOT MEET THE REQUIREMENTS OF THE UDO, PLANTINGS WILL BE SUPPLEMENTED TO MEET THE REQUIREMENT.
- 13. REQUIRED DECORATIVE FEATURES EACH UNIT SHALL UTILIZE AT LEAST ONE OF THE FOLLOWING: A DECORATIVE FRONT DOOR (MINIMUM 25% GLAZING); WINDOW TRANSOM, DOOR SIDELIGHTS, OR DOOR TRANSOM.
- 14. WINDOW TREATMENTS: WINDOWS ON FRONT ELEVATIONS SHALL OFFER EITHER TRIM OR SHUTTERS. TRIM ALONG HEADERS AND SILLS SHALL BE A MINIMUM OF 3" WIDE. SHUTTERS ARE DECORATIVE AND

- MAY OR MAY NOT BE "OPERATIONAL". SHUTTERS SHALL HAVE A MINIMUM WIDTH OF 18".
- 15. PORCHES: FRONT PORCHES SHALL EXTEND BEYOND THE FRONT PLAIN OF THE GARAGE ON 20% OF THE HOMES CONSTRUCTED. FRONT PORCHES SHALL BE ALLOWED TO EXTEND BEYOND THE FRONT SETBACK OF THE BUILDING ENVELOP A MAXIMUM OF 10'.
- 16. FRONT PORCHES SHALL WRAP AROUND THE CORNER OF THE FRONT FAÇADE ON A MINIMUM OF 20% OF THE HOMES CONSTRUCTED.
- 17. ALL HOMES WILL HAVE A REAR PATIO OR DECK OF AT LEAST 100 SQUARE FEET.
- 18. ACCESSORY BUILDINGS SHALL BE CONSTRUCTED OF MATERIALS THAT MATCH THE SINGLE-FAMILY DWELLING.
- 19. IN ORDER TO PROMOTE VARIATION IN HOME APPEARANCE, NO FRONT ELEVATION OR PRIMARY SIDING COLOR SHALL BE CONSTRUCTED WITHIN TWO HOUSES OF AN IDENTICAL ELEVATION OR PRIMARY SIDING COLOR ON THE SAME SIDE OF THE STREET OR ACROSS THE STREET. FOR CORNER LOTS, NO IDENTICAL ELEVATION OR PRIMARY SIDING COLOR WILL BE CONSTRUCTED DIAGONALLY ACROSS AN INTERSECTION.
- 20. HOMEOWNERS ASSOCIATION WILL LIMIT THE NUMBER OF RENTAL HOMES TO A MAXIMUM OF 20%. THIS RESTRICTION SHALL BE RECORED IN HOA COVENATS, CONDITIONS AND RESTRICTIONS.
- 21. ALL HOME DESIGN AND CONSTRUCTION SHALL CONFORM TO SECTION 5.2 OF THE TOWN OF ZEBULON'S UNIFIED DEVELOPEMENT ORDINACE.

SITE DATA

OWNER/DEVELOPER:

WALL PURDY FAMILY LLC JAMES B BLACK III (AGENT) 3309 FELTON PL

RALEIGH NC 27612-5001

2620 ZEBULON RD STREET ADDRESS: ZEBULON, NC

PROPOSED PROJECT AREA: 43.61 AC

CURRENT ZONING: R40W

1797613206 PARCEL IDENTIFICATION NUMBER:

MINIMUM REQUIRED LOT WIDTH: 70' TOTAL # LOTS

OPEN SPACE CALCULATIONS

OPEN SPACE

TOTAL REQUIRED O.S. = 10% OF THE PROJECT AREA = 10% X 43.61 AC = 4.36 AC

TOTAL O.S. PROVIDED = 10.59 AC

TOTAL ACTIVE O.S. REQURED = 25% OF THE TOTAL REQUIRED O.S. = 25% X 4.36 AC = 1.09 AC

TOTAL ACTIVE O.S. PROVIDED = 1.5 AC

SECTIONAL VIEW

* 2:1 MAX., 3:1 RECOMMENDED

NOTES:
1. SIDEWALK TO BE PROVIDED ON BOTH SIDES OF THE STREET PER DETAIL ABOVE.

CUL-DE-SAC 26' N/A 3" 8" 50'

PRELIMINARY NOT FOR CONSTRUCTION

PIEDMONT LANDDESIGN LLP

RALEIGH, NORTH CAROLINA 27615 919.845.7600 PHONE 919.845.7703 FAX ENGR. FIRM LICENSE NO. F-0843

8522-204 SIX FORKS ROAD

PROPERT

2620 ZEE ZEBULO

ISSUED: **01 AUG 2022**

REVISIONS:

AMIL

1 20 SEPT 2022 PER TOWN COMMENTS

2 7 NOV 2022 PER TOWN COMMENTS

3 28 NOV 2022 PER TOWN COMMENTS

13 JAN 2023 PER TOWN COMMENTS

DRAWN BY: **JET** CHECKED BY: MLS

PROJECT: **FDCWP9**

CONCEPTUAL LOT LAYOUT **EXHIBIT**

DWG. NO. **EX 1.1**

NORTH SCALE IN FEET



8522-204 SIX FORKS ROAD RALEIGH, NORTH CAROLINA 27615 919.845.7600 PHONE 919.845.7703 FAX ENGR. FIRM LICENSE NO. F-0843

PRELIMINARY NOT FOR CONSTRUCTION

PROPERT

2620 ZEBULON RD ZEBULON, NC

ISSUED: **24 OCT 2022**

REVISIONS:

FAMILY,

1 7 NOV 2022 PER TOWN COMMENTS

28 NOV 2022 PER TOWN COMMENTS

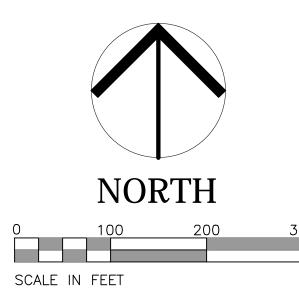
3 13 JAN 2023 PER TOWN COMMENTS

DRAWN BY: **JET** CHECKED BY: **MLS**

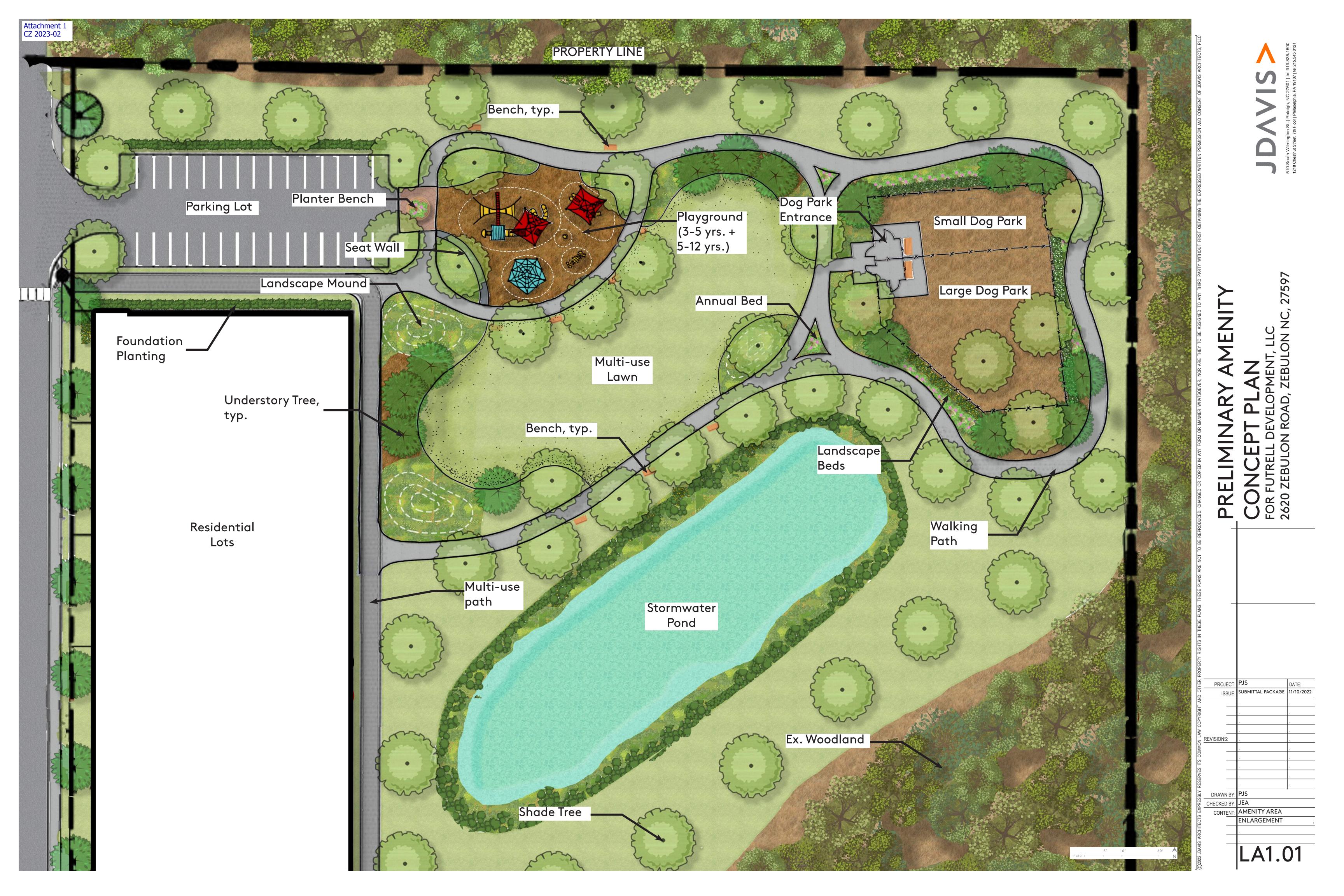
PROJECT: **FDCWP9**

CONCEPTUAL STORM **DRAINAGE PLAN**

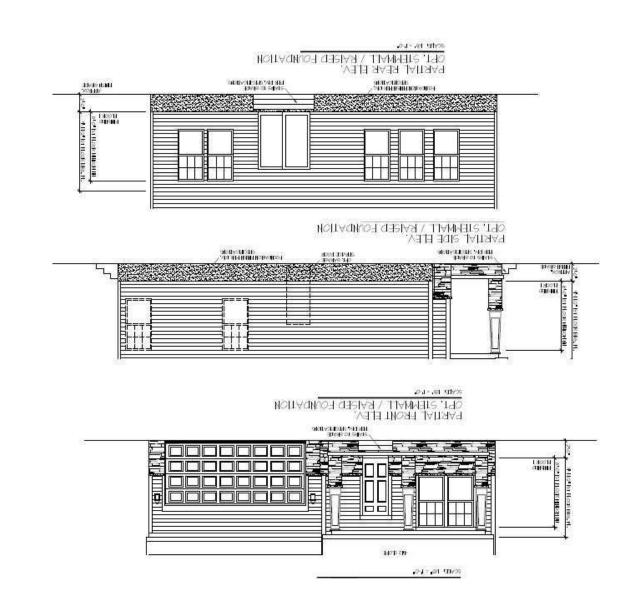
DWG. NO. **EX 2.0**





































RAMEY KEMP ASSOCIATES

TOGETHER WE ARE LIMITLESS



July 29, 2022

Mr. Meade Bradshaw Assistant Planning Director Town of Zebulon 1003 N. Arendell Avenue Zebulon, NC 27597 P: (919) 269-7455

E: mbradshaw@townofzebulon.org

Subject: Traffic Assessment

Weaver's Pointe - Zebulon, North Carolina

Dear Mr. Bradshaw:

This letter provides a summary of a Traffic Assessment prepared for the Weaver's Pointe residential development located north of Pippin Road and east of NC 96 (Zebulon Road) in Zebulon, North Carolina. Refer to the attachments for the site location map. The purpose of the study is to determine how traffic generated by the proposed development is expected to impact the surrounding roadways and intersections.

The proposed additional phase is expected to consist of 105 single-family homes and is anticipated to be completed by 2024. Site access will be provided via connections to the existing Weaver's Pond and Weaver's Ridge developments via Yulee Drive and Golden Plum Lane.

Refer to the attachments for a copy of the preliminary site plan and for an illustration of the existing lane configurations within the study area.

Study Area

Based on coordination with the Town of Zebulon (Town) and the North Carolina Department of Transportation (NCDOT), the study area consists of the following intersections:

- NC 96 (Zebulon Road) & Pippin Road
- NC 96 (Zebulon Road) & Glory Road
- Pippin Road & Pearces Road

Analysis Scenarios

All capacity analyses were performed utilizing Synchro (Version 10.3). All study intersections were analyzed during the weekday AM and PM peak hours and PM school peak hours under the following proposed traffic scenarios:

- 2022 Existing Traffic Conditions
- 2025 (Build +1) No-Build Traffic Conditions
- 2025 (Build +1) Build Traffic Conditions



TOGETHER WE ARE LIMITLESS

- Attachment 1 - CZ 2023-02

2022 Existing Traffic Volumes

Through coordination with the Town and NCDOT, existing peak hour traffic volumes were determined based on previously conducted turning movement counts collected as part of the Weaver's Pond and Weaver's Ridge traffic studies. Previously collected turning movement counts were grown from the year collected to the 2022 existing analysis year using a 3% annual growth rate.

Peak hour turning movement counts were conducted at the following study intersections during the weekday AM and PM peak hours at the listed dates:

- NC 96 (Zebulon Road) & Pippin Road August 30th, 2017
- NC 96 (Zebulon Road) & Glory Road April 10th, 2019
- Pippin Road & Pearces Road October 3rd, 2017

It should be noted that the Weaver's Pond development is currently fully build-out with the exception of Phase 5 (73 single-family homes). Therefore, the traffic associated with the currently built portion of the development was added to the grown traffic counts to accurately model existing traffic conditions. Refer to Table 1 below, for a breakdown of the expected trip generation of Weaver's Pond and how the expected development trips were applied in this study.

Land Use (ITE Code)	Intensity	Daily Traffic	AM Pea Trips		PM Peak Hour Trips (vph)		
(112 code)		(vpd)	Enter	Exit	Enter	Exit	
Weaver's Pond	Full Buildou	ıt (From W	eaver's Po	nd TIA)			
Single-Family Housing (210)	525 DU	5,000	99	295	331	194	
Townhouse (230)	55 DU	400	4	20	19	10	
Total		5,400	103	315	350	204	
Weave	er's Pond Pl	nase 5 (To 1	Be Built)				
Single-Family Housing (210) (Approx. 14% of Total SFH)	73 DU	695	14	41	46	27	
Existing Development (Currently Built)							
Existing Weaver's Pond Develo (As of 2022)	Existing Weaver's Pond Development			274	304	177	

Table 1: Weaver's Pond Development Traffic

Volumes were balanced along NC 96 (Zebulon Road) to account for any variance associated with the different count dates. Volumes were not balanced along Pippin Road due to the reasonable imbalance in comparison to the land uses between the intersections along Pippin Road. A copy of the count data is attached to this report. Refer to the attachments for an illustration of the 2022 existing weekday AM and PM peak hour traffic volumes.



Background Traffic Volumes

Based on a review of traffic growth patterns and adjacent development information, background traffic volumes were determined by projecting 2022 existing traffic volumes to the 2025 build-out year using a 3% annual growth rate. It should be noted that the balanced 2022 existing traffic volumes were not rebalanced after the growth rate was applied for the 2025 projected traffic volumes, which can result in minor vehicles imbalances due to rounding. Refer to the attachments for an illustration of the 2025 projected peak hour traffic volumes. Through coordination with the Town and NCDOT, the following adjacent developments were identified to be included in this study:

- Weaver's Pond (Phase 5)
- Weaver's Ridge
- Taryn Lake & Taryn Creek
- Pearces Road

Refer to the attachments for an illustration of the adjacent development peak hour traffic volumes.

It should be noted that per the completed traffic study for Weaver's Ridge, it was assumed that a portion of the Weaver's Pond development site traffic would be rerouted to utilize the future site driveway along NC 96 (Zebulon Road). Refer to the attachments for an illustrations of this rerouted development traffic.

Future Improvements

Through coordination with the Town and NCDOT, it was determined that the future improvements associated with Weaver's Ridge should be considered in the analysis of future conditions. Geometric improvements are expected at both study intersections along NC 96 by these developments.

2025 No-Build Traffic Volumes

The 2025 no-build traffic volumes were determined by projecting the 2022 existing peak hour traffic to the year 2025, and adding the adjacent development trips. Refer to the attachments for an illustration of the 2025 no-build peak hour traffic volumes.

Trip Generation

Average weekday daily, AM peak hour, and PM peak hour trips for the proposed development were estimated using methodology contained within the ITE *Trip Generation Manual*, 11th Edition. Refer to Table 2, for a detailed breakdown of the proposed site trip generation.

Table 2: Trip Generation Summary

Land Use (ITE Code)	Intensity					Peak Hour ps (vph)	
(III couc)		(vpd)	Enter	Exit	Enter	Exit	
Single-Family Housing (210)	105 units	1,055	20	58	66	38	



RAMEY KEMP ASSOCIATES

Traffic Assessment – Weaver's Pointe | Page 4 of 11

TOGETHER WE ARE LIMITLESS Attachment 1

CZ 2023-02

It is estimated that the proposed development will generate approximately 1,055 total site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 78 trips (20 entering and 58 exiting) will occur during the weekday AM peak hour and 104 trips (66 entering and 38 exiting) will occur during the weekday PM peak hour.

Trip Distribution and Assignment

The primary site trips are distributed based on the locations of existing traffic patterns, population centers adjacent to the study area, and engineering judgment. A summary of the overall distributions is below:

- 50% to/from the south via NC 96 (Zebulon Road)
- 25% to/from the south via NC 96 (Zebulon Road)
- 20% to/from the north via Pearces Road
- 5% to/from the south via Pearces Road

It should be noted that the regional distributions were based on the approved distributions from the Weaver's Pond and Weaver's Ridge traffic studies. Refer to the attachments for illustrations of the site trip distribution and site trip assignment, respectively.

2025 Build Traffic

To estimate the 2025 build traffic conditions with the site fully built-out, the total site trips were added to the 2025 no-build traffic volumes. Refer to the attachments for an illustration of the 2025 build peak hour traffic volumes.

Capacity Analysis

Study intersections were analyzed using the methodology outlined in the Highway Capacity Manual (HCM), 6th Edition published by the Transportation Research Board. Capacity and level of service are the design criteria for this traffic study. A computer software package, Synchro (Version 10.3), was used to complete the analyses for most of the study area intersections. Please note that the unsignalized capacity analysis does not provide an overall level of service for an intersection; only delay for an approach with a conflicting movement.

The HCM defines capacity as "the maximum hourly rate at which persons or vehicles can reasonably be expected to traverse a point or uniform section of a lane or roadway during a given time period under prevailing roadway, traffic, and control conditions." Level of service (LOS) is a term used to represent different driving conditions and is defined as a "qualitative measure describing operational conditions within a traffic stream, and their perception by motorists and/or passengers." Level of service varies from Level "A" representing free flow, to Level "F" where breakdown conditions are evident. Refer to Table 3 for HCM levels of service and related average control delay per vehicle for both signalized and unsignalized intersections. Control delay as defined by the HCM includes "initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay". An average control delay of 50 seconds at a signalized intersection results in LOS "D" operation at the intersection.



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Table 3 Highway Capacity Manual - Levels-of-Service and Delay

UNSIGNA	ALIZED INTERSECTION	SIGNALIZED INTERSECTION			
LEVEL OF SERVICE	AVERAGE CONTROL DELAY PER VEHICLE (SECONDS)	LEVEL OF SERVICE	AVERAGE CONTROL DELAY PER VEHICLE (SECONDS)		
A	0-10	A	0-10		
В	10-15	В	10-20		
С	15-25	С	20-35		
D	25-35	D	35-55		
Е	35-50	E	55-80		
F	>50	F	>80		

The study intersections were analyzed under 2022 existing, 2025 no-build, and 2025 build traffic conditions with lane configurations and traffic control shown in Tables 4-6. Refer to Tables 4-6 for a summary of the analysis results. The Synchro capacity analysis reports are attached to this report.



Table 4: Analysis Summary of NC 96 (Zebulon Road) and Pippin Road

ANALYSIS	A P P R	LANE	PEAK	DAY AM HOUR S SERVICE	PEA	CKDAY PM AK HOUR OF SERVICE
SCENARIO	O A C H	CONFIGURATIONS	Approach	Overall (seconds)	Approach	Overall (seconds)
2022 Existing Conditions	WB NB SB	1 LT-RT 1 TH, 1 RT 1 LT, 1 TH	C B B	B (18)	D C B	C (22)
2025 No-Build Conditions	WB NB SB	1 LT-RT 1 TH, 1 RT 1 LT, 1 TH	C B B	C (21)	D C B	C (26)
2025 Build Conditions	WB NB SB	1 LT-RT 1 TH, 1 RT 1 LT, 1 TH	C C B	C (22)	D C B	C (27)

^{1.} Level of service for major-street left-turn movement.

Capacity analysis of 2022 existing, 2025 no-build, and 2025 build traffic conditions indicates that this intersection is expected to operate at an overall LOS C or better during the weekday AM and PM peak hours under all analysis scenarios. Additionally, all intersection approaches are expected to operate at LOS D or better during the weekday AM and PM peak hours. Due to the expected acceptable operation of this intersection upon buildout of the proposed development, no improvements are recommended at this intersection by the development.



^{2.} Level of service for minor-street approach.

Table 5: Analysis Summary of NC 96 (Zebulon Road) and Glory Road/Weaver's Ridge Site Drive

ANALYSIS	A P P R LANE		PEAK	DAY AM K HOUR OF SERVICE	WEEKDAY PM PEAK HOUR LEVEL OF SERVICE		
SCENARIO	O A C H	CONFIGURATIONS	Approach	Overall (seconds)	Approach	Overall (seconds)	
2022 Existing Conditions	EB NB SB	1 LT-RT 1 LT-TH 1 TH-RT	C ² A ¹	N/A	C ² A ¹	N/A	
2025 No-Build Conditions	EB WB NB SB	1 LT- <u>TH</u> -RT <u>1 LT, 1 TH-RT</u> 1 LT-TH, <u>1 RT</u> 1 <u>LT</u> -TH-RT	D ² F ² A ¹	N/A	E^2 F^2 A^1 A^1	N/A	
2025 Build Conditions	EB WB NB SB	1 LT- <u>TH</u> -RT <u>1 LT, 1 TH-RT</u> 1 LT-TH, <u>1 RT</u> 1 <u>LT</u> -TH-RT	D ² F ² A ¹	N/A	F ² F ² A ¹	N/A	
2025 Build Conditions Signalized to meet UDO	EB WB NB SB	1 LT- <u>TH</u> -RT <u>1 LT, 1 TH-RT</u> 1 LT-TH, <u>1 RT</u> 1 <u>LT</u> -TH-RT	D D B B	B (17)	D D A B	B (13)	

Improvements and/or revised lane configurations by Weaver's Ridge are shown <u>underlined</u>. **Improvements and/or revised lane configurations by the development are shown in bold**.

- 1. Level of service for major-street left-turn movement.
- 2. Level of service for minor-street approach.

Capacity analysis of 2022 existing traffic conditions indicates that the major-street left-turn movement on NC 96 (Zebulon Road) are expected to operate at LOS A during the weekday AM and PM peak hours and that the minor-street approach of Glory Road are expected to operate at LOS C during the weekday AM and PM peak hours.

Under future traffic conditions, the Weaver's Ridge development is expected to construct the westbound approach of this intersection and provide a dedicated northbound right-turn lane on NC 96 (Zebulon Road). Under 2025 no-build and 2025 build traffic conditions, the major-street left-turn movement on NC 96 (Zebulon Road) are expected to operate at LOS A during the weekday AM and PM peak hours. The eastbound minor-street approach of Glory Road is expected to operate at LOS D during the weekday AM peak hour and at LOS E/F during the weekday PM peak hour. The westbound minor-street approach of Weaver's Ridge Site Drive is expected to operate at LOS F during both the weekday AM and PM peak hours.

Per the Town's UDO, if a site access operates at LOS D or worse, additional site access points may need to be considered. It should be noted that the proposed Weaver's Pointe development has interconnectivity with the northern section of the Weaver's Pond development which two (2) site access points along Pippin Road. Drivers will likely use an alternative site access if they experience significant delay at the more convenient site access.



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In order to meet the Town's UDO, improvements must be identified to improve the intersection to an acceptable level-of-service. Signalization and/or additional capacity along NC 96 (Zebulon Road) would be necessary for significant improvement at the intersection. A traffic signal was considered at this intersection and 2025 build peak hour traffic volumes were analyzed utilizing the criteria contained in the *Manual on Uniform Traffic Control Devices* (MUTCD). A traffic signal was warranted during both the weekday AM and PM peak hours under 2025 build traffic conditions; however, it is not expected that this intersection would satisfy the 8-hour and 4-hour warrants, which NCDOT favors for installation of a traffic signal. These longer period warrants are not typically met for residential and school areas due to the distinct peak traffic periods for these types of development. For these reasons, signalization is not recommended at this intersection. A traffic signal was analyzed at the intersection to meet the Town's UDO requirements. With a signal, the intersection is expected to operate at LOS D with all approaches operating at LOS D or better during the weekday AM and PM peak hours.

Based on a review of SimTraffic simulations, queues for the westbound minor-street approach are expected to be fairly minor and excessive queueing is not expected on this approach upon buildout of the proposed development without signalization. Therefore, no improvements are recommended at this intersection by the proposed development.



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Table 6: Analysis Summary of Pippin Road and Pearces Road

ANALYSIS	A P P R	LANE	PEAK	DAY AM HOUR F SERVICE	PEA	KDAY PM K HOUR OF SERVICE
SCENARIO	O A C H	CONFIGURATIONS	Approach	Overall (seconds)	Approach	Overall (seconds)
2022 Existing Conditions	EB WB NB SB	1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT	$\begin{array}{c} B^2 \\ B^2 \\ A^1 \\ A^1 \end{array}$	N/A	C^2 C^2 A^1 A^1	N/A
2025 No-Build Conditions	EB WB NB SB	1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT	C ² C ² A ¹	N/A	C ² C ² A ¹	N/A
2025 Build Conditions	EB WB NB SB	1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT	C ² C ² A ¹	N/A	D ² C ² A ¹	N/A

- 1. Level of service for major-street left-turn movement.
- 2. Level of service for minor-street approach.

Capacity analysis of 2022 existing, 2025 no-build, and 2025 build traffic conditions indicates that the major-street left-turn movements on Pearces Road are expected to operate at LOS A during the weekday AM and PM peak hours and that the minor-street approaches of Pippin Road are expected to operate at LOS D or better during the weekday AM and PM peak hours under all analysis scenarios. Due to the expected acceptable operation of this intersection upon buildout of the proposed development, no improvements are recommended at this intersection by the development.



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Recommendations

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Based on the findings of this study, the planned future geometric improvements committed by other developments are expected to accommodate future traffic conditions upon buildout of the proposed development. See a more detailed description of the recommended improvements below. Refer to the attachments for an illustration of the committed roadway improvements by other developments and future lane configurations expected within the study area.

Improvements by Weaver's Ridge

NC 96 (Zebulon Road) and Glory Road / Weaver's Ridge Site Drive

- Provide site access via westbound approach with one ingress lane and two egress lanes striped as one left-turn lane and one shared through/right-turn lane.
- Provide designated northbound right-turn lane with at least 100 feet of storage and appropriate decel and taper.
- Provide designated southbound left-turn lane with at least 50 feet of storage and appropriate decel and taper.
- Monitor intersection for signalization.

Improvements by Weaver's Pointe

NC 96 (Zebulon Road) and Glory Road / Weaver's Ridge Site Drive

• Monitor intersection for signalization.



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Findings and Summary

Based on the findings of this study, specific geometric improvements have been identified and are recommended to accommodate future traffic conditions and to mitigate the development's proportional impact on the surround transportation network.

If you should have any questions, please feel free to contact me at (919) 872-5115.

Sincerely,

Caroline Cheeves, P.E.

Traffic Engineering Project Manager Infrastructure Consulting Services, Inc. dba

Caroline Cheeves

Ramey Kemp Associates

License # F-1489

Attachments: Figures

Traffic Counts

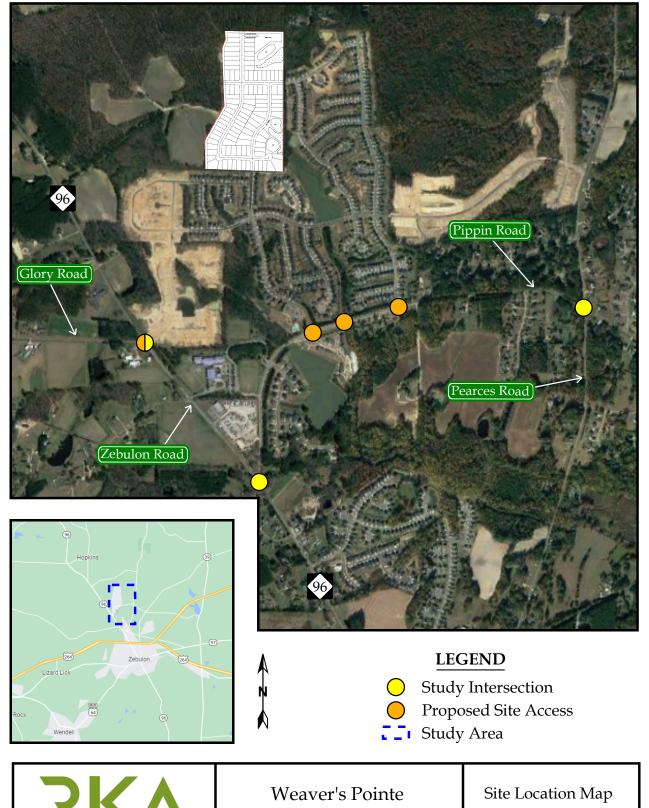
Adjacent Development Information

Synchro Reports

MUTCD Signal Warrant Analysis

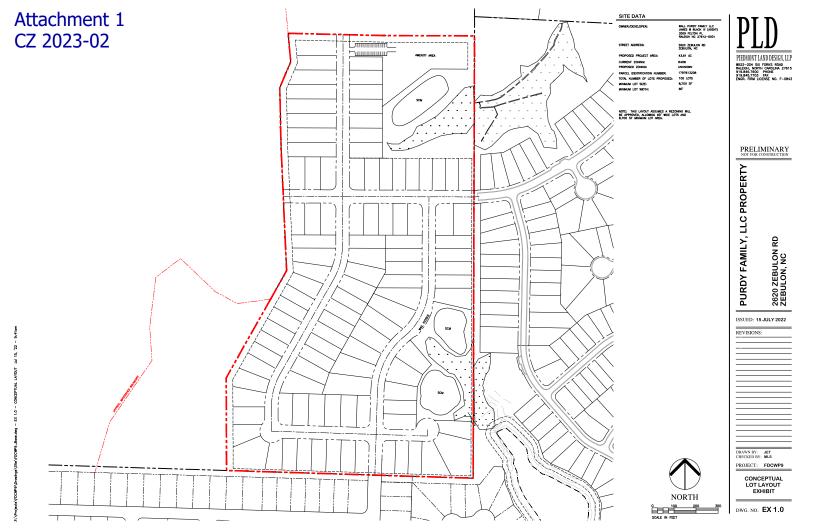


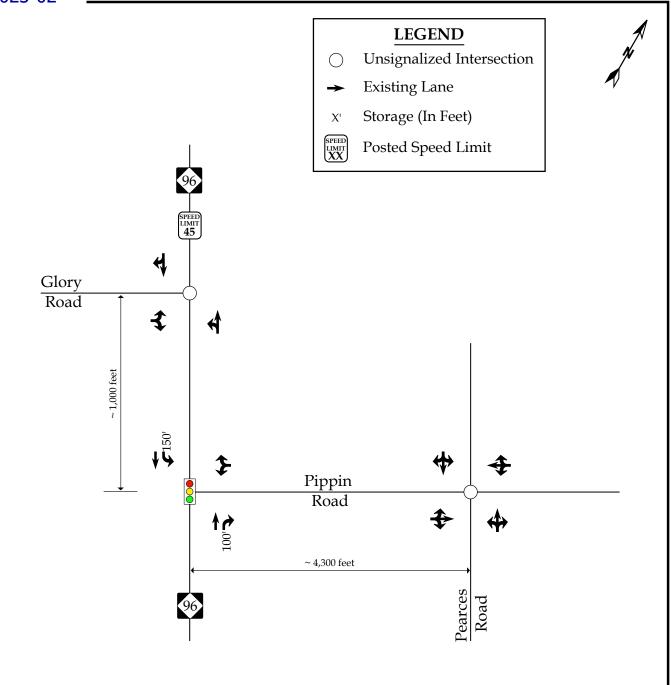






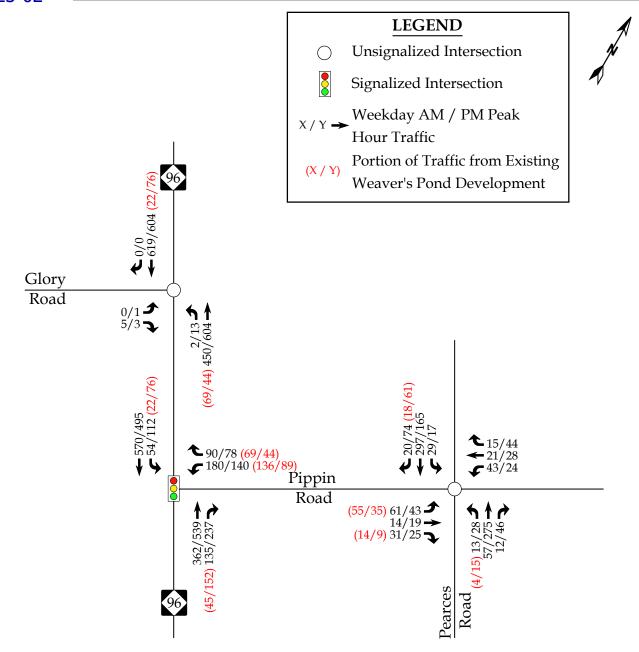
Weaver's Pointe Zebulon, NC







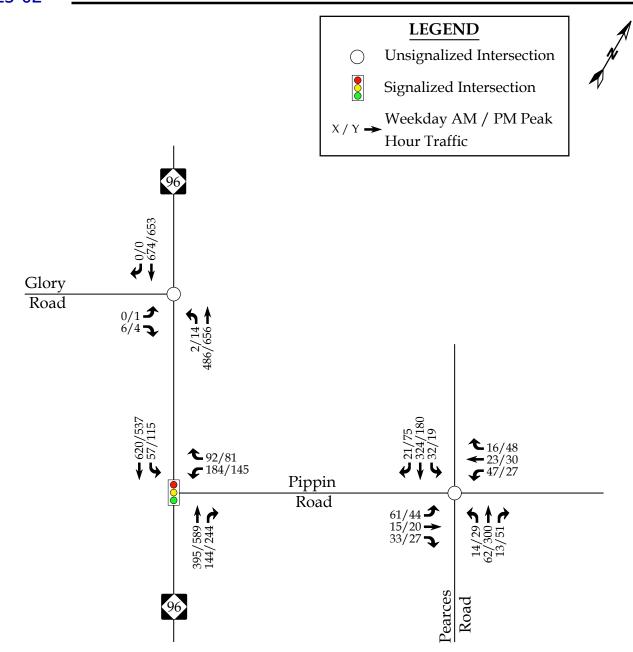
Weaver's Pointe Zebulon, NC 2022 Existing Lane Configurations



Note: Based on NCDOT Congestion Management guidelines, a volume of 4 vehicles per hour (vph) was analyzed for any movement with less than 4 vph.



Weaver's Pointe Zebulon, NC 2022 Existing Peak Hour Traffic



Note: Based on NCDOT Congestion Management guidelines, a volume of 4 vehicles per hour (vph) was analyzed for any movement with less than 4 vph.



Weaver's Pointe Zebulon, NC 2025 Projected Peak Hour Traffic

LEGEND

Unsignalized Intersection

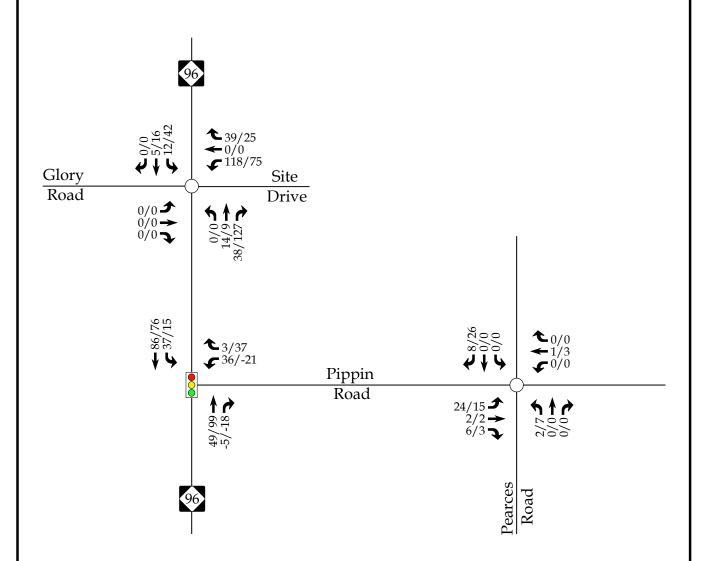


Signalized Intersection

X / Y →

Weekday AM / PM Peak Hour Adjacent Development Trips *Note: Negative numbers are the result of rerouted Weaver's Pond traffic with the addition of the Weaver's Ridge site driveway on NC 96

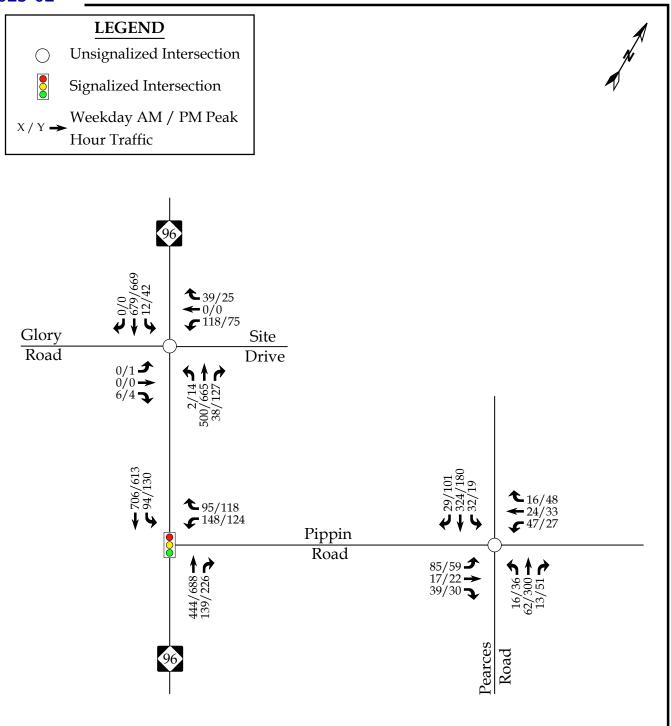






Weaver's Pointe Zebulon, NC

Peak Hour Adjacent Development Trips

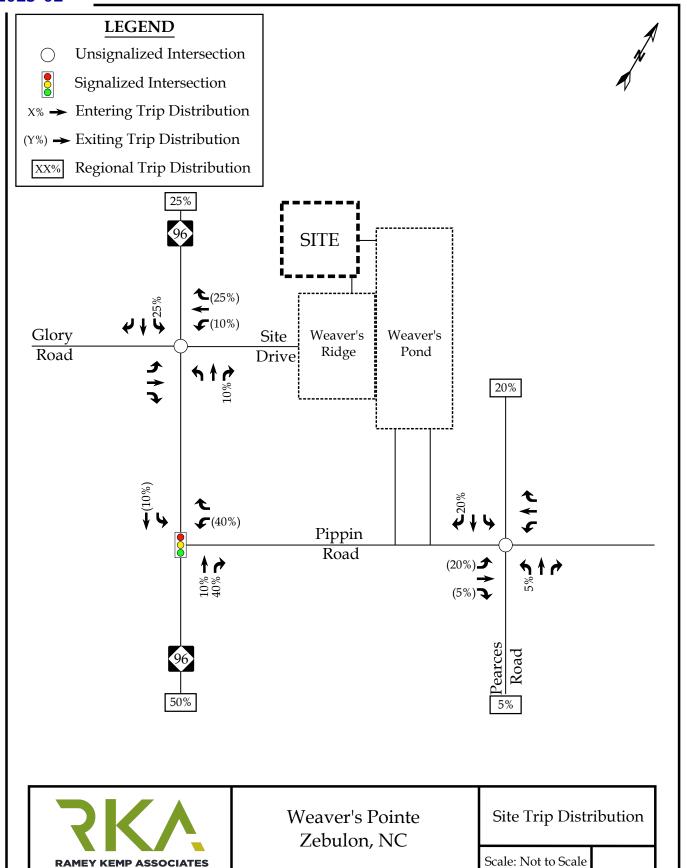


Note: Based on NCDOT Congestion Management guidelines, a volume of 4 vehicles per hour (vph) was analyzed for any movement with less than 4 vph.



Weaver's Pointe Zebulon, NC 2025 No-Build Peak Hour Traffic

RAMEY KEMP ASSOCIATES



LEGEND Unsignalized Intersection Signalized Intersection Weekday AM / PM Peak Hour Site Trips SITE Glory Site Weaver's Weaver's Road Pond Ridge Drive Pippin Road



Weaver's Pointe Zebulon, NC

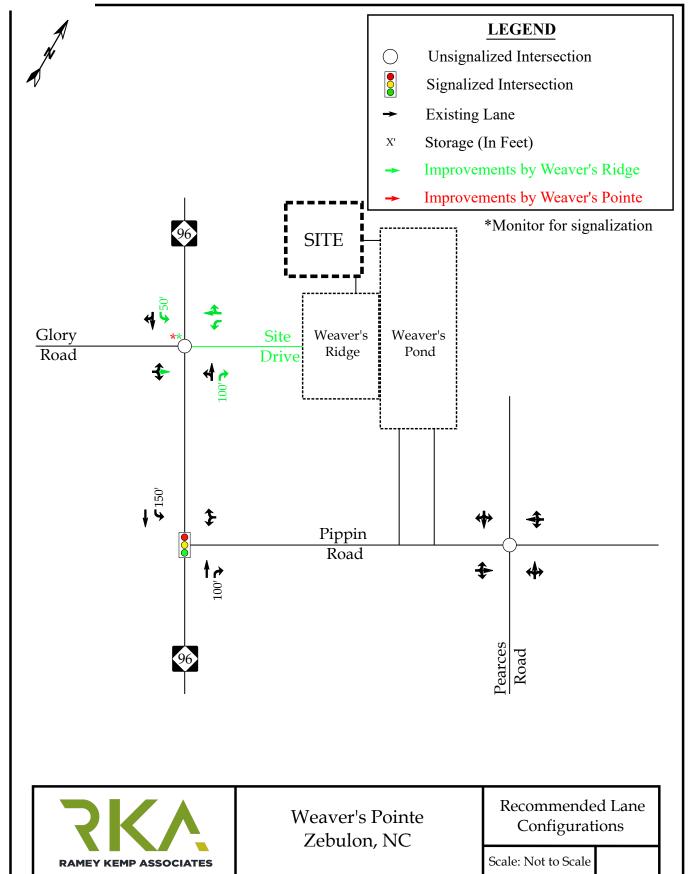
Site Trip Assignment

LEGEND Unsignalized Intersection Signalized Intersection Weekday AM / PM Peak Hour Traffic **SITE** 54/35 0/0124/79 Glory Site Weaver's Weaver's Road Ridge Pond Drive 0/1 **→** © 95/118 © 170/138 Pippin Road 97/67 **→**17/22 **→**42/32 **→**

Note: Based on NCDOT Congestion Management guidelines, a volume of 4 vehicles per hour (vph) was analyzed for any movement with less than 4 vph.



Weaver's Pointe Zebulon, NC 2025 Build Peak Hour Traffic





File Name: NC 96 and Pippin Road

Site Code : 00083017 Start Date : 8/30/2017

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										Gro	ups Pi	rinted	- Car	s & T	rucks	;								
			Pip	pin R	load				NC 9	6									NC 9	6				
			Fr	om N	orth			Fr	om E	ast			Fre	om So	outh			Fr	om W	/est				
	Start Time	Right	Thru	Left	TRKS	App. Total	Right	Thru	Left	TRKS	App. Total	Right	Thru	Left	TRKS	App. Total	Right	Thru	Left	TRKS	App. Total	Exclu. Total	Inclu. Total	Int. Total
	06:00 AM	3	0	4	0	7	1	50	0	2	51	0	0	0	0	0	0	41	1	8	42	10	100	110
	06:15 AM	3	0	3	0	6	5	55	0	0	60	0	0	0	0	0	0	52	0	2	52	2	118	120
	06:30 AM	1	0	12	0	13	3	84	0	3	87	0	0	0	0	0	0	82	2	1	84	4	184	188
	06:45 AM	6	2	_17	1_	25	6	90	0	3_	96	0	0	0	0	0	0	91	1_	4	92	8	213	221
	Total	13	2	36	1	51	15	279	0	8	294	0	0	0	0	0	0	266	4	15	270	24	615	639
	07:00 AM	9	0	22	0	31	16	70	3	1	89	0	0	0	0	0	0	81	8	6	89	7	209	216
	07:00 AM	11	0	21	0	32	6	75	0	0	81	0	0	0	0	0	0	71	4	2	75	2	188	190
	07:30 AM	5	0	14	0	19	22	89	1	3	112	0	0	0	0	0	0	121	6	1	127	4	258	262
	07:45 AM	5	0	7	0	12	9	62	Ó	3	71	0	0	0	0	0	0	152	6	5	158	8	241	249
	Total	30	0	64	0	94	53	296	4	<u></u>	353	0	0	0	0	0	0	425	24	14	449	21	896	917
	rotar	. 50	U	04	U	54	. 55	250	7	,	555		U	U	O	O		720	27	17	773	21	000	317
	08:00 AM	4	0	8	0	12	20	86	0	5	106	0	0	0	0	0	0	107	8	8	115	13	233	246
	08:15 AM	4	0	9	0	13	27	70	0	2	97	0	0	0	0	0	0	89	8	5	97	7	207	214
	08:30 AM	4	0	9	0	13	27	57	0	2	84	0	0	0	0	0	0	86	16	5	102	7	199	206
	08:45 AM	4	0	_16	1	20	38	62	0	4	100	0	0	0	0	0	0	72	_11_	4	83	9	203	212
	Total	16	0	42	1	58	112	275	0	13	387	0	0	0	0	0	0	354	43	22	397	36	842	878
	1	ı															ı							
	09:00 AM	8	1	70	0	79	50	56	0	9	106	0	0	0	0	0	0	77	11	6	88	15	273	288
	09:15 AM	9	0	34	0	43	17	43	0	1	60	0	0	0	0	0	0	80	12	4	92	5	195	200
	09:30 AM	5	0	13	0	18	13	64	0	4	77	0	0	0	0	0	0	56	3	4	59	8	154	162
	09:45 AM	3	0	4_	0	7	7	45	0_	6_	52	0	0	0	0_	0	0	72	1_	5	73	11	132	143
	Total	25	1	121	0	147	87	208	0	20	295	0	0	0	0	0	0	285	27	19	312	39	754	793
	10:00 AM	4	0	6	1	10	10	40	0	3	50	0	0	0	0	0	0	55	4	4	59	8	119	127
	10:15 AM	4	0	6	0	10	4	54	0	3	58	0	0	0	0	0	0	36	2	4	38	7	106	113
	10:30 AM	7	0	13	0	20	4	48	0	3	52	0	0	0	0	0	0	56	4	1	60	4	132	136
	10:45 AM	6	0	6	0	12	6	45	0	6	51	0	0	0	0	0	0	54	5	2	59	8	122	130
	Total	21	0	31	1	52	24	187	0	15	211	0	0	0	0	0	0	201	15	11	216	27	479	506
	11:00 AM	0	0	9	0	9	9	43	0	3	52	0	0	0	0	0	0	54	6	5	60	8	121	129
	11:15 AM	2	0	7	1	9	7	43	Ō	4	50	0	0	Ō	0	0	0	52	6	3	58	8	117	125
	11:30 AM	5	0	12	1	17	9	45	Ö	2	54	0	Ö	Ö	0	Ő	ő	49	3	2	52	5	123	128
	11:45 AM	4	Ö	10	0	14	10	45	Ö	4	55	Ö	Ö	Ö	Ö	Ö	Ö	42	3	6	45	10	114	124
-	Total	11	0	38	2	49	35	176	0	13	211	0	0	0	0	0	0	197	18	16	215	31	475	506
			J	00							211		J	J	3		, 5		.0			01		
	12:00 PM	3	0	10	2	13	12	52	0	5	64	0	0	0	0	0	0	50	1	3	51	10	128	138
	12:15 PM	6	0	10	1	16	9	65	0	5	74	0	0	0	0	0	0	48	3	2	51	8	141	149
	12:30 PM	3	0	12	0	15	13	50	0	3	63	0	0	0	0	0	0	44	4	3	48	6	126	132
	12:45 PM	3	0	_12_	0	15	11	46	0	3_	57	0	0	0	0	0	0	43	0	3	43	6	115	121_
	Total	15	0	44	3	59	45	213	0	16	258	0	0	0	0	0	0	185	8	11	193	30	510	540



File Name: NC 96 and Pippin Road

Site Code : 00083017 Start Date : 8/30/2017

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round Drinted Core & Trucks

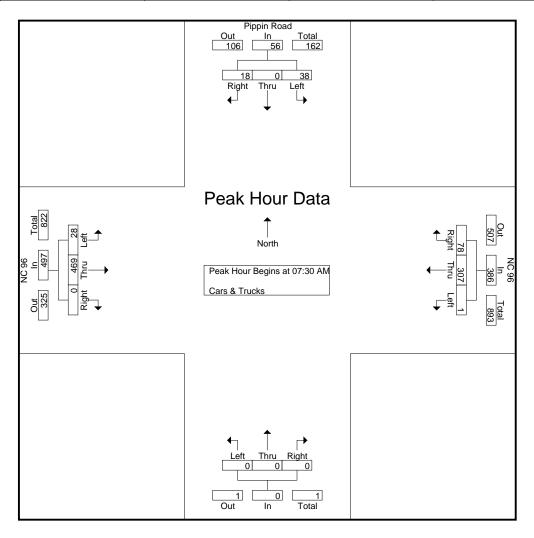
Pippin Road From North From East From South From West From West Start Time Right Time Left Times Right Time Right										Gro	ups P	rinted	l- Cars	s & T	rucks	;								
Start Time Right Throl Left TarkS Right T			Pip	pin F	Road				NC 9	6	·								NC 9	6				
Start Infe Right Park Start			<u>Fr</u>	<u>om N</u>	orth			Fı	rom E	ast			Fro	om So	outh			Fr	om V	/est				
01130 PM	Start Time	Right	Thru		TRKS	App. Total	Right	Thru	Left	TRKS	App. Total	Right	Thru	Left	TRKS	App. Total	Right	Thru	Left	TRKS	App. Total			Int. Total
01:45 PM 3 0 5 1 6 16 73 0 12 89 0 0 0 0 0 0 0 56 3 2 59 15 154 169 01:45 PM 3 0 5 0 8 6 61 0 6 67 0 0 0 0 0 0 0 0 36 5 5 41 11 116 127 Total 12 0 17 2 29 28 255 0 24 283 0 0 0 0 0 0 0 0 194 16 18 210 44 522 566 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	01:00 PM	5	0	2	0	7	3	59	0	4	62	0	0	0	0	0	0	54	3	5	57	9	126	135
Other Part	01:15 PM	3	0	5	1	8	3	62	0	2	65	0	0	0	0	0	0	48	5	6	53	9	126	135
Total 12	01:30 PM	1	0	5	1	6	16	73	0	12	89	0	0	0	0	0	0	56	3	2	59	15	154	169
O2:00 PM 3	01:45 PM	3	0		0	8	6	61	0		67	0	0_	0	0	0	0	36		5		11	116	127_
02:16 PM 6 0 8 0 14 7 63 0 5 70 0 0 0 0 0 0 72 3 6 75 11 159 170 02:30 PM 6 0 6 1 12 5 71 0 3 76 0 0 0 0 0 0 0 0 67 3 8 70 12 158 170 02:30 PM 8 0 8 1 16 12 79 0 2 91 0 0 0 0 0 0 0 67 3 8 70 12 158 170 120:45 PM 8 0 8 1 16 12 79 0 2 91 0 0 0 0 0 0 0 0 89 3 7 92 10 199 209 Total 23 0 33 3 56 33 291 0 16 324 0 0 0 0 0 0 0 0 293 13 25 306 44 686 730 10 159 10	Total	12	0	17	2	29	28	255	0	24	283	0	0	0	0	0	0	194	16	18	210	44	522	566
02:30 PM	02:00 PM	3	0	11	1	14	9	_	0	6	-	0	0	0	0	0	0		4	4		11	170	181
O2:45 PM	02:15 PM	6	0		0						-	0			_	0	_			_		11	159	
Total 23 0 33 3 56 33 291 0 16 324 0 0 0 0 0 0 0 293 13 25 306 44 686 730 03:00 PM 5 0 10 0 15 15 82 0 1 97 0 0 0 0 0 0 0 0 68 7 3 75 4 187 191 03:15 PM 2 0 12 1 14 31 93 0 4 124 0 0 0 0 0 0 0 79 5 6 84 11 222 233 03:30 PM 6 0 25 1 31 20 75 0 1 95 0 0 0 0 0 0 0 79 6 7 85 10 203 213 03:45 PM 6 0 25 1 31 20 75 0 1 95 0 0 0 0 0 0 0 72 6 0 78 5 10 203 213 03:45 PM 6 0 25 1 31 20 75 0 1 95 0 0 0 0 0 0 0 72 6 0 78 5 10 203 213 04:00 PM 12 0 48 1 60 17 90 0 4 107 0 0 0 0 0 0 0 0 97 9 7 106 12 273 285 04:15 PM 4 0 11 0 15 12 114 0 2 126 0 0 0 0 0 0 0 97 9 7 106 12 273 285 04:45 PM 4 0 11 0 15 12 114 0 2 126 0 0 0 0 0 0 0 97 9 7 106 12 273 242 04:30 PM 11 0 18 1 29 9 119 0 4 128 0 0 0 0 0 0 0 81 6 2 87 7 244 251 04:45 PM 8 0 20 0 28 11 123 0 2 134 0 0 0 0 0 0 0 0 81 6 2 87 7 244 251 Total 35 0 97 2 132 49 446 0 12 495 0 0 0 0 0 0 0 0 341 29 17 370 31 997 1028 05:00 PM 4 0 7 1 11 18 125 0 1 143 0 0 0 0 0 0 0 0 0 0 127 9 1 136 4 311 315 Total 20 0 25 1 45 55 344 0 8 399 0 0 0 0 0 0 0 0 0 0 127 9 1 136 4 311 315 Total 20 0 25 1 45 55 344 0 8 399 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	02:30 PM	6	0	6	1		_		0	_	-	0	0	0	0	0	0	_		8	- 1	12	158	170
03:00 PM		_										_			_				_					
03:15 PM	Total	23	0	33	3	56	33	291	0	16	324	0	0	0	0	0	0	293	13	25	306	44	686	730
03:30 PM	03:00 PM		0		0	15				1	-	0	0	0	0	0	0			3				
O3:45 PM 6	03:15 PM	2	0	12	1				0	4		0	0	0	0	0	0	_	5	6	-			
Total 19 0 56 2 75 92 327 0 9 419 0 0 0 0 0 0 298 24 16 322 27 816 843 04:00 PM 12 0 48 1 60 17 90 0 4 107 0 0 0 0 0 0 0 0 97 9 7 106 12 273 285 04:15 PM 4 0 11 0 15 12 114 0 2 126 0 0 0 0 0 0 0 0 90 4 5 94 7 235 242 04:30 PM 11 0 18 1 29 9 119 0 4 128 0 0 0 0 0 0 0 81 6 2 87 7 244 251 04:45 PM 8 0 20 0 28 11 123 0 2 134 0 0 0 0 0 0 0 81 6 2 87 7 244 251 Total 35 0 97 2 132 49 446 0 12 495 0 0 0 0 0 0 0 341 29 17 370 31 997 1028 05:00 PM 4 0 4 0 8 13 94 0 4 107 0 0 0 0 0 0 0 341 29 17 370 31 997 1028 05:30 PM 4 0 7 1 11 18 125 0 1 143 0 0 0 0 0 0 0 0 95 6 1 101 3 255 258 05:45 PM 12 0 14 0 26 24 125 0 3 149 0 0 0 0 0 0 0 0 127 9 1 136 4 311 315 Total 20 0 25 1 45 55 344 0 8 399 0 0 0 0 0 0 0 0 0 0 0 76 8 2 84 7 224 231 06:30 PM 7 0 10 0 17 18 107 0 0 125 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		6	0	_	0	_	_	77	0	3		0	0	0	0	0	0	_		7		10	203	
04:00 PM	03:45 PM	_	0					75				0			0		0	72		0				
04:15 PM	Total	19	0	56	2	75	92	327	0	9	419	0	0	0	0	0	0	298	24	16	322	27	816	843
04:30 PM	04:00 PM	12	0	48				90	0		-	0	0		0	0	0	97	9					
04:45 PM 8 0 20 0 28 11 123 0 2 134 0 0 0 0 0 73 10 3 83 5 245 250 Total 35 0 97 2 132 49 446 0 12 495 0 0 0 0 0 341 29 17 370 31 997 1028 05:00 PM 4 0 4 0 4 107 0 0 0 0 0 0 83 6 2 89 6 204 210 ***********************************	04:15 PM	4	0	11	0	15	12	114	0		-	0	0	0	0	0	0	90	4	5	94	-		
Total 35 0 97 2 132 49 446 0 12 495 0 0 0 0 0 0 341 29 17 370 31 997 1028 05:00 PM 4 0 4 0 8 13 94 0 4 107 0 0 0 0 0 0 0 83 6 2 89 6 204 210 ****BREAK **** 05:30 PM 4 0 7 1 11 18 125 0 1 143 0 0 0 0 0 0 0 95 6 1 101 3 255 258 05:45 PM 12 0 14 0 26 24 125 0 3 149 0 0 0 0 0 0 0 127 9 1 136 4 311 315 Total 20 0 25 1 45 55 344 0 8 399 0 0 0 0 0 0 0 305 21 4 326 13 770 783 06:00 PM 7 0 10 0 17 18 107 0 0 125 0 0 0 0 0 0 0 0 305 21 4 326 13 770 783 06:30 PM 6 0 13 2 19 13 108 0 3 121 0 0 0 0 0 0 0 0 76 8 2 84 7 224 231 06:30 PM 7 0 7 0 14 9 86 0 2 95 0 0 0 0 0 0 0 0 104 4 1 108 3 217 220 ***BREAK *** Total 20 0 3634 20 897 668 3598 4 166 4270 0 0 0 0 0 0 3619 262 194 3881 380 9048 9428 Apprich % 29 0.3 70.7		1	-	_		-			_		-	-	-	-	_	-	_	_	_		-			
05:00 PM		_					_																	
*** BREAK *** 05:30 PM	Total	35	0	97	2	132	49	446	0	12	495	0	0	0	0	0	0	341	29	17	370	31	997	1028
O5:45 PM 12 0 14 0 26 24 125 0 3 149 0 0 0 0 0 127 9 1 136 4 311 315 Total 20 0 25 1 45 55 344 0 8 399 0 0 0 0 0 0 305 21 4 326 13 770 783 06:00 PM 7 0 10 0 17 18 107 0 0 125 0 0 0 0 0 95 8 3 103 3 245 248 06:15 PM 6 0 13 2 19 13 108 0 3 121 0 0 0 0 0 0 76 8 2 84 7 224 231 06:30 PM 7 0 7			0	4	0	8	13	94	0	4	107	0	0	0	0	0	0	83	6	2	89	6	204	210
Total 20 0 25 1 45 55 344 0 8 399 0 0 0 0 0 0 305 21 4 326 13 770 783 06:00 PM 7 0 10 0 17 18 107 0 0 125 0 0 0 0 0 0 95 8 3 103 3 245 248 06:15 PM 6 0 13 2 19 13 108 0 3 121 0 0 0 0 0 0 76 8 2 84 7 224 231 06:30 PM 7 0 7 0 14 9 86 0 2 95 0 0 0 0 0 0 104 4 1 108 3 217 220 **** BREAK *** Total 20 0 30 2 50 40 301 0 5 341 0 0 0 0 0 0 0 275 20 6 295 13 686 699 Grand Total 260 3 634 20 897 668 3598 4 166 4270 0 0 0 0 0 0 3619 262 194 3881 380 9048 9428 Apprich % 29 0.3 70.7	05:30 PM	4	0	7	1	11	18	125	0	1	143	0	0	0	0	0	0	95	6	1	101	3	255	258
06:00 PM	05:45 PM	12	0	14	0	26	24	125	0	3_	149	0	0	0	0	0	0	127	9	1_	136	4	311	315
06:15 PM 6 0 13 2 19 13 108 0 3 121 0 0 0 0 0 0 76 8 2 84 7 224 231 06:30 PM 7 0 7 0 14 9 86 0 2 95 0 0 0 0 0 0 104 4 1 108 3 217 220 *** BREAK *** Total 20 0 30 2 50 40 301 0 5 341 0 0 0 0 0 0 0 0 275 20 6 295 13 686 699 Grand Total 260 3 634 20 897 668 3598 4 166 4270 0 0 0 0 0 0 3619 262 194 3881 380 9048 9428 Apprch % 29 0.3 70.7 15.6 84.3 0.1 0 0 0 0 0 0 0 0 93.2 6.8 380 9048 9428	Total	20	0	25	1	45	55	344	0	8	399	0	0	0	0	0	0	305	21	4	326	13	770	783
06:30 PM 7 0 7 0 14 9 86 0 2 95 0 0 0 0 0 104 4 1 108 3 217 220 ***BREAK *** Total 20 0 30 2 50 40 301 0 5 341 0 0 0 0 0 0 0 275 20 6 295 13 686 699 Grand Total 260 3 634 20 897 668 3598 4 166 4270 0 0 0 0 0 0 0 3619 262 194 3881 380 9048 9428 Apprich % 29 0.3 70.7 15.6 84.3 0.1 0 0 0 0 0 0 0 93.2 6.8	06:00 PM	7	0	10	0	17	18	107	0	0	125	0	0	0	0	0	0	95	8	3	103	3	245	248
*** BREAK *** Total 20 0 30 2 50 40 301 0 5 341 0 0 0 0 0 0 0 275 20 6 295 13 686 699 Grand Total 260 3 634 20 897 668 3598 4 166 4270 0 0 0 0 0 0 3619 262 194 3881 380 9048 9428 Apprich % 29 0.3 70.7 15.6 84.3 0.1 0 0 0 0 0 0 93.2 6.8	06:15 PM	6	0	13	2	19	13		0	3		0	0	0	0	0	0	76	8	2		7		
Total 20 0 30 2 50 40 301 0 5 341 0 0 0 0 0 0 275 20 6 295 13 686 699 Grand Total 260 3 634 20 897 668 3598 4 166 4270 0 0 0 0 0 0 3619 262 194 3881 380 9048 9428 Apprich % 29 0.3 70.7 15.6 84.3 0.1 0 0 0 0 0 0 93.2 6.8			0	7	0	14	9	86	0	2	95	0	0	0	0	0	0	104	4	1	108	3	217	220
Grand Total 260 3 634 20 897 668 3598 4 166 4270 0 0 0 0 0 3619 262 194 3881 380 9048 9428 Apprch % 29 0.3 70.7 15.6 84.3 0.1 0 0 0 0 0 0 93.2 6.8	*** BREAK	(***																						
Apprch % 29 0.3 70.7 15.6 84.3 0.1 0 0 0 0 93.2 6.8	Total	20	0	30	2	50	40	301	0	5	341	0	0	0	0	0	0	275	20	6	295	13	686	699
	Grand Total	260	3	634	20	897	668	3598	4	166	4270	0	0	0	0	0	0	3619	262	194	3881	380	9048	9428
Total % 2.9 0 7 9.9 7.4 39.8 0 47.2 0 0 0 0 0 0 40 2.9 42.9 4 96	Apprch %	29	0.3	70.7			15.6	84.3	0.1			0					0							
	Total %	2.9	0	7		9.9	7.4	39.8	0		47.2	0	0	0		0	0	40	2.9		42.9	4	96	



File Name: NC 96 and Pippin Road

Site Code : 00083017 Start Date : 8/30/2017

		Pippir	n Road			NC	96							NC	96		
		From	North			From	n East			From	South			From	West		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour An	alysis F	rom 06	1A 00:6	Л to 11:4	5 AM -	Peak 1	of 1		-								
Peak Hour for	Entire	Interse	ction B	egins at	07:30 A	λM											
07:30 AM	5	0	14	19	22	89	1	112	0	0	0	0	0	121	6	127	258
07:45 AM	5	0	7	12	9	62	0	71	0	0	0	0	0	152	6	158	241
08:00 AM	4	0	8	12	20	86	0	106	0	0	0	0	0	107	8	115	233
08:15 AM	4	0	9	13	27	70	0	97	0	0	0	0	0	89	8	97	207
Total Volume	18	0	38	56	78	307	1	386	0	0	0	0	0	469	28	497	939
% App. Total	32.1	0	67.9		20.2	79.5	0.3		0	0	0		0	94.4	5.6		
PHF	.900	.000	.679	.737	.722	.862	.250	.862	.000	.000	.000	.000	.000	.771	.875	.786	.910

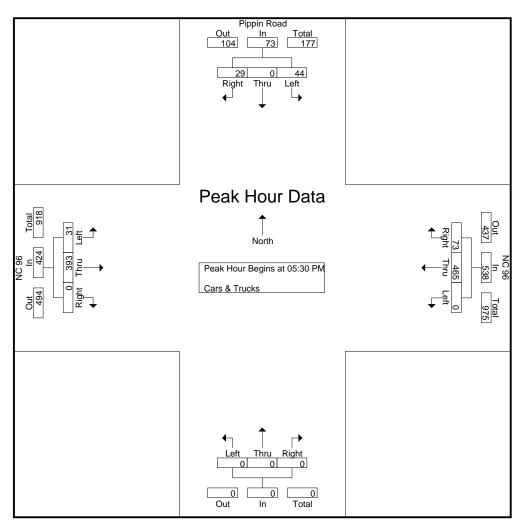




File Name: NC 96 and Pippin Road

Site Code : 00083017 Start Date : 8/30/2017

		Pippir	n Road			NC	96							NC	96		
		From	North			From	n East			From	South			From	West		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour An	alysis F	rom 12	2:00 PN	√ to 06:4	5 PM -	Peak 1	of 1										
Peak Hour for	Entire	Interse	ction B	egins at	05:30 F	PM											
05:30 PM	4	0	7	11	18	125	0	143	0	0	0	0	0	95	6	101	255
05:45 PM	12	0	14	26	24	125	0	149	0	0	0	0	0	127	9	136	311
06:00 PM	7	0	10	17	18	107	0	125	0	0	0	0	0	95	8	103	245
06:15 PM	6	0	13	19	13	108	0	121	0	0	0	0	0	76	8	84	224
Total Volume	29	0	44	73	73	465	0	538	0	0	0	0	0	393	31	424	1035
% App. Total	39.7	0	60.3		13.6	86.4	0		0	0	0		0	92.7	7.3		
PHF	.604	.000	.786	.702	.760	.930	.000	.903	.000	.000	.000	.000	.000	.774	.861	.779	.832



File Name: NC 96 and Glory Road Site Code: 00041019

Site Code : 00041019 Start Date : 4/10/2019

Grouns	Printed-	Cars &	Trucks
Oloubs	I IIIILEU-	Cars 6	IIIUUNO

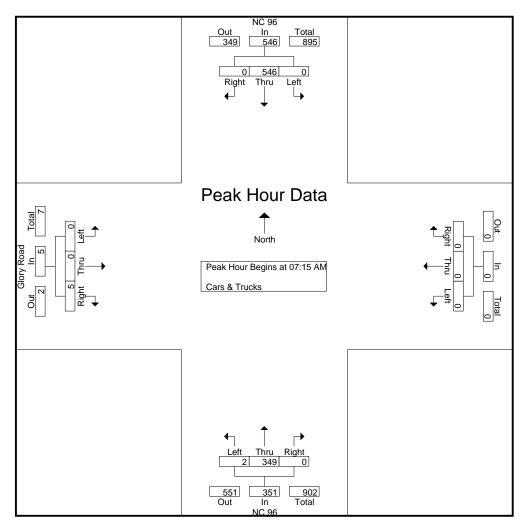
		_	NC 9					_					NC 96					ory Ro					
			om No					om E					om So					om W					
Start Time	Right	Thru	Left	TRKS	App. Total	Right	Thru	Left	TRKS	App. Total	Right	Thru		TRKS	App. Total	Right	Thru	Left	TRKS	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	92	0	3	92	0	0	0	0	0	0	90	2	4	92	0	0	1	0	1	7	185	192
07:15 AM	0	114	0	5	114	0	0	0	0	0	0	107	0	2	107	1	0	0	0	1	7	222	229
07:30 AM	0	153	0	1	153	0	0	0	0	0	0	82	1	3	83	1	0	0	0	1	4	237	241
07:45 AM	0	151	0	6	151	0	0	0	0	0	0	83	0	4	83	2	0	0	0	2	10	236	246
Total	0	510	0	15	510	0	0	0	0	0	0	362	3	13	365	4	0	1	0	5	28	880	908
i																							
08:00 AM	0	128	0	4	128	0	0	0	0	0	0	77	1	1	78	1	0	0	0	1	5	207	212
08:15 AM	0	110	0	7	110	0	0	0	0	0	0	78	1	4	79	2	0	1	0	3	11	192	203
08:30 AM	1	78	0	5	79	0	0	0	0	0	0	85	1	4	86	2	0	0	0	2	9	167	176
*** BREAK	***																						
Total	1	316	0	16	317	0	0	0	0	0	0	240	3	9	243	5	0	1	0	6	25	566	591
*** BREAK	***																						
04:00 PM	0	89	0	1	89	0	0	0	0	0	0	98	0	1	98	2	0	0	0	2	2	189	191
04:15 PM	0	90	0	1	90	0	0	0	0	0	0	98	0	0	98	1	0	0	0	1	1	189	190
04:30 PM	0	95	0	4	95	0	0	0	0	0	0	117	1	1	118	2	0	0	0	2	5	215	220
04:45 PM	0	106	0	5	106	0	0	0	0	0	0	104	1	2	105	2	0	0	0	2	7	213	220
Total	0	380	0	11	380	0	0	0	0	0	0	417	2	4	419	7	0	0	0	7	15	806	821
05:00 PM	0	107	0	4	107	0	0	0	0	0	0	129	3	6	132	1	0	0	0	1	10	240	250
05:15 PM	0	119	0	3	119	0	0	0	0	0	0	100	1	1	101	1	0	0	0	1	4	221	225
05:30 PM	0	140	0	3	140	0	0	0	0	0	0	160	4	5	164	1	0	1	0	2	8	306	314
05:45 PM	0	117	0	2	117	0	0	0	0	0	0	121	4	0	125	0	0	0	0	0	2	242	244
Total	0	483	0	12	483	0	0	0	0	0	0	510	12	12	522	3	0	1	0	4	24	1009	1033
Grand Total	1	1689	0	54	1690	0	0	0	0	0	0	1529	20	38	1549	19	0	3	0	22	92	3261	3353
Apprch %	0.1	99.9	0			0	0	0			0	98.7	1.3			86.4	0	13.6					
Total %	0	51.8	0		51.8	0	0	0		0	0	46.9	0.6		47.5	0.6	0	0.1		0.7	2.7	97.3	



File Name: NC 96 and Glory Road Site Code: 00041019

Site Code : 00041019 Start Date : 4/10/2019

		NC	96							NC	96			Glory	/ Road		
		From	North			Fron	n East			From	South			From	West		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analys					<pre>1 of 1</pre>												
Peak Hour for Ent	ire Interse	ection Be	gins at 0	7:15 AM													
07:15 AM	0	114	0	114	0	0	0	0	0	107	0	107	1	0	0	1	222
07:30 AM	0	153	0	153	0	0	0	0	0	82	1	83	1	0	0	1	237
07:45 AM	0	151	0	151	0	0	0	0	0	83	0	83	2	0	0	2	236
08:00 AM	0	128	0	128	0	0	0	0	0	77	1	78	1	0	0	1	207
Total Volume	0	546	0	546	0	0	0	0	0	349	2	351	5	0	0	5	902
% App. Total	0	100	0		0	0	0		0	99.4	0.6		100	0	0		
PHF	.000	.892	.000	.892	.000	.000	.000	.000	.000	.815	.500	.820	.625	.000	.000	.625	.951

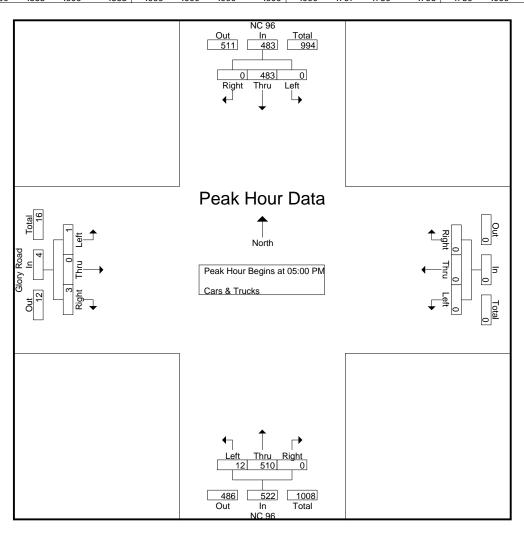




File Name: NC 96 and Glory Road Site Code: 00041019

Site Code : 00041019 Start Date : 4/10/2019

		NC	96							NC	96			Glory	Road		
		From	North			From	East			From	South			From	West		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analys	is From 1	2:00 PM	to 05:45	PM - Peak	(1 of 1												
Peak Hour for Ent	ire Interse	ection Beg	gins at 0	5:00 PM													
05:00 PM	0	107	0	107	0	0	0	0	0	129	3	132	1	0	0	1	240
05:15 PM	0	119	0	119	0	0	0	0	0	100	1	101	1	0	0	1	221
05:30 PM	0	140	0	140	0	0	0	0	0	160	4	164	1	0	1	2	306
05:45 PM	0	117	0	117	0	0	0	0	0	121	4	125	0	0	0	0	242
Total Volume	0	483	0	483	0	0	0	0	0	510	12	522	3	0	1	4	1009
% App. Total	0	100	0		0	0	0		0	97.7	2.3		75	0	25		
PHF	.000	.863	.000	.863	.000	.000	.000	.000	.000	.797	.750	.796	.750	.000	.250	.500	.824



Attachment 1 CZ 2023-02

Burns Service Inc.

1202Langdon Terace Drive Indian Trail, NC, 28079

File Name: Zebulon(Pearces and Pippin) AM Peak

Site Code:

Start Date : 10/3/2017

Page No : 1
Groups Printed- Cars + - Trucks

			es Road bound	t			n Road bound				es Road bound	k			n Road bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
07:00	0	73	5	78	2	3	11	16	0	10	4	14	1	0	0	1	109
07:15	1	62	6	69	1	7	12	20	4	11	3	18	5	4	3	12	119
07:30	1	50	9	60	4	2	8	14	2	19	1	22	6	3	1	10	106
07:45	0	71	5	76	6	6	6	18	4	9	0	13	3	5_	1_	9	116
Total	2	256	25	283	13	18	37	68	10	49	8	67	15	12	5	32	450
								1									
08:00	1	51	10	62	6	9	4	19	1	17	3	21	4	1	2	7	109
08:15	1	48	6	55	1	1	9	11	5	17	1	23	5	2	1	8	97
08:30	4	47	2	53	2	6	8	16	1	16	0	17	6	3	0	9	95
08:45	2	48	4	54	3	6	13	22	3	19	2	24	2	5	2	9	109
Total	8	194	22	224	12	22	34	68	10	69	6	85	17	11	5	33	410
Grand Total	10	450	47	507	25	40	71	136	20	118	14	152	32	23	10	65	860
Apprch %	2	88.8	9.3		18.4	29.4	52.2		13.2	77.6	9.2	.02	49.2	35.4	15.4	00	000
Total %	1.2	52.3	5.5	59	2.9	4.7	8.3	15.8	2.3	13.7	1.6	17.7	3.7	2.7	1.2	7.6	
Cars +	10	448	47	505	25	40	71	136	20	118	14	152	32	23	10	65	858
		-								_		-					
<u> % Cars +</u>	100	99.6	100	99.6	100	100	100	100	100	100	100	100	100	100	100	100	99.8
Trucks	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
% Trucks	0	0.4	0	0.4	0	0	0	0	0	0	0	0	0	0	0	0	0.2

Burns Service Inc.

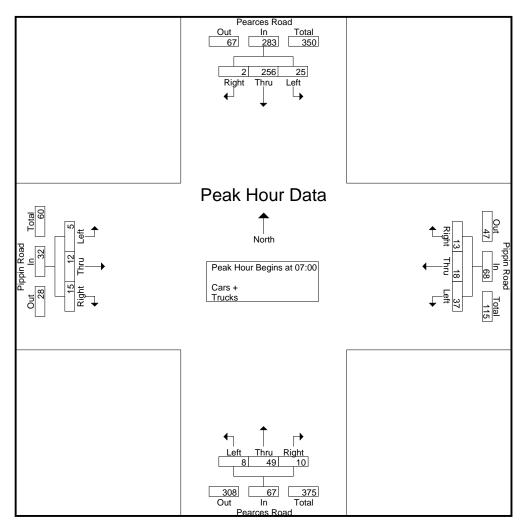
1202Langdon Terace Drive Indian Trail, NC, 28079

File Name: Zebulon(Pearces and Pippin) AM Peak

Site Code:

Start Date : 10/3/2017

		Pearce		-			Road				s Road				n Road		
		South	<u>bound</u>			vvest	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analys	is From 0	7:00 to 0	8:45 - P	eak 1 of 1													
Peak Hour for Ent	ire Interse	ection Be	gins at 0	7:00													
07:00	0	73	5	78	2	3	11	16	0	10	4	14	1	0	0	1	109
07:15	1	62	6	69	1	7	12	20	4	11	3	18	5	4	3	12	119
07:30	1	50	9	60	4	2	8	14	2	19	1	22	6	3	1	10	106
07:45	0	71	5	76	6	6	6	18	4	9	0	13	3	5	1	9	116
Total Volume	2	256	25	283	13	18	37	68	10	49	8	67	15	12	5	32	450
% App. Total	0.7	90.5	8.8		19.1	26.5	54.4		14.9	73.1	11.9		46.9	37.5	15.6		
PHF	.500	.877	.694	.907	.542	.643	.771	.850	.625	.645	.500	.761	.625	.600	.417	.667	.945



Attachment 1 CZ 2023-02

Burns Service Inc.

1202Langdon Terace Drive Indian Trail, NC, 28079

File Name: Zebulon(Pearces and Pippin) PM Peak

Site Code:

Start Date : 10/3/2017

Page No : 1
Groups Printed- Cars + - Trucks

			es Road bound				n Road bound				es Road bound	t			n Road bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
16:00	1	25	3	29	6	4	4	14	10	44	4	58	3	14	2	19	120
16:15	0	28	5	33	4	4	7	15	9	45	5	59	2	6	3	11	118
16:30	1	28	6	35	14	8	3	25	11	63	2	76	4	2	3	9	145
16:45	1	30	6	37	11	3	5	19	15	56	0	71	3	3	2	8	135
Total	3	111	20	134	35	19	19	73	45	208	11	264	12	25	10	47	518
			_			_		1	_		_	1	_	_		_	
17:00	1	31	7	39	12	3	1	16	9	53	3	65	3	5	1	9	129
17:15	1	35	3	39	8	8	6	22	8	49	1	58	0	3	2	5	124
17:30	2	31	2	35	7	4	6	17	12	69	4	85	3	4	4	11	148
17:45	7	45	3	55	11	9	8	28	11	66	3	80	8	4	0	12	175
Total	11	142	15	168	38	24	21	83	40	237	11	288	14	16	7	37	576
Grand Total	14	253	35	302	73	43	40	156	85	445	22	552	26	41	17	84	1094
Apprch %	4.6	83.8	11.6		46.8	27.6	25.6		15.4	80.6	4		31	48.8	20.2		
Total %	1.3	23.1	3.2	27.6	6.7	3.9	3.7	14.3	7.8	40.7	2	50.5	2.4	3.7	1.6	7.7	
Cars +	14	252	34	300	73	43	40	156	85	445	22	552	26	41	17	84	1092
% Cars +	100	99.6	97.1	99.3	100	100	100	100	100	100	100	100	100	100	100	100	99.8
Trucks	0	1	1	2	0	0	0	0	0	0	0	0	0	0	0	0	2
% Trucks	0	0.4	2.9	0.7	0	0	0	0	0	0	0	0	0	0	0	0	0.2

Burns Service Inc.

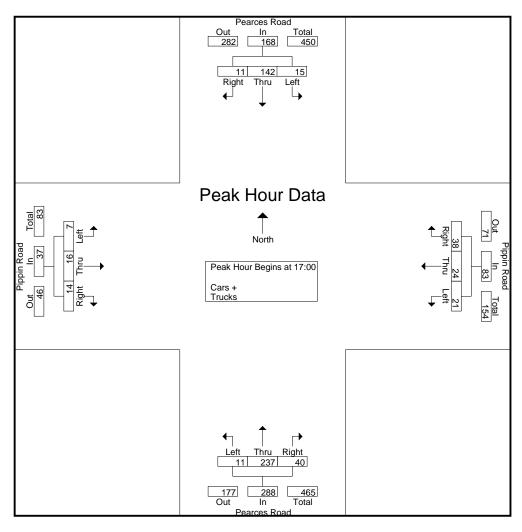
1202Langdon Terace Drive Indian Trail, NC, 28079

File Name: Zebulon(Pearces and Pippin) PM Peak

Site Code:

Start Date : 10/3/2017

	Pearces Road				Pippin Road				Pearces Road				Pippin Road				
	Southbound				Westbound				Northbound				Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 17:00																	
17:00	1	31	7	39	12	3	1	16	9	53	3	65	3	5	1	9	129
17:15	1	35	3	39	8	8	6	22	8	49	1	58	0	3	2	5	124
17:30	2	31	2	35	7	4	6	17	12	69	4	85	3	4	4	11	148
17:45	7	45	3	55	11	9	8	28	11	66	3	80	8	4	0	12	175
Total Volume	11	142	15	168	38	24	21	83	40	237	11	288	14	16	7	37	576
% App. Total	6.5	84.5	8.9		45.8	28.9	25.3		13.9	82.3	3.8		37.8	43.2	18.9		
PHF	.393	.789	.536	.764	.792	.667	.656	.741	.833	.859	.688	.847	.438	.800	.438	.771	.823



TRAFFIC IMPACT ANALYSIS

FOR

WEAVER'S POND UPDATE

LOCATED

IN

ZEBULON, NORTH CAROLINA

Prepared For:
Weavers Pond Development Co., LLC
9407 Bartons Creek Road
Raleigh, NC 27615

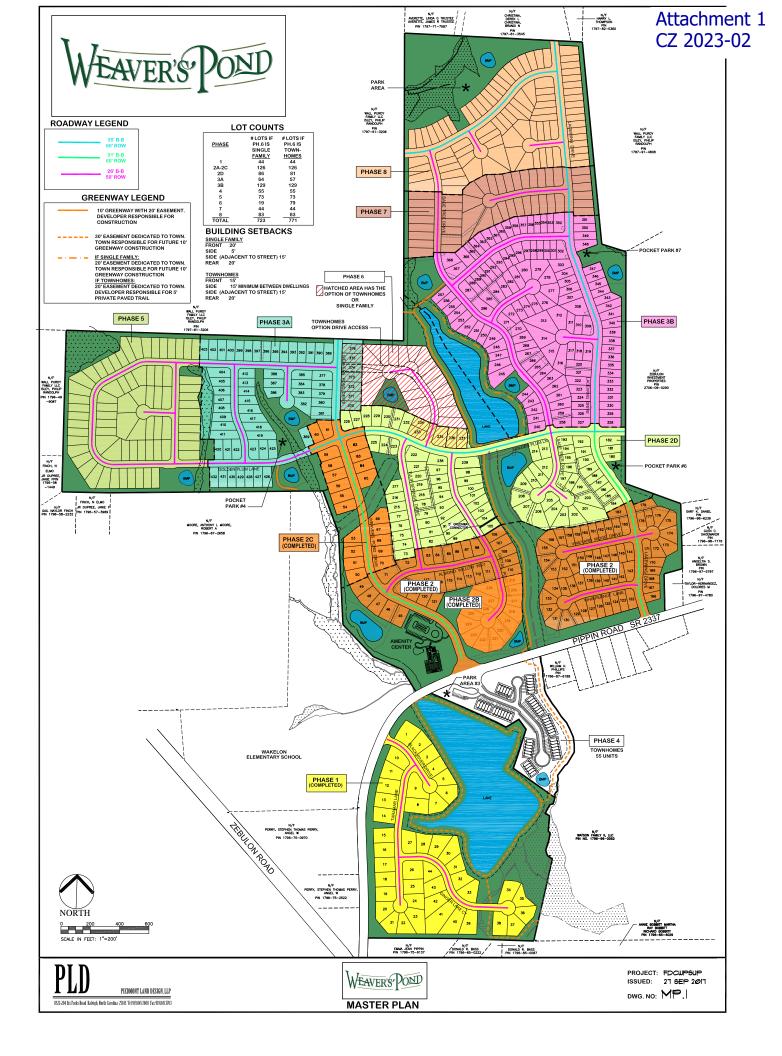
Prepared By:
Ramey Kemp & Associates, Inc.
5808 Faringdon Place, Suite 100
Raleigh, NC 27609
License #C-0910

MARCH 2018



Prepared By: <u>CAB</u>

Reviewed By: JTR



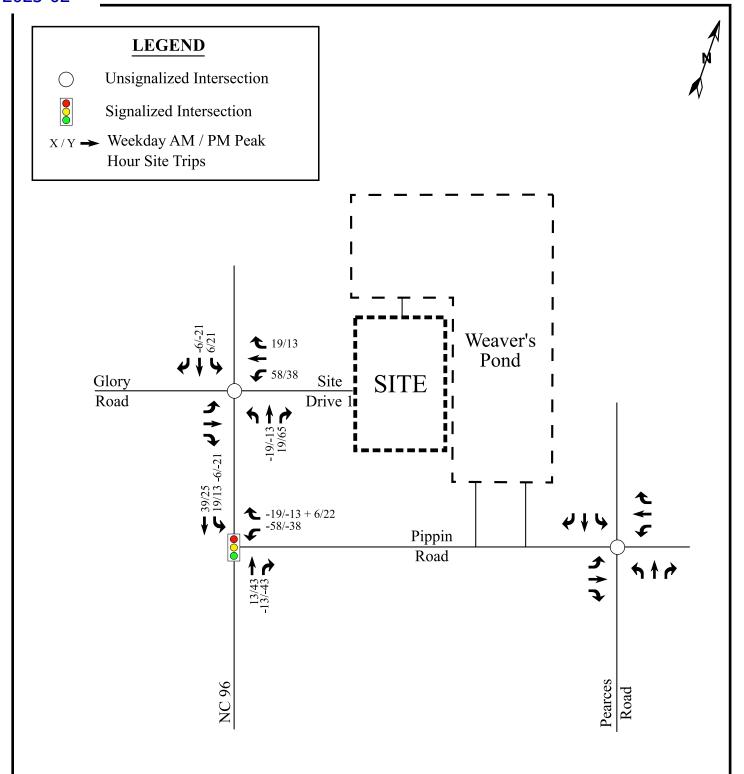
LEGEND Unsignalized Intersection AM / PM Peak Hour Site Trips Roundabout * The existing site drives were not included in the study area for this TIA through coordination with the NCDOT and Town. Site Existing Site Access **1**57/102 Pippin Road



Weaver's Pond Development Zebulon, NC Primary Site
Trip Assignment - Option 1

Scale: Not to Scale

Figure 9A

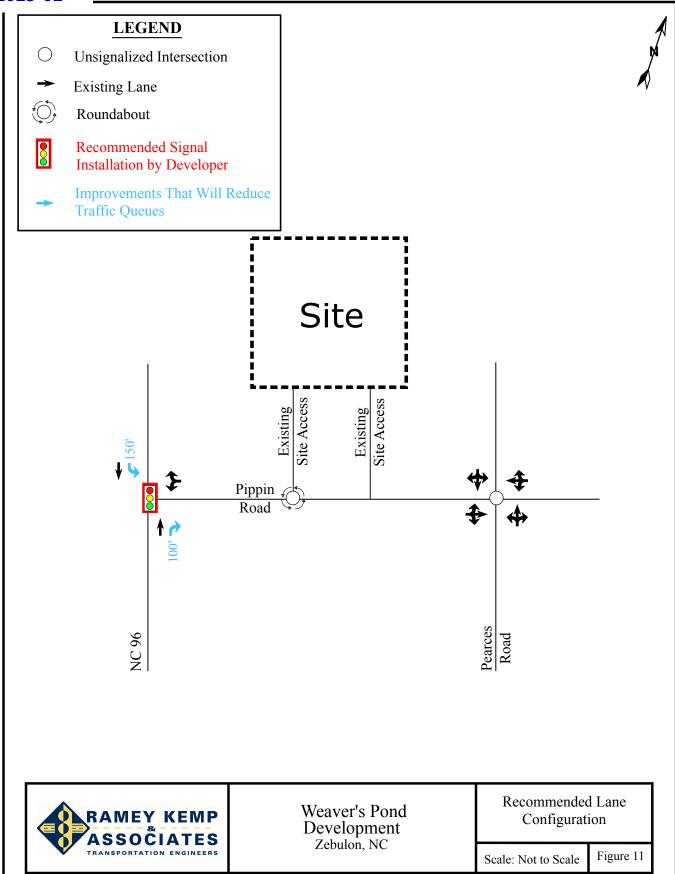




Weaver's Ridge Zebulon, NC Rerouted Weaver's Pond Phases 3A and 5 Site Traffic

Scale: Not to Scale

Figure 10



Traffic Impact Analysis Taryn Lake & Taryn Creek Zebulon, NC

Prepared for:
DR Horton Inc.
2000 Aerial Center Parkway
Suite 110
Morrisville, NC 27560

Prepared by:
Accelerate Engineering, PLLC
July 3, 2017











Overall Development









Land Use (ITE Code)	Intensity	Daily Traffic	AM Pea Trips		PM Peak Hour Trips (vph)		
(III Code)		(vpd)	Enter	Enter	Enter	Exit	
Single-Family Detached Housing (210)	58 units	630	11	35	38	22	
Multifamily Housing (Low-Rise) (Townhomes) (220)	124 units	900	14	45	45	27	
Total Trips	1,530	25	80	83	49		

Table 1: Trip Generation Summary

It is estimated that the proposed development will generate approximately 1,530 total site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 105 trips (25 entering and 80 exiting) will occur during the weekday AM peak hour and 132 trips (83 entering and 49 exiting) will occur during the weekday PM peak hour.

Site Trip Distribution and Assignment

Trip distribution percentages used in assigning site traffic for this development were estimated based on a combination of existing traffic patterns, population centers adjacent to the study area, and engineering judgment. All trip distributions were approved by the Town and NCDOT during the scoping process. It is estimated that trips will be regionally distributed as follows:

- 20% to/from the north via Pearces Road
- 5% to/from the south via Pearces Road
- 50% to/from the south via Zebulon Road (NC 96)
- 25% to/from the north via Zebulon Road (NC 96)

It should be noted that based on engineering judgement it was determined that 100% of site trips will exit the site using the full movement site driveway located along Zebulon Road (NC 96). The site trip distribution is shown in Figure 8. Refer to Figure 9 for the site trip assignment.

It should be noted that based on the layout of the site and the surrounding roadways it was determined that 100% of the new site trips will exit the site using the full movement site driveway to be located along Zebulon Road (NC 96) as the majority (75%) of the Weaver's Pond development was assigned to Zebulon Road (NC 96).

It is not expected that the traffic from the proposed development will utilize the Weaver's Pond access; however, it is expected that a portion of the Weaver's Pond development will utilize the new site drive along Zebulon Road (NC 96). Based on coordination with the NCDOT and Town, Phases 3A and 5 of the Weaver's Pond Master Plan are proposed to be rerouted from the Weaver's Pond access on Pippin Road to the new Zebulon Road (NC 96) site drive aligned with Glory Road. Phases 3A and 5 include 137 homes combined, or



Conclusions

This traffic study was conducted to determine the potential traffic impacts for the proposed Weaver's Ridge residential development located east of Zebulon Road (NC 96) across from Glory Road in Zebulon, North Carolina.

The proposed residential development is assumed to consist of 124 townhomes and 58 single-family homes. The study analyzes traffic conditions during the weekday AM and PM peak hours for the following scenarios:

- Existing (2019) Traffic Conditions
- Background (2023) Traffic Conditions
- Combined (2023) Traffic Conditions
- Combined (2023) Traffic Conditions with Improvements

It is estimated that the proposed development will generate approximately 1,530 total site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 105 trips (25 entering and 80 exiting) will occur during the weekday AM peak hour and 132 trips (83 entering and 49 exiting) will occur during the weekday PM peak hour.

Recommendations

Based on the findings of this study, specific geometric improvements have been identified and are recommended to accommodate future traffic conditions. See a more detailed description of the recommended improvements below. Refer to Figure 12 for an illustration of the recommended lane configuration for the proposed development.

Committed Improvements by Weaver's Pond

Zebulon Road (NC 96) and Pippin Road

- Provide designated northbound right-turn lane with at least 100 feet of storage and appropriate decel and taper.
- Provide designated southbound left-turn lane with at least 150 feet of storage and appropriate decel and taper.
- Install traffic signal at intersection when warranted.

Recommended Improvements by Developer

Zebulon Road (NC 96) and Glory Road / Site Drive 1

- Provide site access via westbound approach with one ingress lane and two egress lanes striped as one left-turn lane and one shared through/right-turn lane.
- Provide designated northbound right-turn lane with at least 100 feet of storage and appropriate decel and taper.
- Provide designated southbound left-turn lane with at least 50 feet of storage and appropriate decel and taper.

• Monitor intersection for signalization.



If you should have any questions, please feel free to contact me at (919) 872-5115.

Sincerely,

Joshua Reinke, P.E. Transportation Engineer

Ramey Kemp & Associates, Inc.

NC Corporate License # C-0910

Attachments: Appendix

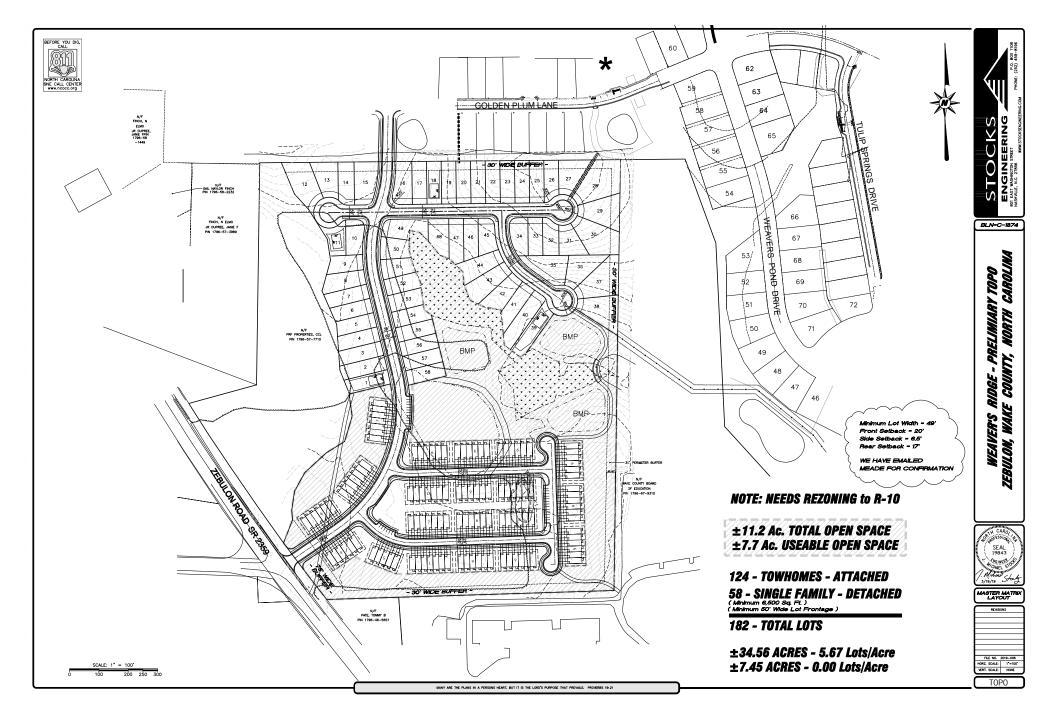


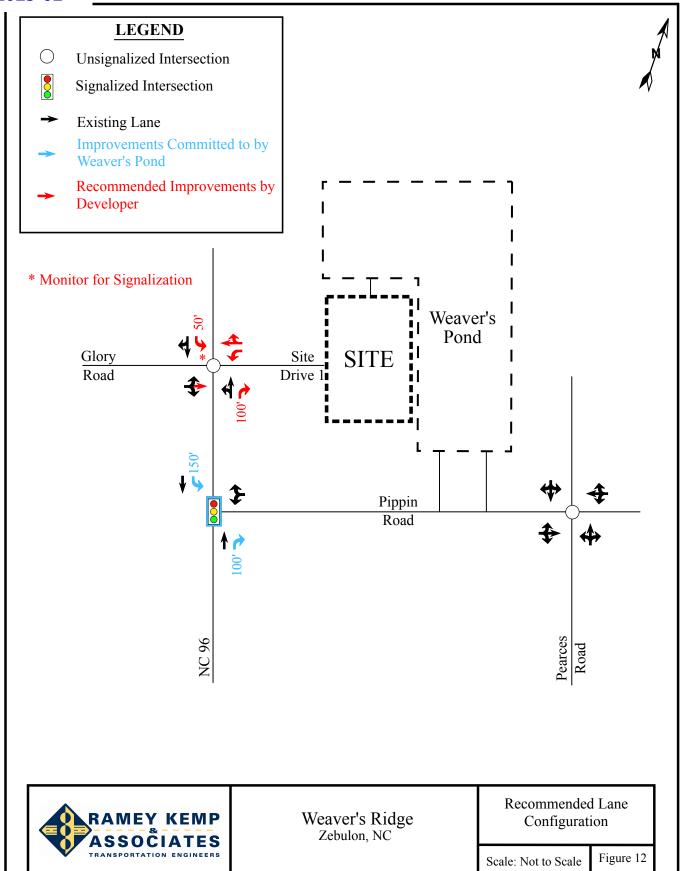


Weaver's Ridge Zebulon, NC

Scale: Not to Scale

Figure 1





	1	•	†	1	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑	7	ሻ	†
Traffic Volume (vph)	180	90	362	135	54	570
Future Volume (vph)	180	90	362	135	54	570
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	.500	100	150	1300
Storage Lanes	1	0		1	1	
Taper Length (ft)	25	U			100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.955	1.00	1.00	0.850	1.00	1.00
Flt Protected	0.968			0.000	0.950	
	1722	0	1863	1583	1770	1863
Satd. Flow (prot)		U	1003	1303		1003
FIt Permitted	0.968	^	4000	4500	0.950	4000
Satd. Flow (perm)	1722	0	1863	1583	1770	1863
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	35		45			45
Link Distance (ft)	1250		1120			2533
Travel Time (s)	24.4		17.0			38.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	200	100	402	150	60	633
Shared Lane Traffic (%)						
Lane Group Flow (vph)	300	0	402	150	60	633
Turn Type	Prot	-	NA	pm+ov	Prot	NA
Protected Phases	4		2	4	1	6
Permitted Phases				2		U
Detector Phase	4		2	4	1	6
Switch Phase	4		Z	4		U
	7.0		10.0	7.0	7.0	10.0
Minimum Initial (s)	7.0		12.0	7.0	7.0	12.0
Minimum Split (s)	14.0		19.0	14.0	14.0	19.0
Total Split (s)	45.0		59.0	45.0	16.0	75.0
Total Split (%)	37.5%		49.2%	37.5%	13.3%	62.5%
Maximum Green (s)	38.0		52.0	38.0	9.0	68.0
Yellow Time (s)	5.0		5.0	5.0	5.0	5.0
All-Red Time (s)	2.0		2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0		-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0
Lead/Lag			Lag		Lead	
Lead-Lag Optimize?			Yes		Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Min		Min	Min	None	Min
Act Effct Green (s)	20.0		25.9	54.2	10.6	33.8
Actuated g/C Ratio	0.31		0.40	0.84	0.16	0.52
G	0.56		0.40	0.04	0.10	0.52
v/c Ratio						
Control Delay	25.9		21.1	2.9	32.2	14.9
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	25.9		21.1	2.9	32.2	14.9
LOS	С		С	Α	С	В
Approach Delay	25.9		16.1			16.4
Approach LOS	С		В			В

	•	•	†	<i>></i>	/	↓	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Queue Length 50th (ft)	106		137	16	22	157	
Queue Length 95th (ft)	218		267	31	69	328	
Internal Link Dist (ft)	1170		1040			2453	
Turn Bay Length (ft)				100	150		
Base Capacity (vph)	1156		1533	1530	327	1752	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.26		0.26	0.10	0.18	0.36	
Intersection Summary							
Area Type:	Other						
Cycle Length: 120							
Actuated Cycle Length: 64	4.6						
Natural Cycle: 60							
Control Type: Actuated-U	ncoordinated						
Maximum v/c Ratio: 0.65							
Intersection Signal Delay:					tersection		
Intersection Capacity Utiliz	zation 53.8%			IC	U Level o	of Service	A
Analysis Period (min) 15							
Splits and Phases: 1: Z	ebulon Road	(NIC 06) 3	R. Dinnin E	2nad			
Spins and Fhases. 1. Z	EDUIUII MUAU	(140 30) (x r-ippin r	wau			I
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WBL	WBR	NBT	NBR	SBL	SBT
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	95				706
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	1.00	1.00	1.00		1.00
	1.00	1.00		1.00	1.00
			0.050	0.050	
	۸	1062	1502		1062
	U	1003	1003		1863
	^	4000	4500		4000
1/13		1863		1//0	1863
	No		No		
35		45			45
					2533
					38.4
	0.00		0.00	0.00	0.90
					784
104	100	493	154	104	7 04
070		400	454	404	704
	U				784
			•		NA
4		2		1	6
4		2	4	1	6
			7.0	7.0	12.0
14.0		19.0	14.0	14.0	19.0
39.0		61.0	39.0	20.0	81.0
32.5%		50.8%	32.5%	16.7%	67.5%
32.0		54.0	32.0	13.0	74.0
					5.0
					2.0
					-2.0
					5.0
5.0			5.0		5.0
2.0			2.0		2.0
					3.0
					Min
					44.5
					0.59
					0.72
					15.8
0.0		0.0	0.0	0.0	0.0
32.6		24.9	3.8	38.0	15.8
С		С	Α	D	В
32.6		19.9			18.4
		В			В
	39.0 32.5% 32.0 5.0 2.0 -2.0 5.0 3.0 Min 20.5 0.27 0.58 32.6 0.0 32.6 C	148 95 148 95 1900 1900 0 0 1 0 25 1.00 1.00 0.947 0.971 1713 0 0.971 1713 0 No 35 1250 24.4 0.90 0.90 164 106 270 0 Prot 4 4 7.0 14.0 39.0 32.5% 32.0 5.0 2.0 -2.0 5.0 3.0 Min 20.5 0.27 0.58 32.6 0.0 32.6 C 32.6 C 32.6	148 95 444 148 95 444 1900 1900 1900 0 0 1 0 25 1.00 1.00 1.00 0.947 0.971 1713 0 1863 0.971 1713 0 1863 No 35 45 1250 1120 24.4 17.0 0.90 0.90 0.90 164 106 493 270 0 493 Prot NA 4 2 4 2 7.0 12.0 14.0 19.0 39.0 61.0 32.5% 50.8% 32.0 54.0 5.0 5.0 2.0 2.0 -2.0 -2.0 5.0 5.0 Lag Yes 3.0 3.0 Min Min 20.5 31.2 0.27 0.41 0.58 0.64 32.6 24.9 0.0 0.0 32.6 24.9 0.0 0.0 32.6 24.9 0.0 0.0 32.6 24.9 0.0 0.0 32.6 24.9 0.0 0.0 32.6 24.9 0.0 0.0 32.6 24.9	148 95 444 139 148 95 444 139 1900 1900 1900 1900 0 0 100 1 0 1 25 1.00 1.00 1.00 1.00 1.00 0.947 0.850 0.971 1713 0 1863 1583 0.971 1713 0 1863 1583 0.971 1713 0 1863 1583 No No 35 45 1250 1120 24.4 17.0 0.90 0.90 0.90 0.90 164 106 493 154 Prot NA pm+ov 4 2 4 2 4 7.0 12.0 7.0 14.0 19.0 14.0 39.0 61.0 39.0 32.5% 50.8% 32.5% 32.0 54.0 32.0 5.0 5.0 5.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 1.0 5.0 5.0	148 95 444 139 94 148 95 444 139 94 1900 1900 1900 1900 1900 0 0 100 150 1 0 1 1 1 25 100 1.00 1.00 1.00 0.947 0.850 0.950 1.00 1.00 0.947 0.850 0.950 0.950 1.00 1713 0 1863 1583 1770 0.971 0.950 0.950 1.70 0.950 1713 0 1863 1583 1770 0.971 0.950 1.90 0.90 0.950 1713 0 1863 1583 1770 0.971 1.20 0.90 0.90 0.90 140 1.20 0.90 0.90 0.90 0.90 140 1.20 7.0 7.0 1.0 1.0

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Queue Length 50th (ft)	111		191	19	44	224	
Queue Length 95th (ft)	238		365	38	119	463	
Internal Link Dist (ft)	1170		1040			2453	
Turn Bay Length (ft)				100	150		
Base Capacity (vph)	832		1404	1420	379	1707	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.32		0.35	0.11	0.27	0.46	
Intersection Summary							
Area Type:	Other						
Cycle Length: 120							
Actuated Cycle Length: 75.	9						
Natural Cycle: 60							
Control Type: Actuated-Und	coordinated						
Maximum v/c Ratio: 0.72							
Intersection Signal Delay: 2					tersection		
Intersection Capacity Utiliza	ation 59.5%			IC	U Level c	of Service B	
Analysis Period (min) 15							
Splits and Phases: 1: Ze	bulon Road	(NC 96) 8	& Pippin F	Road			
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81 s							

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		†	7	*	↑
Traffic Volume (vph)	170	95	446	147	94	712
Future Volume (vph)	170	95	446	147	94	712
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	1300	100	150	1300
	1			100	130	
Storage Lanes		0		I		
Taper Length (ft)	25	4.00	4.00	4.00	100	4.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.951			0.850		
FIt Protected	0.969				0.950	
Satd. Flow (prot)	1717	0	1863	1583	1770	1863
Flt Permitted	0.969				0.950	
Satd. Flow (perm)	1717	0	1863	1583	1770	1863
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	35		45			45
Link Opeed (mph) Link Distance (ft)	1250		1120			2533
Travel Time (s)	24.4		17.0			38.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	189	106	496	163	104	791
Shared Lane Traffic (%)	00=		100	400	40.4	=0.4
Lane Group Flow (vph)	295	0	496	163	104	791
Turn Type	Prot		NA	pm+ov	Prot	NA
Protected Phases	4		2	4	1	6
Permitted Phases				2		
Detector Phase	4		2	4	1	6
Switch Phase						
Minimum Initial (s)	7.0		12.0	7.0	7.0	12.0
Minimum Split (s)	14.0		19.0	14.0	14.0	19.0
Total Split (s)	40.0		61.0	40.0	19.0	80.0
Total Split (%)	33.3%		50.8%	33.3%	15.8%	66.7%
Maximum Green (s)	33.0		54.0	33.0	12.0	73.0
Yellow Time (s)	5.0		5.0	5.0	5.0	5.0
All-Red Time (s)	2.0		2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0		-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0
Lead/Lag			Lag		Lead	
Lead-Lag Optimize?			Yes		Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Min		Min	Min	None	Min
Act Effct Green (s)	21.9		32.2	61.2	12.4	45.3
Actuated g/C Ratio	0.28		0.41	0.78	0.16	0.58
v/c Ratio	0.61		0.65	0.13	0.37	0.73
Control Delay	33.2		25.5	3.6	40.2	17.0
Queue Delay	0.0		0.0	0.0	0.0	0.0
•	33.2		25.5	3.6	40.2	17.0
Total Delay						
LOS	C		C	Α	D	B
Approach Delay	33.2		20.0			19.7
Approach LOS	С		С			В

Lane Group WBL WBR NBT NBR SBL SBT Queue Length 50th (ft) 125 199 21 45 240
Queue Length 50th (ft) 125 100 21 45 240
Queue Length John (It) 125 133 21 45 240
Queue Length 95th (ft) 264 377 38 125 503
Internal Link Dist (ft) 1170 1040 2453
Turn Bay Length (ft) 100 150
Base Capacity (vph) 835 1376 1419 344 1671
Starvation Cap Reductn 0 0 0 0
Spillback Cap Reductn 0 0 0 0
Storage Cap Reductn 0 0 0 0
Reduced v/c Ratio 0.35 0.36 0.11 0.30 0.47
Intersection Summary
Area Type: Other
Cycle Length: 120
Actuated Cycle Length: 78.1
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.73
Intersection Signal Delay: 22.0 Intersection LOS: C
Intersection Capacity Utilization 61.0% ICU Level of Service B
Analysis Period (min) 15
Splits and Phases: 1: Zebulon Road (NC 96) & Pippin Road
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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		†	7	7	↑
Traffic Volume (vph)	140	78	539	237	112	495
Future Volume (vph)	140	78	539	237	112	495
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	1500	100	150	1000
Storage Lanes	1	0		100	130	
Taper Length (ft)	25	U		ı	100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.952	1.00	1.00	0.850	1.00	1.00
				0.000	0.050	
Flt Protected	0.969		4000	4500	0.950	4000
Satd. Flow (prot)	1718	0	1863	1583	1770	1863
FIt Permitted	0.969				0.950	
Satd. Flow (perm)	1718	0	1863	1583	1770	1863
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	35		45			45
Link Distance (ft)	1250		1120			2533
Travel Time (s)	24.4		17.0			38.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	156	87	599	263	124	550
Shared Lane Traffic (%)		31		200		
Lane Group Flow (vph)	243	0	599	263	124	550
Turn Type	Prot	U	NA	pm+ov	Prot	NA
Protected Phases	4		2	•	1	6
	4		Z	4		0
Permitted Phases	4			2	1	
Detector Phase	4		2	4	1	6
Switch Phase			40.0			40.0
Minimum Initial (s)	7.0		12.0	7.0	7.0	12.0
Minimum Split (s)	14.0		19.0	14.0	14.0	19.0
Total Split (s)	34.0		65.0	34.0	21.0	86.0
Total Split (%)	28.3%		54.2%	28.3%	17.5%	71.7%
Maximum Green (s)	27.0		58.0	27.0	14.0	79.0
Yellow Time (s)	5.0		5.0	5.0	5.0	5.0
All-Red Time (s)	2.0		2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0		-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0
Lead/Lag	0.0		Lead	0.0	Lag	0.0
Lead-Lag Optimize?			Yes		Yes	
• .	3.0		3.0	3.0	3.0	3.0
Vehicle Extension (s) Recall Mode						
	Min		Min	Min	None	Min
Act Effet Green (s)	19.8		36.0	61.1	13.3	54.6
Actuated g/C Ratio	0.23		0.42	0.72	0.16	0.64
v/c Ratio	0.61		0.76	0.23	0.45	0.46
Control Delay	38.8		28.5	4.4	43.3	9.4
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	38.8		28.5	4.4	43.3	9.4
LOS	D		С	Α	D	Α
Approach Delay	38.8		21.1			15.7
Approach LOS	D		С			В

	•	•	†	~	1	↓	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Queue Length 50th (ft)	115		259	39	60	129	
Queue Length 95th (ft)	242		463	67	148	250	
Internal Link Dist (ft)	1170		1040			2453	
Turn Bay Length (ft)				100	150		
Base Capacity (vph)	621		1376	1137	353	1673	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.39		0.44	0.23	0.35	0.33	
Intersection Summary							
Area Type:	Other						
Cycle Length: 120							
Actuated Cycle Length: 85	5.1						
Natural Cycle: 60							
Control Type: Actuated-U	ncoordinated						
Maximum v/c Ratio: 0.76							
Intersection Signal Delay:					tersection		
Intersection Capacity Utiliz	zation 59.6%			IC	U Level c	of Service B	
Analysis Period (min) 15							
Splits and Phases: 1: Z	ebulon Road	(NC 96) 8	R Pinnin F	Road			
♣		(. 10 00)	<u> </u>	1000	-	Λ.	
Ø2						Ø1	€ €Ø4
65 s						21 s	34 s
▼ Ø6							
86 s							

Lane Group WBL WBR NBT NBR SBL SBT Lane Configurations ↑
Lane Configurations Y Image: Configuration of the part of
Traffic Volume (vph) 124 118 688 226 130 613 Future Volume (vph) 124 118 688 226 130 613 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 Storage Length (ft) 0 0 100 150 Storage Lanes 1 0 1 1 Taper Length (ft) 25 100 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 Lane Util. Factor 0.934 0.850 0.850 0.850 Fit Protected 0.975 0.950 0.950 0.950 0.950 Satd. Flow (prot) 1696 0 1863 1583 1770 1863 Right Turn on Red No No No No No Satd. Flow (pro
Future Volume (vph) 124 118 688 226 130 613 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Storage Length (ft) 0 0 100 150 Storage Lanes 1 0 1 1 Taper Length (ft) 25 100 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 Fit 0.934 0.850 0.950
Ideal Flow (vphpl) 1900 1800
Storage Length (ft) 0 0 100 150 Storage Lanes 1 0 1 1 Taper Length (ft) 25 100 1.00
Storage Lanes 1 0 1 1 Taper Length (ft) 25 100 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 Frt 0.934 0.850 0.850 0.960 0.960 0.960 0.960 0.960 0.960 0.960 0.90
Taper Length (ft) 25 100 Lane Util. Factor 1.00 1
Lane Util. Factor 1.00 2.00
Frt 0.934 0.850 Flt Protected 0.975 0.950 Satd. Flow (prot) 1696 0 1863 1583 1770 1863 Flt Permitted 0.975 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.963 0.963 0.963 0.963 0.963 0.963 0.963 0.963 0.964 0.964 0.90
Fit Protected 0.975 0.950 Satd. Flow (prot) 1696 0 1863 1583 1770 1863 Fit Permitted 0.975 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.963 1863 1583 1770 1863 <td< td=""></td<>
Satd. Flow (prot) 1696 0 1863 1583 1770 1863 Flt Permitted 0.975 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.963 0.963 0.963 0.963 0.963 0.963 0.963 0.964 0.90
Fit Permitted 0.975 0.950 Satd. Flow (perm) 1696 0 1863 1583 1770 1863 Right Turn on Red No No No Satd. Flow (RTOR) No No Satd. Flow (RTOR) Satd. Flow (RTOR) <td< td=""></td<>
Satd. Flow (perm) 1696 0 1863 1583 1770 1863 Right Turn on Red No No No No Satd. Flow (RTOR) No Satd. Flow (RTOR) Satd. Flow (RTOR) Satd. Flow (PTOR) Flow (PTOR) Satd. Flow (PTOR)
Right Turn on Red No No Satd. Flow (RTOR) Link Speed (mph) 35 45 45 Link Distance (ft) 1250 1120 2533 Travel Time (s) 24.4 17.0 38.4 Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 138 131 764 251 144 681 Shared Lane Traffic (%) Lane Group Flow (vph) 269 0 764 251 144 681 Turn Type Prot NA pm+ov Prot NA Protected Phases 4 2 4 1 6
Satd. Flow (RTOR) Link Speed (mph) 35 45 45 Link Distance (ft) 1250 1120 2533 Travel Time (s) 24.4 17.0 38.4 Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 138 131 764 251 144 681 Shared Lane Traffic (%) Lane Group Flow (vph) 269 0 764 251 144 681 Turn Type Prot NA pm+ov Prot NA Protected Phases 4 2 4 1 6
Link Speed (mph) 35 45 45 Link Distance (ft) 1250 1120 2533 Travel Time (s) 24.4 17.0 38.4 Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 138 131 764 251 144 681 Shared Lane Traffic (%) Lane Group Flow (vph) 269 0 764 251 144 681 Turn Type Prot NA pm+ov Prot NA Protected Phases 4 2 4 1 6
Link Distance (ft) 1250 1120 2533 Travel Time (s) 24.4 17.0 38.4 Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 138 131 764 251 144 681 Shared Lane Traffic (%) Lane Group Flow (vph) 269 0 764 251 144 681 Turn Type Prot NA pm+ov Prot NA Protected Phases 4 2 4 1 6
Link Distance (ft) 1250 1120 2533 Travel Time (s) 24.4 17.0 38.4 Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 138 131 764 251 144 681 Shared Lane Traffic (%) Lane Group Flow (vph) 269 0 764 251 144 681 Turn Type Prot NA pm+ov Prot NA Protected Phases 4 2 4 1 6
Travel Time (s) 24.4 17.0 38.4 Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 138 131 764 251 144 681 Shared Lane Traffic (%) Lane Group Flow (vph) 269 0 764 251 144 681 Turn Type Prot NA pm+ov Prot NA Protected Phases 4 2 4 1 6
Peak Hour Factor 0.90
Adj. Flow (vph) 138 131 764 251 144 681 Shared Lane Traffic (%) Lane Group Flow (vph) 269 0 764 251 144 681 Turn Type Prot NA pm+ov Prot NA Protected Phases 4 2 4 1 6
Shared Lane Traffic (%) Lane Group Flow (vph) 269 0 764 251 144 681 Turn Type Prot NA pm+ov Prot NA Protected Phases 4 2 4 1 6
Lane Group Flow (vph) 269 0 764 251 144 681 Turn Type Prot NA pm+ov Prot NA Protected Phases 4 2 4 1 6
Turn TypeProtNA pm+ovProtNAProtected Phases42416
Protected Phases 4 2 4 1 6
remilled filases Z
Detector Phase
Detector Phase 4 2 4 1 6
Switch Phase
Minimum Initial (s) 7.0 12.0 7.0 12.0
Minimum Split (s) 14.0 19.0 14.0 19.0
Total Split (s) 32.0 68.0 32.0 20.0 88.0
Total Split (%) 26.7% 56.7% 26.7% 16.7% 73.3%
Maximum Green (s) 25.0 61.0 25.0 13.0 81.0
Yellow Time (s) 5.0 5.0 5.0 5.0
All-Red Time (s) 2.0 2.0 2.0 2.0
Lost Time Adjust (s) -2.0 -2.0 -2.0 -2.0
Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0
Lead/Lag Lead Lag
Lead-Lag Optimize? Yes Yes
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0
Recall Mode Min Min Min None Min
Act Effect Green (s) 22.1 48.5 75.8 13.8 67.6
Actuated g/C Ratio 0.22 0.48 0.76 0.14 0.68
v/c Ratio 0.72 0.85 0.21 0.59 0.54
Control Delay 50.4 32.9 3.8 55.6 10.4
Queue Delay 0.0 0.0 0.0 0.0 0.0
Total Delay 50.4 32.9 3.8 55.6 10.4
LOS D C A E B
Approach Delay 50.4 25.7 18.3
Approach LOS D C B

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Queue Length 50th (ft)	168		431	40	92	208	
Queue Length 95th (ft)	291		633	61	#188	318	
Internal Link Dist (ft)	1170		1040			2453	
Turn Bay Length (ft)				100	150		
Base Capacity (vph)	477		1223	1193	276	1536	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.56		0.62	0.21	0.52	0.44	
Intersection Summary							
Area Type:	Other						
Cycle Length: 120							
Actuated Cycle Length: 100).1						
Natural Cycle: 70							
Control Type: Actuated-Und	coordinated						
Maximum v/c Ratio: 0.85							
Intersection Signal Delay: 2					tersection		
Intersection Capacity Utiliza	ation 70.0%			IC	CU Level of	of Service C)
Analysis Period (min) 15							
# 95th percentile volume			eue may	be longer	·.		
Queue shown is maximu	ım after two	cycles.					
Splits and Phases: 1: Zet	bulon Road	(NC 96) a	& Pippin F	Road			
↑ _{Ø2}						Ø	1
68 s						20 s	32 s
↓ Ø6							

	•	•	†	1	-	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑	7	*	↑
Traffic Volume (vph)	138	118	695	252	130	617
Future Volume (vph)	138	118	695	252	130	617
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	1300	100	150	1000
Storage Lanes	1	0		100	130	
•	25	U		ı	100	
Taper Length (ft)		1.00	1.00	1.00		1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.938			0.850	0.050	
Flt Protected	0.974				0.950	
Satd. Flow (prot)	1702	0	1863	1583	1770	1863
Flt Permitted	0.974				0.950	
Satd. Flow (perm)	1702	0	1863	1583	1770	1863
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	35		45			45
Link Distance (ft)	1250		1120			2533
Travel Time (s)	24.4		17.0			38.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	153	131	772	280	144	686
Shared Lane Traffic (%)	100	101	112	200	177	000
` '	284	0	772	280	144	686
Lane Group Flow (vph)		U				NA
Turn Type	Prot		NA 2	pm+ov	Prot	
Protected Phases	4		2	4	1	6
Permitted Phases				2		
Detector Phase	4		2	4	1	6
Switch Phase						
Minimum Initial (s)	7.0		12.0	7.0	7.0	12.0
Minimum Split (s)	14.0		19.0	14.0	14.0	19.0
Total Split (s)	32.0		68.0	32.0	20.0	88.0
Total Split (%)	26.7%		56.7%	26.7%	16.7%	73.3%
Maximum Green (s)	25.0		61.0	25.0	13.0	81.0
Yellow Time (s)	5.0		5.0	5.0	5.0	5.0
All-Red Time (s)	2.0		2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0		-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0
Lead/Lag	5.0			5.0		5.0
			Lead		Lag	
Lead-Lag Optimize?	0.0		Yes	2.0	Yes	2.0
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Min		Min	Min	None	Min
Act Effct Green (s)	22.9		49.5	77.6	13.8	68.5
Actuated g/C Ratio	0.22		0.49	0.76	0.14	0.67
v/c Ratio	0.74		0.85	0.23	0.60	0.55
Control Delay	52.0		33.7	3.9	56.9	10.7
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	52.0		33.7	3.9	56.9	10.7
LOS	D		С	Α	E	В
Approach Delay	52.0		25.7	, ,	_	18.7
Approach LOS	52.0 D		23.7 C			В
Apploacii LOS	U		U			D

	-	•	†	1	-	Ţ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Queue Length 50th (ft)	182		453	45	95	222	
Queue Length 95th (ft)	#323		644	68	#188	323	
Internal Link Dist (ft)	1170		1040			2453	
Turn Bay Length (ft)				100	150		
Base Capacity (vph)	468		1198	1196	270	1514	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.61		0.64	0.23	0.53	0.45	
Intersection Summary							
Area Type:	Other						
Cycle Length: 120							
Actuated Cycle Length: 10	1.8						
Natural Cycle: 70							
Control Type: Actuated-Un	coordinated						
Maximum v/c Ratio: 0.85							
Intersection Signal Delay: 2					tersection		
Intersection Capacity Utiliza	ation 71.2%			IC	CU Level of	of Service C	
Analysis Period (min) 15							
# 95th percentile volume			eue may	be longer			
Queue shown is maxim	um after two	cycles.					
Splits and Phases: 1: Ze	bulon Road	(NC 96) a	& Pippin F	Road			
↑ ø2						Ø	ı € Ø4
68 s						20 s	32 s
₩ Ø6							

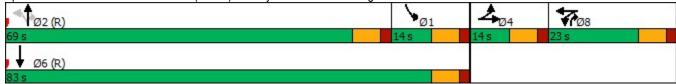
Intersection						
Int Delay, s/veh	0.2					
		EDD	ND	NDT	OPT	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	M	_		4	4	
Traffic Vol, veh/h	4	5	4	450	619	4
Future Vol, veh/h	4	5	4	450	619	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	6	4	500	688	4
Major/Minor	Minor		Major1	A	/oicr2	
	Minor2		Major1		/lajor2	
Conflicting Flow All	1198	690	692	0	-	0
Stage 1	690	-	-	-	-	-
Stage 2	508	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	205	445	903	-	-	-
Stage 1	498	-	-	-	-	-
Stage 2	604	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	204	445	903	-	-	-
Mov Cap-2 Maneuver	204	-	-	-	-	-
Stage 1	495	_	_	-	_	-
Stage 2	604	_	_	_	_	_
Clayo Z	JU-7					
Approach	EB		NB		SB	
HCM Control Delay, s	17.8		0.1		0	
HCM LOS	С					
Minor Lane/Major Mvn	nt	NBL	NRT	EBLn1	SBT	SBR
	IL.	903	NDI	292	ומט	אומט
Capacity (veh/h)				0.034	-	-
HCM Lane V/C Ratio		0.005	0	17.8	-	-
HCM Control Dolou (a)				1 / A	-	-
HCM Control Delay (s)						
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh		A 0	A -	C 0.1	-	-

Intersection														
Int Delay, s/veh	15.8													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		4		*	1			ર્ન	7	*	1			
Traffic Vol, veh/h	4	4	6	118	4	39	4	500	38	12	679	4		
Future Vol., veh/h	4	4	6	118	4	39	4	500	38	12	679	4		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0		
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free		
RT Channelized	_	_	None	_	_	None	_	_	None	_	-	None		
Storage Length	_	_	_	0	-	-	_	_	100	50	_	_		
Veh in Median Storage	.# -	0	_	_	0	_	_	0	-	_	0	_		
Grade, %	-	0	_	_	0	_	_	0	_	_	0	_		
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90		
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2		
Mvmt Flow	4	4	7	131	4	43	4	556	42	13	754	4		
WWITH TOW	7	4	ı	101	7	40	7	550	42	10	7 34	7		
Major/Minor I	Minor2			Minor1			Major1		ı	Major2				
Conflicting Flow All	1391	1388	756	1352	1348	556	758	0	0	598	0	0		
Stage 1	782	782	750	564	564	550	750	-	-	590	-	-		
•	609	606		788	784	_		_	_	_				
Stage 2	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-		
Critical Hdwy						0.22	4.12	-	-		-	-		
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	- 0.40	- 0.40	-	-	- 040	-	-		
Follow-up Hdwy	3.518	4.018	3.318	3.518		3.318		-	-	2.218	-	-		
Pot Cap-1 Maneuver	120	143		~ 127	151	531	853	-	-	979	-	-		
Stage 1	387	405	-	510	508	-	-	-	-	-	-	-		
Stage 2	482	487	-	384	404	-	-	-	-	-	-	-		
Platoon blocked, %								-	-		-	-		
Mov Cap-1 Maneuver	106	140		~ 120	148	531	853	-	-	979	-	-		
Mov Cap-2 Maneuver	106	140	-	~ 120	148	-	-	-	-	-	-	-		
Stage 1	384	400	-	506	504	-	-	-	-	-	-	-		
Stage 2	436	484	-	369	399	-	-	-	-	-	-	-		
Approach	EB			WB			NB			SB				
HCM Control Delay, s	27.9			135.1			0.1			0.2				
HCM LOS	D			F			V 11			V.E				
				'										
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1\	NBLn1V	VBLn2	SBL	SBT	SBR				
Capacity (veh/h)		853	-		173	120	428	979	_	-				
HCM Lane V/C Ratio		0.005	_	_	0.09		0.112		_	_				
HCM Control Delay (s)		9.2	0	_	27.9	179.1	14.5	8.7	<u>-</u>	_				
HCM Lane LOS														
		A	А	-	D	F	B	A	-	-				
HCM 95th %tile Q(veh)		0	-	-	0.3	7.7	0.4	0	-	-				
Notes														
~: Volume exceeds car	pacity	\$: De	elay exc	eeds 3	00s	+: Com	putation	Not De	efined	*: All	major v	olume ii	n platoon	

Intersection														
Int Delay, s/veh	18.7													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		4		Y	1			ર્ન	7	*	f.			
Traffic Vol, veh/h	4	4	6	124	4	54	4	500	40	17	679	4		
Future Vol, veh/h	4	4	6	124	4	54	4	500	40	17	679	4		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0		
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free		
RT Channelized	_	_	None	-	-	None	-	-	None	-	-	None		
Storage Length	-	-	-	0	-	-	-	-	100	50	_	-		
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-		
Grade, %	-	0	-	-	0	-	-	0	_	-	0	_		
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90		
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2		
Mvmt Flow	4	4	7	138	4	60	4	556	44	19	754	4		
WWWIICTIOW	т.	т.	'	100	7	00		000	77	13	704			
Major/Minor I	Minor2			Minor1			Major1		ı	Major2				
Conflicting Flow All	1412	1402	756	1364	1360	556	758	0	0	600	0	0		
Stage 1	794	794	730	564	564	-	730	-	-	-	-	-		
Stage 2	618	608	_	800	796	_	_	<u> </u>	_	_		_		
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_		4.12				
Critical Hdwy Stg 1	6.12	5.52	0.22	6.12	5.52	0.22	4.12		_	4.12				
, ,	6.12	5.52		6.12	5.52		-	-	-	-	-	-		
Critical Hdwy Stg 2			3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-		
Follow-up Hdwy	3.518	4.018		~ 125			853	-			-	-		
Pot Cap-1 Maneuver	116	140			148	531	003	-	-	977	-	-		
Stage 1	381	400	-	510	508	-	-	-	-	-	-	-		
Stage 2	477	486	-	379	399	-	-	-	-	-	-	-		
Platoon blocked, %	20	400	400	440	444	=0.4	0=0	-	-	077	-	-		
Mov Cap-1 Maneuver	98	136		~ 118	144	531	853	-	-	977	-	-		
Mov Cap-2 Maneuver	98	136	-	~ 118	144	-	-	-	-	-	-	-		
Stage 1	378	392	-	506	504	-	-	-	-	-	-	-		
Stage 2	416	483	-	361	391	-	-	-	-	-	-	-		
Approach	EB			WB			NB			SB				
HCM Control Delay, s	29.1			144.5			0.1			0.2				
HCM LOS	D			F										
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1\	VBLn1\	VBLn2	SBL	SBT	SBR				
Capacity (veh/h)		853	-	-	165	118	448	977	-	_				
HCM Lane V/C Ratio		0.005	_	-		1.168			-	_				
HCM Control Delay (s)		9.2	0	_	29.1		14.4	8.8	_	_				
HCM Lane LOS		A	A	_	D	F	В	A	_	_				
HCM 95th %tile Q(veh)		0	-	-	0.3	8.5	0.5	0.1	-	-				
Notes														
~: Volume exceeds cap	nacity	\$: Da	elay exc	pade 3	nns -	+. Com	nutation	n Not De	efined	*· ΔII	maior v	oluma i	n platoon	
. volume exceeds cap	Jacity	ψ. De	ay ext	eeus 3	005	·. Com	pulation	I NOL DE	Sillieu	. All	majur V	Juille I	η ριαισση	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	1			ર્લ	7	1	1	
Traffic Volume (vph)	4	4	6	124	4	54	4	500	40	17	679	4
Future Volume (vph)	4	4	6	124	4	54	4	500	40	17	679	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		100	50		0
Storage Lanes	0		0	1		0	0		1	1		0
Taper Length (ft)	25			25			25			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.937			0.859				0.850		0.999	
Flt Protected		0.987		0.950						0.950		
Satd. Flow (prot)	0	1723	0	1770	1600	0	0	1863	1583	1770	1861	0
Flt Permitted		0.987		0.950				0.996		0.950		
Satd. Flow (perm)	0	1723	0	1770	1600	0	0	1855	1583	1770	1861	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		55			30			45			45	
Link Distance (ft)		1719			1308			2533			1115	
Travel Time (s)		21.3			29.7			38.4			16.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	4	4	7	138	4	60	4	556	44	19	754	4
Shared Lane Traffic (%)	•	•	•	100	•		•	000	• • •		701	•
Lane Group Flow (vph)	0	15	0	138	64	0	0	560	44	19	758	0
Turn Type	Split	NA		Split	NA	•	Perm	NA	pm+ov	Prot	NA	•
Protected Phases	4	4		8	8			2	8	1	6	
Permitted Phases	•	•					2	_	2	•	•	
Detector Phase	4	4		8	8		2	2	8	1	6	
Switch Phase	•	•					-	_		•	•	
Minimum Initial (s)	7.0	7.0		7.0	7.0		12.0	12.0	7.0	7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		19.0	19.0	14.0	14.0	19.0	
Total Split (s)	14.0	14.0		23.0	23.0		69.0	69.0	23.0	14.0	83.0	
Total Split (%)	11.7%	11.7%		19.2%	19.2%		57.5%	57.5%	19.2%	11.7%	69.2%	
Maximum Green (s)	7.0	7.0		16.0	16.0		62.0	62.0	16.0	7.0	76.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	2.0	-2.0		-2.0	-2.0		2.0	-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)		5.0		5.0	5.0			5.0	5.0	5.0	5.0	
Lead/Lag		0.0		0.0	0.0		Lead	Lead	0.0	Lag	0.0	
Lead-Lag Optimize?							Yes	Yes		Yes		
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None		C-Min	C-Min	None	None	C-Min	
Act Effct Green (s)	110110	9.3		16.6	16.6		O IVIIII	81.5	106.0	9.5	87.5	
Actuated g/C Ratio		0.08		0.14	0.14			0.68	0.88	0.08	0.73	
v/c Ratio		0.11		0.57	0.29			0.44	0.03	0.14	0.56	
Control Delay		53.3		56.8	48.4			11.0	0.4	53.5	11.6	
Queue Delay		0.0		0.0	0.0			0.0	0.0	0.0	0.0	
Total Delay		53.3		56.8	48.4			11.0	0.4	53.5	11.6	
LOS		55.5 D		50.0 E	40.4 D			11.0 B	0.4 A	55.5 D	11.0 B	
Approach Delay		53.3			54.2			10.2	A	U	12.6	
Approach LOS		55.5 D			54.2 D			10.2 B			12.0 B	
Approach LOS		U			U			В			D	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)		11		101	45			3	0	14	183	
Queue Length 95th (ft)		33		161	84			494	m1	38	504	
Internal Link Dist (ft)		1639			1228			2453			1035	
Turn Bay Length (ft)									100	50		
Base Capacity (vph)		133		279	252			1285	1385	139	1376	
Starvation Cap Reductn		0		0	0			0	0	0	0	
Spillback Cap Reductn		0		0	0			0	0	0	0	
Storage Cap Reductn		0		0	0			0	0	0	0	
Reduced v/c Ratio		0.11		0.49	0.25			0.44	0.03	0.14	0.55	
Intersection Summary												
7 1	ther											
Cycle Length: 120												
Actuated Cycle Length: 120												
Offset: 0 (0%), Referenced to	phase 2:	NBTL and	6:SBT, 9	Start of G	reen							
Natural Cycle: 75												
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.57												
Intersection Signal Delay: 17.4					tersection							
Intersection Capacity Utilization	on 60.1%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
m Volume for 95th percentil	e queue i	s metered	l by upstr	eam sign	al.							
Splits and Phases: 2: Zebul	lon Road	(NC 96) 8	& Glorv R	oad/Wea	ver's Rido	e Site Dri	ve					
Ø2 (R)		,/-	,		- 1		Ø1	40	4	₹ 7ø8		



Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	₽	
Traffic Vol., veh/h	4	4	13	604	604	4
Future Vol, veh/h	4	4	13	604	604	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	_	0	0	_
Grade, %	0	_	<u>-</u>	0	0	_
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	4	4	14	671	671	4
MINITIL FIOW	4	4	14	0/1	0/1	4
Major/Minor I	Minor2		Major1	N	/lajor2	
Conflicting Flow All	1372	673	675	0	-	0
Stage 1	673	-	-	-	-	-
Stage 2	699	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	_	-	_	-
Critical Hdwy Stg 2	5.42	-	_	-	-	-
Follow-up Hdwy		3.318	2.218	-	-	_
Pot Cap-1 Maneuver	161	455	916	_	_	_
Stage 1	507	-	-	_	_	_
Stage 2	493	_	_	_	_	_
Platoon blocked, %	400			_	_	_
Mov Cap-1 Maneuver	157	455	916	_	_	_
Mov Cap-1 Maneuver	157	-	-	_	_	_
Stage 1	495		_		_	_
Stage 2	493	-	_	-	_	
Stage 2	493	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	21.1		0.2		0	
HCM LOS	С					
		NE	NET	EDL 1	057	000
Minor Lane/Major Mvm	<u>it</u>	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		916	-	233	-	-
HCM Lane V/C Ratio		0.016		0.038	-	-
HCM Control Delay (s)		9	0	21.1	-	-
HCM Lane LOS		Α	Α	С	-	-
HCM 95th %tile Q(veh)		0	-	0.1	-	-

Intersection													
Int Delay, s/veh	12.1												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4		*	î,			र्स	7	*	1		
Traffic Vol., veh/h	4	4	4	75	4	25	14	665	127	42	669	4	
Future Vol, veh/h	4	4	4	75	4	25	14	665	127	42	669	4	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	_	-	None	_	_	None	
Storage Length	_	-	_	0	-	-	-	_	100	50	_	_	
Veh in Median Storage	e.# -	0	_	_	0	_	_	0	_	_	0	_	
Grade, %	-,	0	_	_	0	_	_	0	_	_	0	_	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	4	4	4	83	4	28	16	739	141	47	743	4	
	-	T		- 00		20	10	, 00	171	- 11	1 10		
Major/Minor	Minor2			Minor1			Major1		ı	Major2			
Conflicting Flow All	1697	1751	745	1614	1612	739	747	0	0	880	0	0	
Stage 1	839	839	745	771	771	739	141	-	-	000	-	-	
ū	858	912		843	841								
Stage 2			- 6.00			6.00	4.12	-	-	4 40	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	- 0.40	-	-	- 0.40	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318		-		2.218	-	-	
Pot Cap-1 Maneuver	73	86	414	84	104	417	861	-	-	768	-	-	
Stage 1	360	381	-	393	410	-	-	-	-	-	-	-	
Stage 2	352	353	-	358	380	-	-	-	-	-	-	-	
Platoon blocked, %							221	-	-		-	-	
Mov Cap-1 Maneuver	61	78	414	~ 74	94	417	861	-	-	768	-	-	
Mov Cap-2 Maneuver	61	78	-	~ 74	94	-	-	-	-	-	-	-	
Stage 1	346	358	-	378	394	-	-	-	-	-	-	-	
Stage 2	313	340	-	328	357	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	49			179.6			0.2			0.6			
HCM LOS	Е			F									
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBL n1\	VBL n2	SBL	SBT	SBR			
Capacity (veh/h)		861	-	-	95	74	283	768	-	-			
HCM Lane V/C Ratio		0.018	_	_		1.126			_				
HCM Control Delay (s)		9.3	0			241.6	19.3	10					
HCM Lane LOS		9.5 A	A	_	49 E	241.0 F	19.5 C	A	-	-			
HCM 95th %tile Q(veh)	١	0.1	-	_	0.5	6.2	0.4	0.2					
	,	0.1			0.5	0.2	0.4	U.Z					
ì													
Notes ~: Volume exceeds car			elay exc					n Not De					n platoon

Intersection													
Int Delay, s/veh	15.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4		*	1>			ન	7	*	1>		
Traffic Vol, veh/h	4	4	4	79	4	35	14	665	134	59	669	4	
Future Vol, veh/h	4	4	4	79	4	35	14	665	134	59	669	4	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	_	_	-	0	_	-	_	_	100	50	_	-	
Veh in Median Storage		0	_	-	0	_	_	0	-	-	0	_	
Grade, %	, π	0	_	_	0	_	<u>-</u>	0	<u>-</u>	<u>-</u>	0	<u>-</u>	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	4	4	4	88	4	39	16	739	149	66	743	4	
MINITE FIOW	4	4	4	00	4	39	10	139	149	00	743	4	
Major/Minor	Minor2		ı	Minor1			Major1		N	Major2			
	1744	1797			1650	739	747	0	0	888	0	0	
Conflicting Flow All	877	877	745	1652 771									
Stage 1			-		771	-	-	-	-	-	-	-	
Stage 2	867	920	-	881	879	-	4.40	-	-	1.10	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
ollow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	68	80	414	~ 79	99	417	861	-	-	763	-	-	
Stage 1	343	366	-	393	410	-	-	-	-	-	-	-	
Stage 2	348	350	-	341	365	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	54	70	414	~ 68	87	417	861	-	-	763	-	-	
Mov Cap-2 Maneuver	54	70	-	~ 68	87	-	-	-	-	-	-	-	
Stage 1	330	334	-	378	394	-	-	-	-	-	-	-	
Stage 2	300	337	-	304	333	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	55.1			214.2			0.2			0.8			
HCM LOS	F			F									
Minor Lane/Major Mvn	nt	NBL	NBT	NBR I	EBLn1V	VBLn1V	VBLn2	SBL	SBT	SBR			
Capacity (veh/h)		861	-	-	85	68	300	763	_				
HCM Lane V/C Ratio		0.018	-	_		1.291			-	_			
HCM Control Delay (s)		9.3	0	-		310.6	19	10.2	-	-			
HCM Lane LOS		A	A	_	F	F	C	В	_	-			
HCM 95th %tile Q(veh)	0.1	-	-	0.5	7.1	0.5	0.3	-	-			
Notes	,												
~: Volume exceeds ca		ф. D	dane se	eeds 30	20-			n Not De	المماك	*. A II	!	ا ساما	n platoon

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	1			ર્લ	7	7	1	
Traffic Volume (vph)	4	4	4	79	4	35	14	665	134	59	669	4
Future Volume (vph)	4	4	4	79	4	35	14	665	134	59	669	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		100	50		0
Storage Lanes	0		0	1		0	0		1	1		0
Taper Length (ft)	25			25			25			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.955			0.864				0.850		0.999	
Flt Protected		0.984		0.950				0.999		0.950		
Satd. Flow (prot)	0	1750	0	1770	1609	0	0	1861	1583	1770	1861	0
Flt Permitted		0.984		0.950				0.979		0.950		
Satd. Flow (perm)	0	1750	0	1770	1609	0	0	1824	1583	1770	1861	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		55			30			45			45	
Link Distance (ft)		1719			1404			2533			1320	
Travel Time (s)		21.3			31.9			38.4			20.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	4	4	4	88	4	39	16	739	149	66	743	4
Shared Lane Traffic (%)	•	•	•		•			100	1 10	00	7 10	•
Lane Group Flow (vph)	0	12	0	88	43	0	0	755	149	66	747	0
Turn Type	Split	NA		Split	NA		Perm	NA	pm+ov	Prot	NA	
Protected Phases	4	4		8	8			2	8	1	6	
Permitted Phases	•	•					2	_	2	•		
Detector Phase	4	4		8	8		2	2	8	1	6	
Switch Phase	•	•					-	_		•		
Minimum Initial (s)	7.0	7.0		7.0	7.0		12.0	12.0	7.0	7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		19.0	19.0	14.0	14.0	19.0	
Total Split (s)	14.0	14.0		23.0	23.0		69.0	69.0	23.0	14.0	83.0	
Total Split (%)	11.7%	11.7%		19.2%	19.2%		57.5%	57.5%	19.2%	11.7%	69.2%	
Maximum Green (s)	7.0	7.0		16.0	16.0		62.0	62.0	16.0	7.0	76.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)		-2.0		-2.0	-2.0			-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)		5.0		5.0	5.0			5.0	5.0	5.0	5.0	
Lead/Lag		0.0		0.0	0.0		Lag	Lag	0.0	Lead	0.0	
Lead-Lag Optimize?							Yes	Yes		Yes		
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None		C-Min	C-Min	None	None	C-Min	
Act Effct Green (s)	110110	9.0		13.4	13.4		O IVIIII	78.1	95.5	10.7	91.0	
Actuated g/C Ratio		0.08		0.11	0.11			0.65	0.80	0.09	0.76	
v/c Ratio		0.09		0.45	0.24			0.64	0.12	0.42	0.53	
Control Delay		53.5		56.2	50.5			6.2	1.0	60.3	9.3	
Queue Delay		0.0		0.0	0.0			0.0	0.0	0.0	0.0	
Total Delay		53.5		56.2	50.5			6.2	1.0	60.3	9.3	
LOS		55.5 D		50.2 E	50.5 D			0.2 A	Α	00.5 E	9.5 A	
Approach Delay		53.5			54.4			5.4	A		13.4	
Approach LOS		55.5 D			34.4 D						13.4 B	
Approach LOS		U			U			А			D	

07/28/2022

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)		9		65	31			74	0	49	150	
Queue Length 95th (ft)		29		114	65			169	m13	98	431	
Internal Link Dist (ft)		1639			1324			2453			1240	
Turn Bay Length (ft)									100	50		
Base Capacity (vph)		131		265	241			1191	1297	157	1411	
Starvation Cap Reductn		0		0	0			0	0	0	0	
Spillback Cap Reductn		0		0	0			0	0	0	0	
Storage Cap Reductn		0		0	0			0	0	0	0	
Reduced v/c Ratio		0.09		0.33	0.18			0.63	0.11	0.42	0.53	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 12.7
Intersection Capacity Utilization 68.3%

Intersection LOS: B
ICU Level of Service C

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Zebulon Road (NC 96) & Glory Road/Weaver's Ridge Site Drive



Intersection												
Int Delay, s/veh	4.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	61	14	31	43	21	15	13	57	12	29	297	20
Future Vol, veh/h	61	14	31	43	21	15	13	57	12	29	297	20
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	300	2	2	2	2
Mvmt Flow	68	16	34	48	23	17	14	63	13	32	330	22
Major/Minor	Minor2			Minor1			Major1		ľ	Major2		
Conflicting Flow All	523	509	341	528	514	70	352	0	0	76	0	0
Stage 1	405	405	-	98	98	_	-	-	-	-	-	-
Stage 2	118	104	-	430	416	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	465	467	701	461	464	993	1207	-	-	1523	-	-
Stage 1	622	598	-	908	814	-	-	-	-	-	-	-
Stage 2	887	809	-	603	592	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	426	449	701	414	446	993	1207	-	-	1523	-	-
Mov Cap-2 Maneuver	426	449	-	414	446	-	-	-	-	-	-	-
Stage 1	615	582	-	897	804	-	-	-	-	-	-	-
Stage 2	837	799	-	544	577	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	14.8			14.3			1.3			0.6		
HCM LOS	В			В								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR	EBLn1V	VRI n1	SBL	SBT	SBR			
Capacity (veh/h)		1207	- 1101	-	485	476	1523		יופט			
HCM Lane V/C Ratio		0.012	_			0.184		_	_			
HCM Control Delay (s)		8	0	_	14.8	14.3	7.4	0	_			
HCM Lane LOS		A	A	_	14.0 B	14.3 B	Α.4	A	_			
HCM 95th %tile Q(veh))	0	-	_	0.9	0.7	0.1	-	_			
HOW JOHN JOHN GUIVEN		U			0.0	0.1	0.1					

Intersection												
Int Delay, s/veh	6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LDL	4	LDI	1100	4	TIDIO	HUL	4	HOIL	ODL	4	CDIK
Traffic Vol, veh/h	85	17	39	47	24	16	16	62	13	32	324	29
Future Vol, veh/h	85	17	39	47	24	16	16	62	13	32	324	29
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage	e.# -	0	_	_	0	_	_	0	_	_	0	_
Grade, %	-, "	0	-	_	0	-	-	0	_	-	0	_
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	94	19	43	52	27	18	18	69	14	36	360	32
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	583	567	376	591	576	76	392	0	0	83	0	0
Stage 1	448	448	370	112	112	-	J9Z -	-	-	00	-	-
Stage 2	135	119	_	479	464	_	_	_		_		
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12	_	_
Critical Hdwy Stg 1	6.12	5.52	U.ZZ -	6.12	5.52	-	- 1.12	_	_		_	_
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	_	_	_	_	_	_	_
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	_	_	2.218	_	_
Pot Cap-1 Maneuver	424	433	670	419	428	985	1167	_	_	1514	_	_
Stage 1	590	573	-	893	803	-	- 101	_	_		_	_
Stage 2	868	797	_	568	564		_	_		_	_	_
Platoon blocked, %	300	101		500	JU-1			_	_		_	_
Mov Cap-1 Maneuver	382	413	670	365	408	985	1167	-	-	1514	_	_
Mov Cap-2 Maneuver	382	413	-	365	408	-		_	_	-	_	_
Stage 1	581	555	-	879	790	-	-	-	-	-	-	-
Stage 2	810	784	-	497	547	_	_	_	_	_	_	_
Approach	EB			WB			NB			SB		
HCM Control Delay, s	17.7			15.9			1.4			0.6		
HCM LOS	C			C			1.7			0.0		
TOW LOO	J			J								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)	•	1167	-	-	438	427			-			
HCM Lane V/C Ratio		0.015	_		0.358			_				
HCM Control Delay (s)		8.1	0	-	17.7	15.9	7.4	0				
HCM Lane LOS		Α	A	_	C	C	Α	A	_			
HCM 95th %tile Q(veh)	0	-	_	1.6	0.9	0.1		_			
HOW JOHN JOHNE W(VEI)	1	U		_	1.0	0.9	0.1		_			

Intersection												
Int Delay, s/veh	6.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	97	17	42	47	24	16	17	62	13	32	324	33
Future Vol, veh/h	97	17	42	47	24	16	17	62	13	32	324	33
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	_	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	_	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	108	19	47	52	27	18	19	69	14	36	360	37
Major/Minor	Minor2			Minor1			Major1			Major2		
	588	F70	379	598	583	76	397	0		83	0	0
Conflicting Flow All	451	572 451		114	114	76	39 <i>1</i>	U	0	03	0	0
Stage 1		121	-	484	469		-	-	-	-	-	-
Stage 2	137		6.00			6.00	4 40	-	-	4 40	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	2 240	- 0.40	-	-	- 0.40	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	421	430	668	414	424	985	1162	-	-	1514	-	-
Stage 1	588	571	-	891	801	-	-	-	-	-	-	-
Stage 2	866	796	-	564	561	-	-	-	-	-	-	-
Platoon blocked, %	070	440	000	050	40.4	005	4400	-	-	4544	-	-
Mov Cap-1 Maneuver	378	410	668	358	404	985	1162	-	-	1514	-	-
Mov Cap-2 Maneuver	378	410	-	358	404	-	-	-	-	-	-	-
Stage 1	578	553	-	876	787	-	-	-	-	-	-	-
Stage 2	808	782	-	491	544	-	-	<u>-</u>	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	18.8			16.1			1.5			0.6		
HCM LOS	С			С								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1162	-	-	432	420	1514	-				
HCM Lane V/C Ratio		0.016	_		0.401		0.023	_	_			
HCM Control Delay (s)		8.1	0	_	18.8	16.1	7.4	0	_			
HCM Lane LOS		A	A	_	C	C	A	A	_			
HCM 95th %tile Q(veh))	0.1	-	-	1.9	0.9	0.1	-	-			
		V . 1			1.0	5.5	J .,					

Intersection												
Int Delay, s/veh	4.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	43	19	25	24	28	44	28	275	46	17	165	74
Future Vol, veh/h	43	19	25	24	28	44	28	275	46	17	165	74
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	48	21	28	27	31	49	31	306	51	19	183	82
Major/Minor I	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	696	681	224	681	697	332	265	0	0	357	0	0
Stage 1	262	262	-	394	394	-	-	-	-	-	-	-
Stage 2	434	419	-	287	303	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	356	373	815	364	365	710	1299	-	-	1202	-	-
Stage 1	743	691	-	631	605	-	-	-	-	-	-	-
Stage 2	600	590	-	720	664	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	298	355	815	323	347	710	1299	-	-	1202	-	-
Mov Cap-2 Maneuver	298	355	-	323	347	-	-	-	-	-	-	-
Stage 1	721	678	-	612	587	-	-	-	-	-	-	-
Stage 2	513	572	-	661	651	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	17.6			15.7			0.6			0.5		
HCM LOS	С			C								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR	EBLn1V	VRI n1	SBL	SBT	SBR			
Capacity (veh/h)		1299	-	-	381	442		-				
HCM Lane V/C Ratio		0.024	-		0.254							
HCM Control Delay (s)		7.8	0	-	17.6	15.7	8	0	_			
HCM Lane LOS		7.0 A	A	<u> </u>	17.0	13.7 C	A	A	_			
HCM 95th %tile Q(veh)	\	0.1	-		1	0.9	0	-	_			
How Jour Joure Q(Ver)		0.1	_	_		0.9	U		_			

Intersection												
Int Delay, s/veh	5.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	59	22	30	27	33	48	36	300	51	19	180	101
Future Vol, veh/h	59	22	30	27	33	48	36	300	51	19	180	101
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	66	24	33	30	37	53	40	333	57	21	200	112
Major/Minor I	Minor2			Minor1			Major1		l	Major2		
Conflicting Flow All	785	768	256	769	796	362	312	0	0	390	0	0
Stage 1	298	298	-	442	442	-	-	-	-	-	-	-
Stage 2	487	470	-	327	354	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	310	332	783	318	320	683	1248	-	-	1169	-	-
Stage 1	711	667	-	594	576	-	-	-	-	-	-	-
Stage 2	562	560	-	686	630	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	247	311	783	273	300	683	1248	-	-	1169	-	-
Mov Cap-2 Maneuver	247	311	-	273	300	-	-	-	-	-	-	-
Stage 1	682	652	-	570	552	-	-	-	-	-	-	-
Stage 2	464	537	-	618	616	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	23.2			18.4			0.7			0.5		
HCM LOS	С			С								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1248	-	-	319	387	1169	-	-			
HCM Lane V/C Ratio		0.032	_		0.387	0.31	0.018	_	_			
HCM Control Delay (s)		8	0	_		18.4	8.1	0	-			
HCM Lane LOS		A	A	_	C	C	A	A	_			
HCM 95th %tile Q(veh))	0.1	-	_	1.8	1.3	0.1	-	_			
1.5W Cour June W(VOII)		0.1			1.0	1.0	J. 1					

Intersection												
Int Delay, s/veh	6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	67	22	32	27	33	48	39	300	51	19	180	114
Future Vol, veh/h	67	22	32	27	33	48	39	300	51	19	180	114
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	74	24	36	30	37	53	43	333	57	21	200	127
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	799	782	264	784	817	362	327	0	0	390	0	0
Stage 1	306	306	-	448	448	-	-	-	-	-	-	-
Stage 2	493	476	-	336	369	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	304	326	775	311	311	683	1233	-	-	1169	-	-
Stage 1	704	662	-	590	573	-	-	-	-	-	-	-
Stage 2	558	557	-	678	621	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	240	304	775	265	290	683	1233	-	-	1169	-	-
Mov Cap-2 Maneuver	240	304	-	265	290	-	-	-	-	-	-	-
Stage 1	672	647	-	563	547	-	-	-	-	-	-	-
Stage 2	458	532	-	609	607	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	25.4			18.9			0.8			0.5		
HCM LOS	D			С								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1233		-	308	378	1169	_				
HCM Lane V/C Ratio		0.035	_		0.437			_	_			
HCM Control Delay (s)		8	0	-		18.9	8.1	0	_			
HCM Lane LOS		A	A	_	D	C	A	A	_			
HCM 95th %tile Q(veh))	0.1	-	_	2.1	1.3	0.1	-	_			
		V. I				0	V. 1					

Traffic Signal Warrant Analysis

Warrants 1 - 3 (Volume Warrants)

Project Name	Weaver's Pointe
Project/File #	22390
Scenario	2025 Build

Intersection Information							
Major Street (N/S Road)	NC 96 (Zebulon Road)	Minor Street (E/W Road)	Glory Road / Site Access				
Analyzed with	1 approach lane	Analyzed with	2 or more approach lanes				
Total Approach Volume	2779 vehicles	Total Approach Volume	303 vehicles				
Total Ped/Bike Volume	0 crossings	Total Ped/Bike Volume	0 crossings				
Right turn reduction of	100 percent applied	Right turn reduction of	0 percent applied				

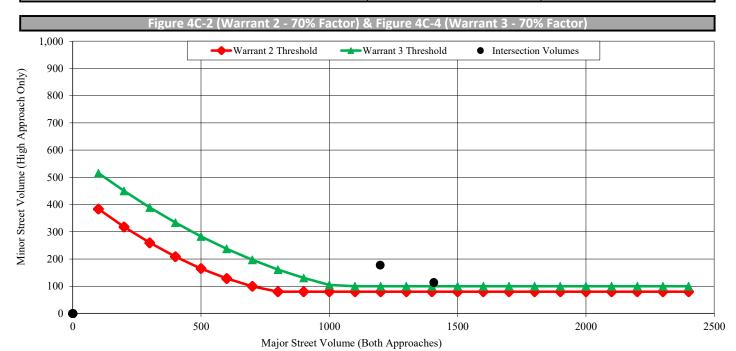
No high speed or isolated community reduction applied to the Volume Warrant thresholds.

Warrant 1, Eight Hour Vehicular Volume							
	Condition A	Condition B	Condition A+B*				
Condition Satisfied?	Not Satisfied	Not Satisfied	Not Satisfied				
Required values reached for	1 hour	2 hours	2 (Cond. A) & 2 (Cond. B)				
Criteria - Major Street (veh/hr)	350	525	280 (Cond. A) & 420 (Cond. B)				
Criteria - Minor Street (veh/hr)	140	70	112 (Cond. A) & 56 (Cond. B)				

* Should be applied only after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems.

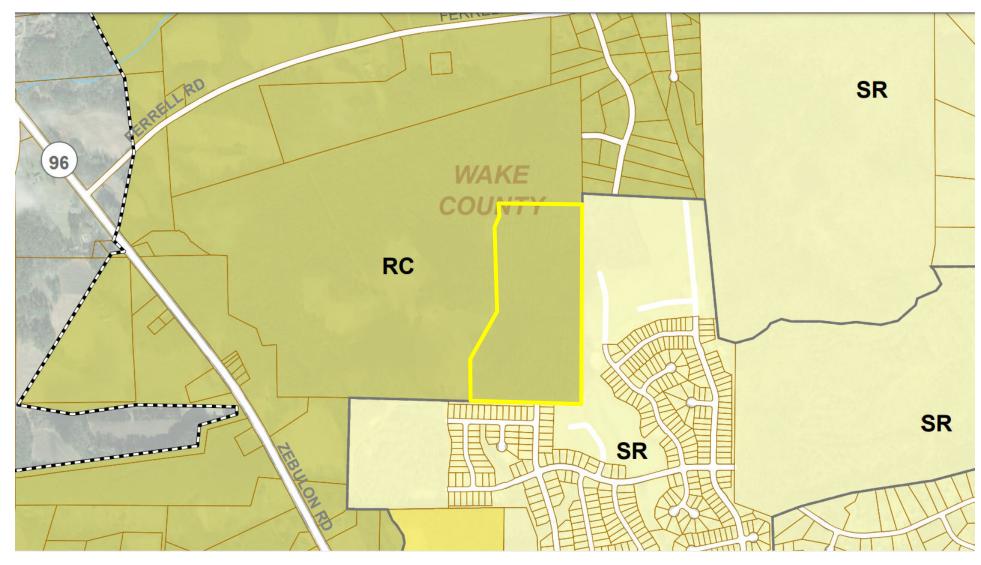
Warrant 2, Four Hour Vehicular Volume					
Condition Satisfied?	Not Satisfied				
Required values reached for	2 hours				
Criteria	See Figure Below				

Warrant 3, Peak Hour Vehicular Volume							
Condition A Condition B							
Condition Satisfied?	Not Satisfied	Satisfied					
Required values reached for	1422 total, 178 minor, 0 delay	2 hours					
Criteria - Total Approach Volume (veh in one hour)	800						
Criteria - Minor Street High Side Volume (veh in one hour)	100	See Figure Below					
Criteria - Minor Street High Side Delay (veh-hrs)	4						



Attachment 2 - Future Land Use and Character Map









General Residential (GR)



Suburban Residential (SR)



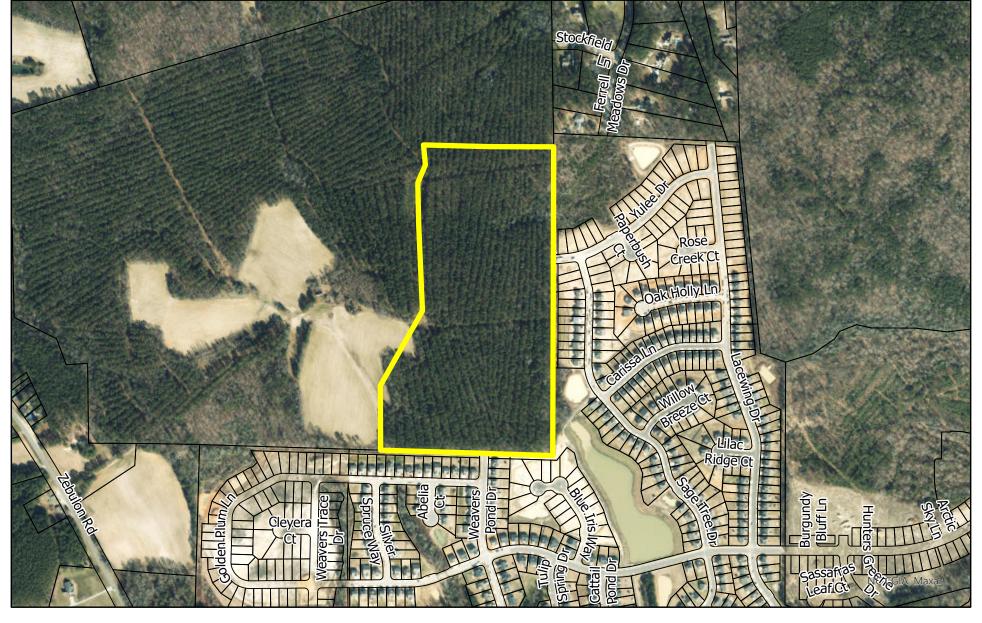
Rural Conservation (RC)



Comprehensive Plan Study Area

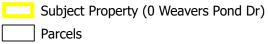
Attachment 3 - Aerial Map



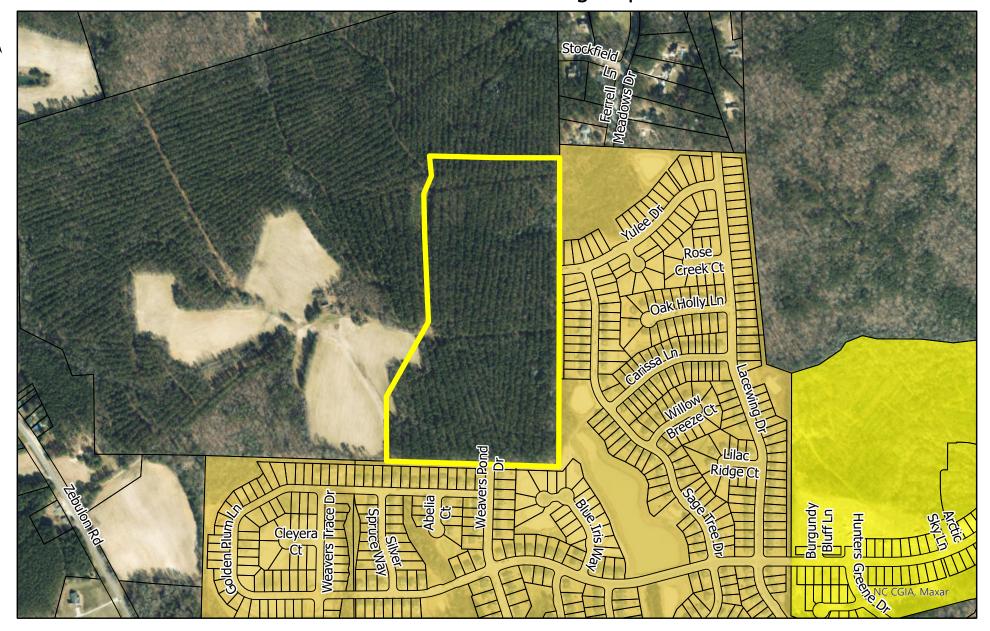






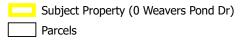


Attachment 4 - Zoning Map









Zoning Districts

R4, Residential Neighborhood

R-13 SUD, Residential 13 - Special Use Districts



View of property at the end of Yulee Dr



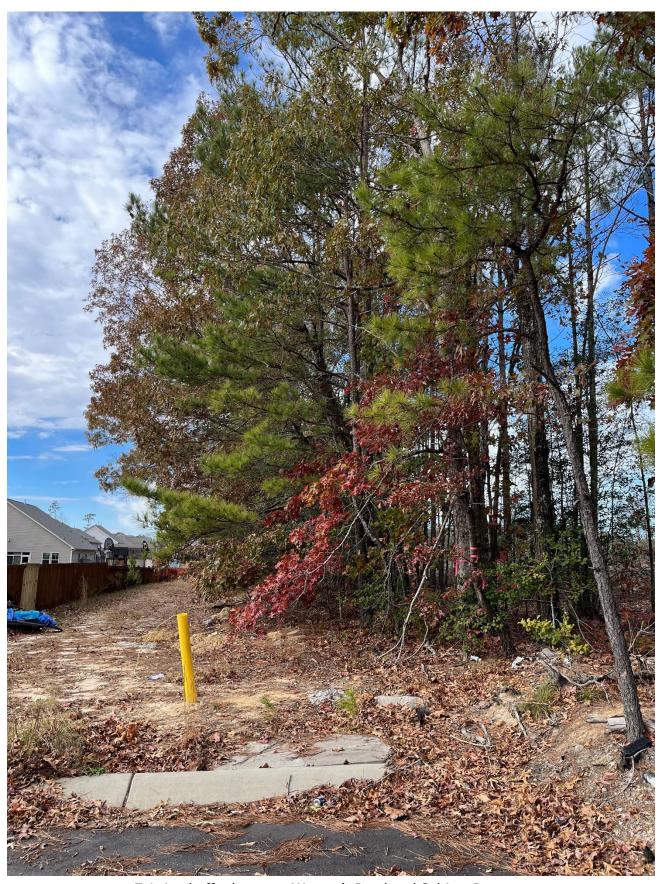
View north on property near Yulee Dr



View west on property



View south on property (remaining trees on the left)



Existing buffer between Weaver's Pond and Subject Property



CASE # CZ 2023-02 IDT# 825909 - The Wall Purdy Tract Conditional Rezoning

PROJECT ADDRESS 0 Weavers Pond Dr

PIN NUMBER: 1797701367

HEARING DATE: March 13, 2023

State of North Carolina	
County of Wake	
this day of day of lawful ag deposes and says:	_ 20 <u>3</u> 3, personally appeared Michael J. Clark,
I Michael J. Clark, acting as the Planning Director for to Public Notice Procedures have been completed in acceptatute and Town of Zebulon Unified Development Of above referenced hearing.	cordance with applicable North Carolina General
 First Class Mailing Sent on 2/22/2023 (see attention of the second content of the second content	on sent on 2/27/2023 (Wake weekly, publication n 2/22/2023 (pictures attached)
Michael J. Clark, AICP, CZO	Date
Subscribed and sworn to before me, this	day of March 20 23
[Notary Seal:] STACIE PARATORE NOTARY PUBLIC WAKE COUNTY, N.C.	
Stain Paytore	Stacie Paratore
[signature of Notary]	[printed name of Notary]
NOTARY PUBLIC	
My commission expires: $(1/\sqrt{37})$, 20 $\sqrt{35}$.	

Notice of Public Hearing

Notice is hereby given pursuant to the provisions of Article 2.2.6 of the Town of Zebulon Unified Development Ordinance that a public hearing will be held on March 13, 2023 at 6:00 PM at the Zebulon Municipal Complex, 1003 N. Arendell Avenue, and will be conducted by the Board of Commissioners and Planning Board of the Town of Zebulon for the purpose of considering the following items:

IDT Project Number 825909 - CZ 2023-02 - The Wall Purdy Tract (0 Weavers Pond Dr)

PIN # 1797701367. A request by Jason Barron of Morningstar Law Group on behalf of Weaver's Pond Development Company, LLC and property owners Wall Purdy Family LLC and Philip Randolph Isley, for a Conditional Rezoning to the Residential Neighborhood Conditional (R4-C) zoning district for the development of 87 single-family residential lots.

Public comments may be submitted to Deputy Town Clerk Stacie Paratore at SParatore@TownofZebulon.org no later than 12:00 Noon on the day of the hearing to be read into the record. Links will be provided along with the full application packet and documentation on the Planning Department web page at https://www.townofzebulon.org/departments/planning/public-hearing-information For questions or additional information, please contact us at (919) 823-1816.

Posted in Wake Weekly March 3 and March 10, 2023 Mailed 150-foot Radius February 22, 2023 Planning E-mail Distribution List February 27, 2023 Posted to Planning Website February 22, 2023

PIN_NUM	OWNER	ADDR1	ADDR2
	5 CEESAY, DAM JAW CEESAY, AWA	2140 BLUE IRIS WAY	ZEBULON NC 27597-9270
	7 AVERETTE, LINDA G TRUSTEE AVERETTE, JAMES R TRUSTEE	1701 FERRELL MEADOWS DR	ZEBULON NC 27597-9512
	SCOTT, NATHANIEL LOVELACE-SCOTT, YAKIMA	564 YULEE DR	ZEBULON NC 27597-3308
	SHUSTER, STEVEN M	2156 BLUE IRIS WAY	ZEBULON NC 27597-9270
	REAMS, SHERITA WITHERSPOON REAMS, REGINALD L.	2144 BLUE IRIS WAY	ZEBULON NC 27597-9270
	7 SELLERS, TIARA DENEEN BRYANT, TRAVIS LEE	1653 SAGE TREE DR	ZEBULON NC 27597-6083
) ROBINSON, DAVID L III	604 YULEE DR	ZEBULON NC 27597-3309
	WEAVERS POND DEVELOPMENT CO LLC	9407 BARTONS CREEK RD	RALEIGH NC 27615-9707
) WEAVERS POND DEVELOPMENT CO LLC	9407 BARTONS CREEK RD	RALEIGH NC 27615-9707
) WALL PURDY FAMILY LLC ISLEY, PHILIP RANDOLPH	3309 FELTON PL	RALEIGH NC 27612-5001
	WALL PURDY FAMILY LLC ISLEY, PHILIP RANDOLPH	3309 FELTON PL	RALEIGH NC 27612-5001
	BEZERRA, LUCILENE TORRES JAILLET, ZACHARY AARON	937 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	KNOLL, ERICA RUTH, LILLIAN	933 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	OOGA, ISAAC M OOGA, MARY N	913 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	B RORIE, BRETTA NICOLE	917 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	WEAVERS POND DEVELOPMENT CO LLC	9407 BARTONS CREEK RD	RALEIGH NC 27615-9707
	WEAVERS POND DEVELOPMENT CO LLC	9407 BARTONS CREEK RD	RALEIGH NC 27615-9707
	LOVE, RALPH EDWARD JR LOVE, TIA T	901 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	PELZER, VICTORIA	929 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	WEAVERS POND DEVELOPMENT CO LLC	9407 BARTONS CREEK RD	RALEIGH NC 27615-9707
	MCKOY, DAVID KENYATTA	925 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	YOUNG, LOUIS ANTHONY YOUNG, KIMBERLY DEWON	909 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	KILKUSKIE, INELL M STANTON	4176 WEAVERS POND DR	ZEBULON NC 27597-6271
	OCHOA TORRES, SUSANA IRENE RAMIREZ TORRES, LUIS ALEXIS	889 GOLDEN PLUM LN	ZEBULON NC 27597-9724
	HILLS, TASHA CALHOUN	893 GOLDEN PLUM LN	ZEBULON NC 27597-9724
	RICHARDSON, CHRYSTAL	885 GOLDEN PLUM LN	ZEBULON NC 27597-9724
	TUCK, SARAH	921 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	BRYAN, ALICIA	4180 WEAVERS POND DR	ZEBULON NC 27597-6271
	NEWKIRK, GENIA LARESE	897 GOLDEN PLUM LN	ZEBULON NC 27597-9724
	FIGUEROA, ARMANDO ODOM, KAYLEIGH	905 GOLDEN PLUM LN	ZEBULON NC 27597-9725
	WEAVERS POND DEVELOPMENT CO LLC	9407 BARTONS CREEK RD	RALEIGH NC 27615-9707
	WITHROW, KENNETH W WITHROW, REBECCA	1629 SAGE TREE DR	ZEBULON NC 27597-6083
1797707562	VIERA, MELANIE G LANGLEY, TEHREL M	1649 SAGE TREE DR	ZEBULON NC 27597-6083

1797707158 MCCLARY, SHANTELLE HAWKINS, TIFFANY	1631 6465 7055 00	
	1621 SAGE TREE DR	ZEBULON NC 27597-6083
1797707850 PARKER-MINCEY, JEREMY	600 YULEE DR	ZEBULON NC 27597-3309
1796795391 HILL, DIEVA SESSOMS, KELVIN JR	2148 BLUE IRIS WAY	ZEBULON NC 27597-9270
1797707263 THOMPSON, ASHLEY JOLANE	1625 SAGE TREE DR	ZEBULON NC 27597-6083
1797707051 LONIE, COURTNEY DUHAMEL, JUSTIN	1609 SAGE TREE DR	ZEBULON NC 27597-6083
1797707460 CUTTINO, GRACE	1637 SAGE TREE DR	ZEBULON NC 27597-6083
1797707466 HOSKINSON, CHRISTOPHER MICHAEL HOSKINSON, LAURA	1641 SAGE TREE DR	ZEBULON NC 27597-6083
1797707057 LOPEZ, ALDO ALFREDO TEYTUD DE, SANTIAGO ISAMAR	1613 SAGE TREE DR	ZEBULON NC 27597-6083
1797707152 MOORE, RON STEVAN MOORE, LEZEL K	1617 SAGE TREE DR	ZEBULON NC 27597-6083
1797707633 MENDOZA, CARLA	1657 SAGE TREE DR	ZEBULON NC 27597-6083
1797707364 MONTANEZ, KAROL ILEANA BRUNO COLLAZO, GIAN CARLO CUSTODIO	1633 SAGE TREE DR	ZEBULON NC 27597-6083
1796795313 WATKINS, DONTE BELL, DONNIECE	2152 BLUE IRIS WAY	ZEBULON NC 27597-9270
Service Control of the Control of th	LIDE DEOL INID VVAI	ZEDOLON NC 2/39/-92/0

